

Simulation of Canal and Control-Pond Operation at the Quivira National Wildlife Refuge, South-Central Kansas

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CONVERSION FACTORS, ABBREVIATION, AND DEFINITIONS

Multiply	By	To obtain
acre	4,047	square meter
acre-foot (acre-ft)	1,233	cubic meter
acre-foot per day (acre-ft/d)	1,233	cubic meter per day
cubic foot per day (ft ³ /d)	0.02832	cubic meter per day
cubic foot per second (ft ³ /s)	0.02832	cubic meter per second
foot (ft)	0.3048	meter
foot per day (ft/d)	0.3048	meter per day
foot per second (ft/s)	0.3048	meter per second
foot per second squared (ft/s ²)	0.3048	meter per second squared
inch (in.)	2.54	centimeter
inch per day (in/d)	25.4	millimeter per day
inch per year (in/yr)	25.4	millimeter per year
mile (mi)	1.609	kilometer
mile per day (mi/d)	1.609	kilometer per day
millimeter (mm)	0.03937	inch
millimeter per day (mm/d)	0.03937	inch per day
square foot (ft ²)	0.09290	square meter
square mile (mi ²)	2.590	square kilometer

Temperature can be converted to degrees Celsius (°C) or degrees Fahrenheit (°F) by the equations:

$$^{\circ}\text{C} = 5/9 (^{\circ}\text{F} - 32),$$

$$^{\circ}\text{F} = 9/5 (^{\circ}\text{C}) + 32.$$

Sea level: In this report, “sea level” refers to the National Geodetic Vertical Datum of 1929—a geodetic datum derived from a general adjustment of the first-order level nets of the United States and Canada, formerly called Sea Level Datum of 1929.

Water year: Water year is the 12-month period October 1 through September 30. The water year is designated by the calendar year in which it ends. Thus, the year ending September 30, 1991, is called the “1991 water year.”

Simulation of Canal and Control-Pond Operation at the Quivira National Wildlife Refuge, South-Central Kansas

By Xiaodong Jian

Abstract

Efficient water management of the Quivira National Wildlife Refuge, located in the Rattlesnake Creek Basin of south-central Kansas, is a complicated task. In a cooperative study with the Kansas Geological Survey, the U.S. Geological Survey developed a computer-based water-budget and flow-routing model to assist the U.S. Fish and Wildlife Service in determining the outcome of possible water-management options. The computer model uses network analysis to determine the optimal operation of canals and control ponds on the refuge. Applications of the model are presented that investigate the daily operation of canals and control ponds on the refuge using historical discharges and pond water levels.

Simulation of the daily operation of the canal and control-pond system at the refuge from June 11 through December 11, 1996, was conducted using the computer model. Simulated pond water levels matched well with measured ones. The root mean square error (RMSE) between the simulated and measured water levels in ponds was less than 0.13 foot except for two ponds. Water storage in ponds during the simulation period was substantially reduced due to water-surface evaporation and canal-flow transmission losses. Simulation of canal and control-pond operation was also conducted with different target pond water levels. This simulation used 1991 discharge, precipitation, and water-surface evaporation data to consider model results during drought conditions. Results indicated that lowering target pond water levels reduced water-surface evaporation, resulted

in more water stored in ponds at the north end of the refuge, and caused a substantial decrease in the final volume of water stored in the main water-storage unit at the south end of the refuge (Little Salt Marsh).

INTRODUCTION

The Quivira National Wildlife Refuge is located in the Rattlesnake Creek Basin of south-central Kansas (fig. 1). The refuge was established in 1953 to provide food, water, and a resting place for waterfowl and certain endangered species during their annual migrations. To provide the proper type of feeding and resting areas for wildlife, water is diverted from Rattlesnake Creek into a system of canals and impoundments. There are more than 30 control marshes and ponds (collectively referred to hereinafter as ponds) (water units) ranging in size from 7 to 1,768 acres currently (1997) on the refuge, three main canals, and numerous smaller canals and waterways (fig. 2).

Water is managed to provide a mixture of marsh and wet-meadow habitat in and adjacent to the control ponds. Large ponds, such as Little Salt Marsh (water unit 5, fig. 2) at the south end of the refuge, provide habitat and serve as the main water-storage units on the refuge. It is difficult for the refuge manager to determine optimally how much water should be stored in the large ponds instead of being released to smaller ponds. Habitat losses occur when water is too deep in the larger ponds and also when there is insufficient water to supply the smaller ponds.

An additional complication in the surface-water management of the refuge results from ground-water inflows to the area. The north part of the refuge is within a ground-water discharge area, and some of the

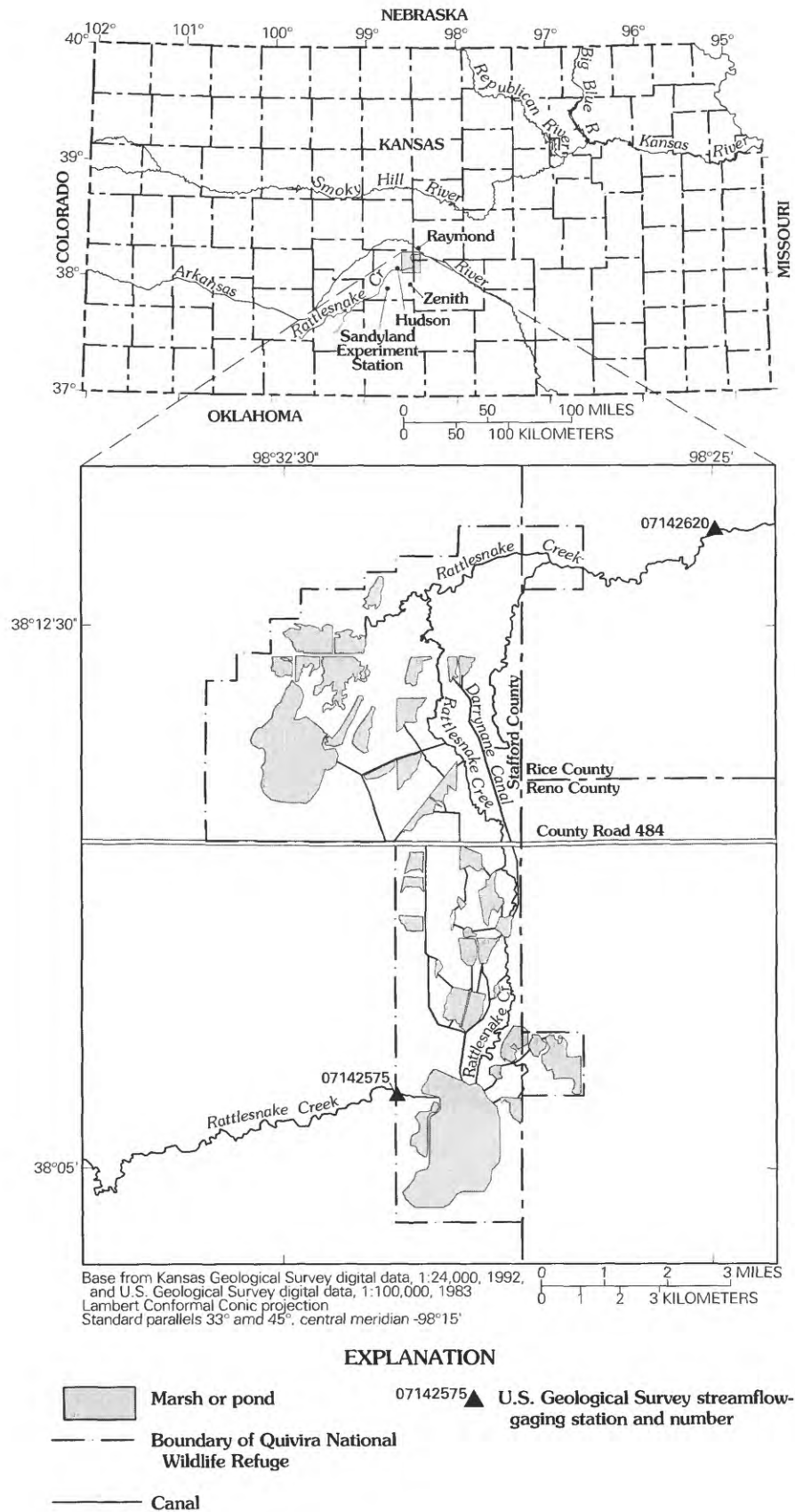
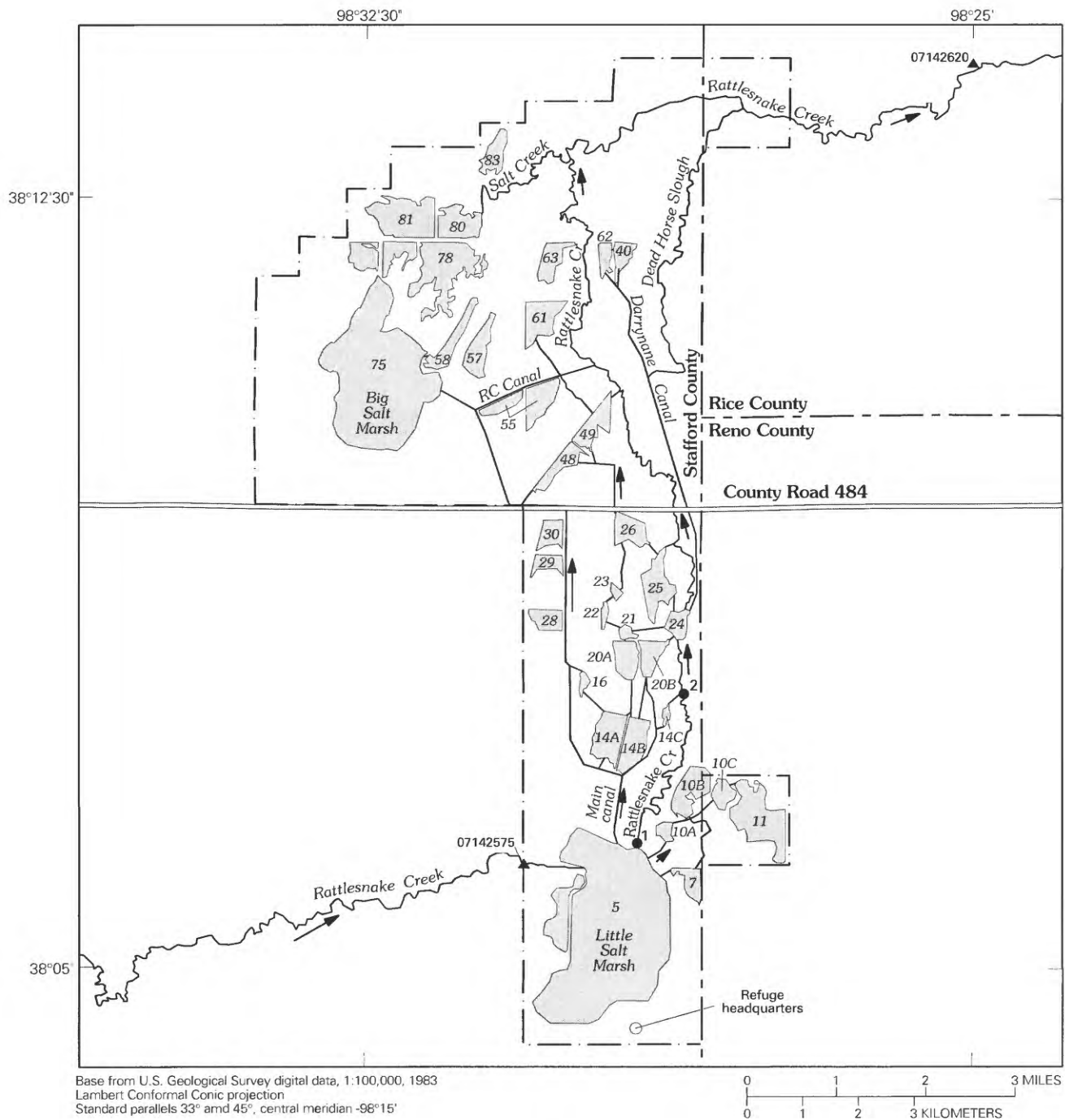


Figure 1. Location of Quivira National Wildlife Refuge, south-central Kansas.



EXPLANATION

- | | | | |
|--|---|--|--|
| | Marsh or pond —Number is water-unit number used for identification in tables | | U.S. Geological Survey streamflow-gaging station and number |
| | Boundary of Quivira National Wildlife Refuge | | Seepage test site and number |
| | Canal | | |
| | Direction of flow | | |

Figure 2. Ponds and canals at Quivira National Wildlife Refuge.

ponds receive a large part of their total water supply from ground-water sources (Megan Estep-Johnston, U.S. Fish and Wildlife Service, written commun., 1995). This is especially true of Big Salt Marsh (water unit 75, fig. 2). It is very difficult to determine the volume of water that is contributed to the refuge water supply from ground-water sources without a comprehensive simulation study.

Beginning in 1995, a 3-year study to develop a water-budget and flow-routing model to assist the U.S. Fish and Wildlife Service (USFWS) in determining the outcome of possible water-management options was begun by the U.S. Geological Survey (USGS) in cooperation with the Kansas Geological Survey (KGS). The objectives of the study were:

1. To develop a network flow model that incorporates linear-programming techniques to determine efficient management strategies for water use.
2. To provide simulation results using historic stream-flow and water-level data.

The network flow model developed for this study can be adapted to any configuration of canals and ponds for similar water-management problems.

Purpose and Scope

The purpose of this report is to describe the development of a computer model to simulate the water budget and surface-water flow routing for the Quivira National Wildlife Refuge to help determine the effects of possible water-management options on the distribution of available water within the refuge. The report includes a description of a network model to simulate the water budget of the refuge and the routing of water throughout the refuge. The network flow model consisted of nodes and arcs, where a node was any location in the refuge where the water budget was computed. Nodes represented all ponds, joints of canals, and any other terrestrial areas of interest. Arcs connected the nodes and allowed simulation of water movement from one node to another. A linear-network flow technique was used to simulate flow through arcs.

This report also presents results of simulations of daily operation of canals and ponds based on 1996 and 1991 conditions. The simulation for 1996 was used to investigate pond operation with measured pond levels. The simulation for 1991 was used to investigate the operation of ponds under drought flow conditions with different simulated pond target levels.

Acknowledgments

The author thanks Marios Sophocleous and Greg Pouch of the Kansas Geological Survey for providing valuable data and suggestions. The author also thanks Megan Estep-Johnston of the U.S. Fish and Wildlife Service and Quivira National Wildlife Refuge personnel for providing crucial information and essential hydrologic data. Special thanks are given to David M. Wolock and Andrew C. Ziegler of the U.S. Geological Survey for their advice throughout the study.

PHYSICAL AND HYDROLOGIC FEATURES OF QUIVIRA NATIONAL WILDLIFE REFUGE

Description of Refuge

Quivira National Wildlife Refuge is located in south-central Kansas in northeast Stafford County (fig. 1). The refuge covers about 32 mi². It contains more than 30 control marshes and ponds ranging in size from 7 to 1,768 acres and about 21 mi of canals ranging in length from 0.1 to 7 mi (fig. 2).

The nearest climatic station to the refuge is at Hudson, 9 mi west of the refuge (fig. 1). The Hudson station records daily precipitation and temperature. The Sandyland Experiment Station (approximately 18 mi southwest of the refuge, fig. 1) is operated by the Kansas State University Agricultural Extension Office in Manhattan, Kansas, and has been recording hourly precipitation and temperature data for the last several years. Precipitation data also have been collected at the refuge headquarters (fig. 2) since 1996 and at the USGS streamflow-gaging station near Zenith (station 07142575, fig. 1).

Surface Water

The major source of water to the refuge is Rattlesnake Creek. Hourly streamflow of Rattlesnake Creek is recorded at USGS streamflow-gaging stations near Zenith (station 07142575) and near Raymond (station 07142620). The long-term, lowest, and highest annual mean discharges for Rattlesnake Creek near Zenith for the 1973 through 1995 water years were 50.6, 6.59, and 186 ft³/s, respectively (Putnam and others, 1996). For Rattlesnake Creek near Raymond, the long-term, low-

est, and highest annual mean discharges for the same period were 48.4, 2.77, and 190 ft³/s, respectively (Putnam and others, 1996).

In addition to the water supplied by Rattlesnake Creek, surface runoff to ponds generated by precipitation also plays an important role. The delineated drainage areas of the ponds are listed in table 1 (Marios Sophocleous, Kansas Geological Survey, written commun., 1997). These drainage areas were used for the calculation of overland surface runoff to the ponds. Surface runoff was estimated using the SCS curve-number method (Soil Conservation Service, 1985). SCS curve numbers for control ponds at Quivira National Wildlife Refuge also are listed in table 1. A description of the SCS curve-number method is found in the section "Estimation of Direct Overland Surface Runoff."

The refuge currently diverts water from the Little Salt Marsh (water unit 5), which is supplied by Rattlesnake Creek, into the main canal and into water units 7, 10A, 10B, 10C, and 11 (fig. 2). Water also flows from the Little Salt Marsh back into Rattlesnake Creek. Water in the creek flows north to water unit 24, where part of the water is diverted into the Darrynane Canal and into units 21 and 25. Some water flows into Rattlesnake Creek north of unit 24 and is transported to the west and north into the units north of County Road 484.

Ground Water

The ponds in the north part of the refuge are within a ground-water discharge area. Table 2 shows the estimated monthly ground-water discharge from shallow aquifers to ponds for 1994 (Marios Sophocleous, Kansas Geological Survey, written commun., 1997). These values were estimated using a previous ground-water simulation done by Sophocleous and Perkins (1992) and the delineated drainage area of the ponds (table 1). The total ground-water discharge to ponds for 1994 was about 6,200 acre-ft.

Physical Features of Control Ponds

Bottom elevations and full-pond capacities of control ponds are listed in table 3 (Megan Estep-Johnston, U.S. Fish and Wildlife, written commun., 1995). To express mathematically the elevation-volume-area relation of a pond, the pond storage was first divided into several water-depth zones. The number of zones

Table 1. Drainage area and SCS curve number for control ponds at Quivira National Wildlife Refuge, south-central Kansas

[SCS, U.S. Soil Conservation Service. Drainage areas and SCS curve numbers are from the Kansas Geological Survey (Marios Sophocleous, written commun., 1997)]

Water-unit number (fig. 2)	Drainage area (acres)	SCS curve number
5	1,890.7	74.020
7	140.8	42.680
10A	84.9	48.397
10B	201.4	47.575
10C	84.5	47.575
11	341.4	47.575
14A	149.9	71.217
14B	124.9	71.693
14C	59.5	33.552
16	180.0	40.753
20A	179.4	58.461
20B	116.4	73.101
21	60.0	76.469
22	82.5	45.543
23	43.8	46.615
24	259.9	51.636
25	226.7	55.711
26	194.7	67.952
28	228.4	38.659
29	78.9	60.995
30	69.0	61.968
40	207.2	42.018
48	305.6	73.499
49	137.4	71.000
55	582.8	72.250
57	257.5	69.910
58	186.7	62.831
61	258.8	71.481
62	90.4	52.546
63	201.4	71.000
75	5,621.7	69.583
78	635.9	70.544
80	187.1	70.544
81	620.9	70.544
83	149.4	70.544
Total	14,240.5	

Table 2. Ground-water discharge to selected control ponds at Quivira National Wildlife Refuge for 1994

[Data from Kansas Geological Survey (Marios Sophocleous, written commun., 1997). Negative numbers indicate that pond gains water from shallow aquifers]

Water-unit number (fig. 2)	Monthly ground-water discharge (acre-feet per day)												Total discharge (acre-feet)
	January	February	March	April	May	June	July	August	September	October	November	December	
5	1.30	1.36	1.42	1.51	1.46	1.49	1.60	1.51	1.52	1.60	1.57	1.59	545.00
10B	.16	.17	.17	.18	.18	.18	.19	.18	.19	.19	.18	.18	65.46
14B	-.05	-.05	-.05	-.05	-.05	-.05	-.05	-.05	-.05	-.05	-.05	-.05	-18.00
14C	.09	.09	.09	.09	.09	.09	.10	.10	.10	.10	.10	.09	34.33
20B	.04	.04	.04	.04	.04	.04	.04	.04	.04	.04	.04	.04	15.16
24	.56	.57	.56	.59	.56	.56	.60	.56	.55	.57	.54	.54	206.02
25	.24	.23	.23	.24	.23	.23	.24	.22	.22	.22	.21	.21	82.66
26	.04	.04	.04	.04	.04	.04	.04	.04	.04	.04	.04	.03	13.70
40	-.35	-.32	-.29	-.27	-.26	-.24	-.22	-.21	-.19	-.18	-.17	-.16	-86.52
49	.03	.04	.04	.05	.05	.05	.07	.06	.06	.07	.07	.07	20.33
58	-.49	-.49	-.48	-.49	-.48	-.47	-.48	-.47	-.47	-.46	-.46	-.46	-173.42
61	-.36	-.35	-.33	-.32	-.31	-.30	-.28	-.29	-.28	-.26	-.26	-.25	-109.43
62	-.24	-.23	-.22	-.21	-.20	-.19	-.18	-.18	-.17	-.17	-.16	-.16	-70.29
63	-.51	-.50	-.48	-.46	-.46	-.44	-.42	-.43	-.42	-.40	-.40	-.39	-161.82
75	-14.20	-14.05	-13.95	-14.02	-13.86	-13.70	-13.81	-13.55	-13.45	-13.40	-13.28	-13.22	-5,006.67
78	-1.67	-1.65	-1.64	-1.65	-1.63	-1.61	-1.62	-1.59	-1.58	-1.57	-1.56	-1.55	-587.37
80	-.49	-.49	-.48	-.48	-.48	-.47	-.48	-.47	-.46	-.46	-.46	-.46	-172.86
81	-1.85	-1.83	-1.82	-1.83	-1.81	-1.78	-1.80	-1.76	-1.75	-1.74	-1.73	-1.72	-651.70
83	-.39	-.39	-.38	-.39	-.38	-.38	-.38	-.37	-.37	-.37	-.37	-.36	-137.91
Total	-18.14	-17.81	-17.53	-17.43	-17.27	-16.95	-16.84	-16.66	-16.47	-16.23	-16.15	-16.03	-6,193.33

Table 3. Full-pond elevations, water-surface areas, and capacities for selected control ponds at Quivira National Wildlife Refuge

[Data from U.S. Fish and Wildlife Service (Megan Estep-Johnston, written commun., 1995). BM, bench mark; ft, feet]

Water-unit number (fig. 2)	Bottom elevation (feet above sea level)	Full-pond elevation, in feet above sea level (datum location)		Full-pond surface area (acres)	Full-pond capacity (acre-feet)
5	1,780	1,783	(SPILLWAY)	864	1,866
7	1,774	1,778	(TOP OF STOPLOG SLOT)	26	40
10A&10B	1,774	1,779	(TOP OF STOPLOG SLOT)	64	145
10C	1,772	1,774.4	(TOP OF GAGE)	11	13
11	1,754	1,774.9	(SPILLWAY)	90	338
14A	1,772	1,778	(SPILLWAY)	87	196
14B	1,772	1,776.7	(SPILLWAY)	65	96
14C	1,774	1,777	(14C ¹ BM-0.67 ft)	7	16
16	1,768	1,775	(TOP OF STOPLOG SLOT)	31	80
20A	1,767	1,770.7	(SPILLWAY)	138	195
20B	1,767	1,770.7	(SPILLWAY)	138	195
21	1,764	1,770	(TOP OF STOPLOG SLOT)	30	81
22	1,764	1,766	(22A ¹ BM-0.6 ft)	10	13
23	1,762	1,764.3	(TOP OF GAGE)	9	15
24	1,765	1,769.4	(SPILLWAY)	31	35
25	1,762	1,768.4	(TOP OF GAGE)	94	296
26	1,758	1,762	(SPILLWAY)	59	111
28	1,762	1,768	(28A ¹ BM-0.86 ft)	85	153
29	1,757	1,762	(29C ¹ BM-0.58 ft)	61	91
30	1,756	1,759	high water	78	119
40	1,736	1,742.5	(40B ¹ BM-0.65 ft)	32	66
48	1,750	1,754.4	(SPILLWAY)	89	113
49	1,750	1,754.2	(SPILLWAY)	95	159
57	1,740	1,743.5	(57A ¹ BM-0.6 ft)	127	212
58	1,736	1,742	(58B ¹ BM-0.5 ft)	99	251
61	1,740	1,745.5	(62B ¹ BM-0.58 ft)	218	498
62	1,735	1,744	(TOP OF STOPLOG SLOT)	47	120
63	1,736	1,741.2	(TOP OF GAGE)	154	339
75	1,736	1,740.8	(SPILLWAY)	1,768	2,446
Total				4,607	8,298

¹Letters indicate structure names where water levels are measured.

were different for different ponds. For example, the number of zones for water unit 5 and water unit 24 were two and five, respectively (table 4). The bottom

elevation (above sea level) of each zone was called the zonal elevation base (Z_b) for the corresponding zone. The elevation-volume-area relation of a pond was rep-

Table 4. Zonal elevation base and regression coefficients for elevation-volume-area relations of selected control ponds at Quivira National Wildlife Refuge

[Data from U.S. Fish and Wildlife Service (Megan Estep-Johnston, written commun., 1995)]

Water-unit number (fig. 2)	Zone number	Zonal elevation base, Z_b (feet above sea level)	Regression coefficients		
			A1	A2	A3
5	1	1,780	1.0000	308.1200	110.4650
	2	1,782	1,059.0999	749.9802	56.9399
7	1	1,774	0	.1800	1.6575
	2	1,776	6.9900	6.8100	4.7775
	3	1,778	39.7200	25.9200	7.2600
10A	1	1,774	0	6.2900	4.5450
	2	1,776	30.7600	24.4700	3.6325
	3	1,778	94.2300	39.0000	12.2525
10B	1	1,774	0	6.2900	4.5450
	2	1,776	30.7600	24.4700	3.6325
	3	1,778	94.2300	39.0000	12.2525
10C	1	1,772	0	3.6700	.6825
	2	1,774	10.0700	6.4000	5.3450
11	1	1,754	0	.3000	.5050
	2	1,756	2.6200	2.3200	.5700
	3	1,758	9.5400	4.6000	.5875
	4	1,760	21.0900	6.9500	.6975
	5	1,762	37.7800	9.7400	.7775
	6	1,764	60.3700	12.8500	.7150
	7	1,766	88.9300	15.7100	1.5075
	8	1,768	126.3800	21.7400	2.3025
	9	1,770	179.0700	30.9500	3.0025
	10	1,772	252.9800	42.9600	1.2850
14A	1	1,772	0	3.6700	1.7150
	2	1,774	14.2000	10.5300	7.9625
	3	1,776	67.1100	42.3800	11.0625
14B	1	1,772	0	.0800	2.5050
	2	1,774	10.1800	10.1000	9.5525
	3	1,776	68.5900	48.3100	12.1175
14C	1	1,774	0	.3000	2.6500
	2	1,775	2.9500	5.6000	.3400
16	1	1,768	0	.4700	.9075
	2	1,770	4.5700	4.1000	.9850
	3	1,772	16.7100	8.0400	4.5925
	4	1,774	51.1600	26.4100	2.3550

Table 4. Zonal elevation base and regression coefficients for elevation-volume-area relations of selected control ponds at Quivira National Wildlife Refuge—Continued

Water-unit number (fig. 2)	Zone number	Zonal elevation base, Z_b (feet above sea level)	Regression coefficients		
			A1	A2	A3
20A	1	1,767	0	0.8800	0.8400
	2	1,768	1.7200	2.5600	20.9950
	3	1,769	25.2750	44.5500	35.8750
	4	1,770	105.7000	116.3000	15.8000
20 B	1	1,767	0	.8800	.8400
	2	1,768	1.7200	2.5600	20.9950
	3	1,769	25.2750	44.5500	35.8750
	4	1,770	105.7000	116.3000	15.8000
21	1	1,764	0	1.4200	1.3675
	2	1,766	8.3100	6.8900	2.8300
22	1	1,764	0	3.4700	1.6350
	2	1,766	13.4800	10.0100	1.2825
23	1	1,762	0	3.7900	1.1625
	2	1,764	12.2300	8.4400	1.0625
24	1	1,765	0	.1600	.3700
	2	1,766	.5300	.9000	.6200
	3	1,767	2.0500	2.1400	3.5750
	4	1,768	7.7650	9.2900	6.8600
	5	1,769	23.9150	23.0100	10.3950
25	1	1,762	0	.4600	2.3875
	2	1,764	10.4700	10.0100	15.9925
	3	1,766	94.4600	73.9800	4.1675
26	1	1,758	0	2.4800	5.4875
	2	1,760	26.9100	24.4300	8.7050
28	1	1,762	0	.0400	.8125
	2	1,764	3.3300	3.2900	6.7775
	3	1,766	37.0200	30.4000	13.7275
29	1	1,757	0	.0600	.2650
	2	1,758	.3250	.5900	1.6500
	3	1,759	2.5650	3.8900	5.2800
	4	1,760	11.7350	14.4500	13.6450
	5	1,761	39.8300	41.7400	9.4300
	6	1,762	91.0000	60.6000	12.8350
30	1	1,756	0	1.6200	12.7325
40	1	1,736	0	.1900	.2350

Table 4. Zonal elevation base and regression coefficients for elevation-volume-area relations of selected control ponds at Quivira National Wildlife Refuge—Continued

Water-unit number (fig. 2)	Zone number	Zonal elevation base, Z_b (feet above sea level)	Regression coefficients		
			A1	A2	A3
40	2	1,738	1.4400	1.2500	2.2725
	3	1,740	13.0300	10.3400	4.3375
	4	1,742	51.0600	27.6900	4.4375
48	1	1,750	0	.2700	.4750
	2	1,751	.7450	1.2200	2.1400
	3	1,752	4.1050	5.5000	14.3300
	4	1,753	23.9350	34.1600	21.3150
	5	1,754	79.4100	76.7900	15.6400
49	1	1,750	0	.4600	1.6450
	2	1,751	2.1050	3.7500	11.2350
	3	1,752	17.0900	26.2200	19.3450
	4	1,753	62.6550	64.9100	12.4750
57	1	1,740	0	5.5300	14.5825
	2	1,742	69.3900	63.8600	20.9075
58	1	1,736	0	2.2800	3.9775
	2	1,738	20.4700	18.1900	9.5700
	3	1,740	95.1300	56.4700	10.6425
61	1	1,740	0	10.2900	6.9975
	2	1,742	48.5700	38.2800	25.7175
62	1	1,735	0	.0100	.1000
	2	1,736	.1100	.2100	.1900
	3	1,737	.5100	.5900	.3150
	4	1,738	1.4150	1.2200	1.0550
	5	1,739	3.6900	3.3300	2.4350
	6	1,740	9.4550	8.2000	5.4750
	7	1,741	23.1300	19.1500	4.0800
	8	1,742	46.6360	27.3100	4.8150
63	1	1,736	0	.3800	6.0175
	2	1,738	24.8300	24.4500	24.6250
	3	1,740	172.2300	122.9500	13.0675
75	1	1,736	0	.3100	96.7650
	2	1,737	97.0750	193.8400	84.3200
	3	1,738	375.2350	362.4800	76.1350
	4	1,739	813.8500	514.7500	113.3850
	5	1,740	1,441.9851	741.5198	641.6354

resented by stepwise regression equations in terms of zonal water depth (Megan, Estep-Johnston, U.S. Fish and Wildlife, written commun., 1995):

$$V = A1 + A2 X + A3 X^2, \text{ and} \quad (1)$$

$$A = A2 + (A3 + A3) X, \quad (2)$$

where X is the zonal water depth and is equal to the difference between pond water-surface elevation (Z), in feet, and the corresponding zonal elevation base (Z_b), in feet; that is, $X = Z - Z_b$. $A1$, $A2$, and $A3$ are regression coefficients, volume (V) is in acre-feet, and water-surface area (A) is in acres.

Table 4 summarizes the zonal elevation bases and corresponding regression coefficients of selected control ponds (Megan Estep-Johnston, U.S. Fish and Wildlife Service, written commun. 1995).

As an illustration of the use of these regression equations, consider water unit 5 as an example. Let the water-surface elevation Z be 1,782.5 ft. From table 4, the water surface is located in zone 2 because the water-surface elevation of 1,782.5 ft is higher than the zonal elevation base (Z_b) of 1,782 ft. Therefore, the zonal water depth (X) is $1,782.5 - 1,782.0 = 0.5$ ft with the regression coefficients ($A1$, $A2$, and $A3$) of 1,059.0999, 749.9802, and 56.9399, respectively. Using equations 1 and 2, the corresponding water volume (V) and water-surface area (A) are 1,448.32 acre-ft and 806.92 acres, respectively. However, if the water-surface elevation Z is 1,781.0 ft, the corresponding zone number now is 1 with a zonal elevation base (Z_b) of 1,780 ft and regression coefficients ($A1$, $A2$, and $A3$) of 1.0000, 308.1200, and 110.4650, respectively. Therefore, the corresponding water volume (V) and water-surface area (A) are 419.59 acre-ft and 529.05 acres, respectively. Figure 3 shows the elevation-volume-area curves for Little Salt Marsh (water unit 5, fig. 2) using equations 1 and 2.

LINEAR-NETWORK FLOW MODEL

The optimal operation of the control ponds at the Quivira National Wildlife Refuge can be formulated mathematically as a linear-network flow problem (Jian, 1988; Yu and others, 1989). In this section, the mathematical formulation of a linear-network flow model (Jian, 1988) is modified and expanded. The concepts of

a rule curve for a pond and the zoning of pond storage and canal flow are introduced in this section. These two concepts are bases for formulating a network flow problem. The operating policy for a pond system in terms of priority and cost-penalty coefficients is also discussed. By combining the concepts of a rule curve and zoning and the operating policy, the problem of the operation of pond storage and flow routing can be formulated as a minimum-cost network flow problem, which is a typical topic in network flow analysis.

Network Representation of Flow Systems

To apply network flow analysis to the Quivira National Wildlife Refuge, the flow systems shown in figure 2 were conceptually represented by a network of nodes and arcs (fig. 4). The network was comprised of 67 nodes, of which 34 nodes are pond nodes (oval shape in fig. 4). Water unit 55 was shown as an oval in figure 4 and treated as a canal node because there was no pond information for that unit. Ninety-seven (97) arcs were used to represent canals or waterways on the refuge. Water unit 34 in figure 4 is a proposed pond for future use and is not currently (1997) in operation.

Rule-Curve and Zoning Concepts

A rule curve designated a target water level in a control pond. Using the zoning concept, a control pond at the refuge was divided into four storage zones—extended upper zone, upper zone, lower zone, and inactive zone—and the rule curve was set at the top of the lower zone (fig. 5). The extended upper zone was used during periods of flood. The upper zone and lower zone were called conservation zones and were used to represent normal use. The inactive zone represented the storage area filled up by sediment accumulation. The selection of the number of zones in a particular pond was based on management needs. For example, if the objective of management was to maximize water yields, the target water level (that is, rule curve) was set at the highest elevation of a pond so that high water levels could be maintained after satisfying downstream flow requirements and water demands.

Similarly, flow in a canal was also divided into an upper zone (that is, above-normal zone), a normal zone, and a lower zone (that is, below-normal zone) as shown in figure 6. The selection of the number of canal flow zones was also dependent on management needs.

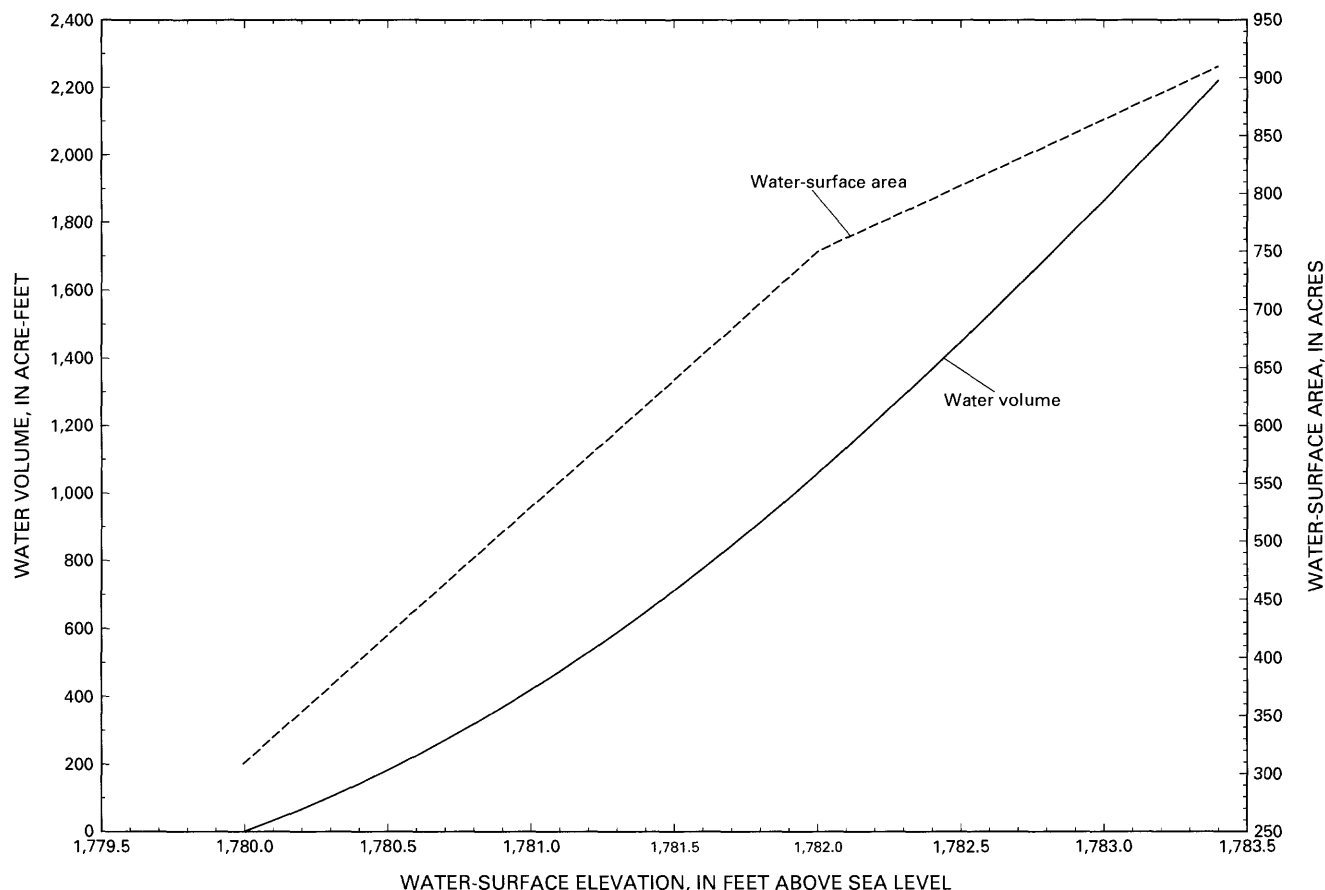


Figure 3. Relations of water-surface elevation, water volume, and water-surface area for Little Salt Marsh (water unit 5, fig. 2).

Because flows in canals at the refuge are not regulated, one flow zone (normal) was used in the flow model development. In model simulations, canal flows were maintained in the normal flow zone as long as possible.

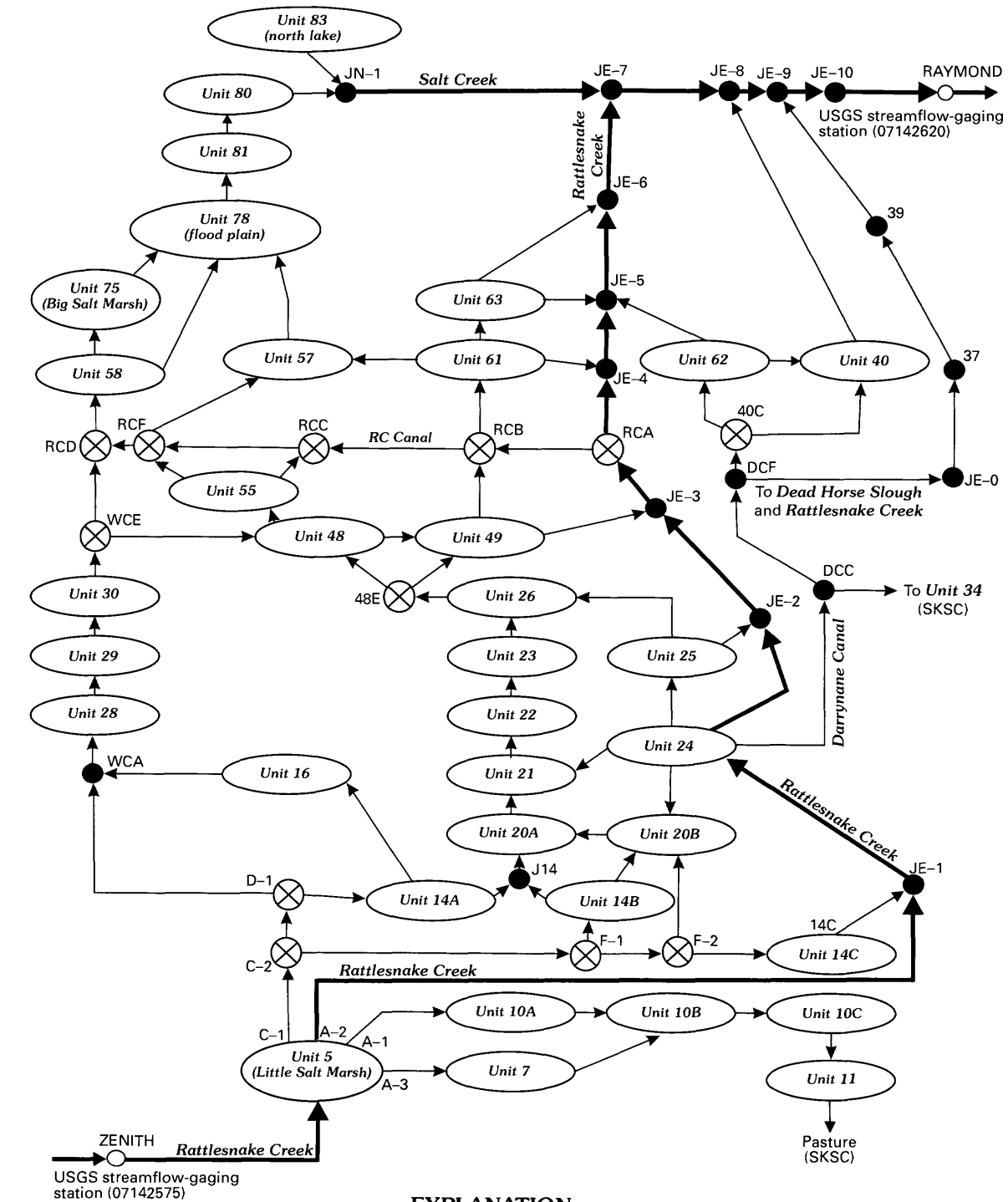
Operating Policy

Under ideal inflow conditions, all pond levels would be maintained at the target water levels (rule curves), and all canal flows would be maintained in the normal flow ranges in addition to satisfying water-management requirements such as minimum desirable streamflow (Kansas Water Office, written commun., 1996). In reality, ideal inflow conditions rarely occur. If a pond water level was higher or lower than its rule curve, a “cost” or “penalty” was assessed to the water storage or depletion deviation from the rule curve. The penalty depended on the amount of water deviation from the target level and the penalty coefficient (cost per unit water deviation from the target

level). A penalty was also assessed to canal flows. In other words, penalty coefficients were assigned to each storage zone of a pond and each flow zone in a canal to assess penalty.

Different penalty coefficients were assigned according to management priorities related to each storage zone of a pond. Penalty coefficients for canal flows were specified in a similar way. Higher penalty coefficients were assigned to the extended upper zone and inactive zone, and smaller penalty coefficients were assigned to the conservation zone (the lower zone and the upper zone) because water levels needed to be maintained in the conservation zone for normal use. The penalty coefficient in the normal-flow zone in a canal was generally zero or less than the penalty coefficient of the pond conservation zone. A higher penalty coefficient was assigned for violation of normal flow range; that is, the higher values of penalty coefficients were assigned to the upper and lower flow zones.

To optimally operate the canal and pond system at the refuge, it was necessary for some interpond rela-



EXPLANATION

	Marsh or pond node and nodal name	(SKSC)	End node of a canal is outside of refuge
	Canal node and nodal name		Creek arc
	Structure node and nodal name		Canal or waterway arc
	U.S. Geological Survey streamflow-gaging station node and nodal name		

Figure 4. Network representation of flow systems at Quivira National Wildlife Refuge.

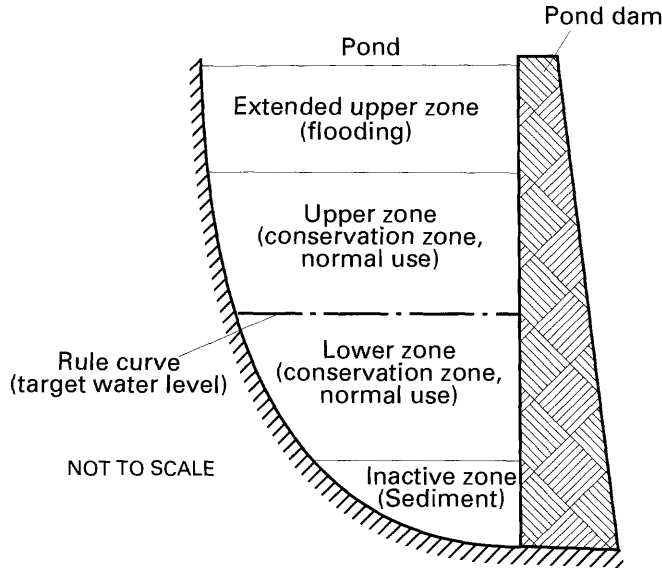


Figure 5. Concepts of rule curve and pond zoning.

tions to be incorporated into the flow model. One of the relations was priority ranking of the ponds. Ponds were ranked according to some specified criteria by assigning different penalty coefficients to storage zones. The lowest priority pond was assigned the smallest penalty coefficient for the same-purpose storage zone; higher priority ponds had higher penalty coefficients. Using this relation, violation of the rule curve first occurred in the pond with the lowest priority. It was common for rule-curve violations to occur first in the downstream ponds rather than the upstream ones. This procedure minimized unnecessary spilling at the most downstream pond in the event of high lateral flows; that is, flows that did not enter the system through upstream ponds. The optimal operation of the canal and pond system minimized the total penalty assessed on the deviations of pond storage from the rule curve and of canal flows from specified normal flows.

Mathematical Expression of Linear-Network Flow Model

The flow network consisted of nodes and directed arcs. A node represented a location where the computation of the water budget was needed, such as at ponds and at canals where diversion of water occurred. An arc represented a stream or a canal along which water moved from one location to another. An arc was also used to represent a storage deviation of a pond or canal,

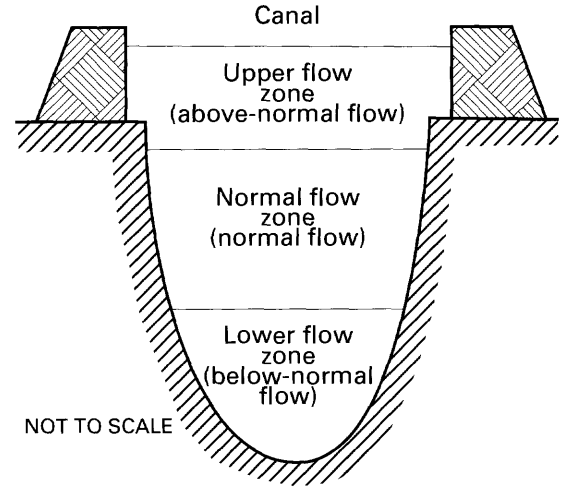


Figure 6. Concepts of canal-flow zoning.

and other additional contributions such as evaporation, seepage, and runoff.

The linear-network flow model was expressed mathematically as a linear programming problem of minimizing the total cost or penalty as follows:

$$\text{Minimize } \sum_i \sum_j C_{ij} Q_{ij} \text{ for all } (i,j) \text{ arcs,} \quad (3)$$

$$\text{subject to } \sum_j Q_{ji} - \sum_j Q_{ij} = 0 \text{ for all } i \text{ nodes, and} \quad (4)$$

$$L_{ij} \leq Q_{ij} \leq U_{ij} \text{ for all } (i,j) \text{ arcs,} \quad (5)$$

where

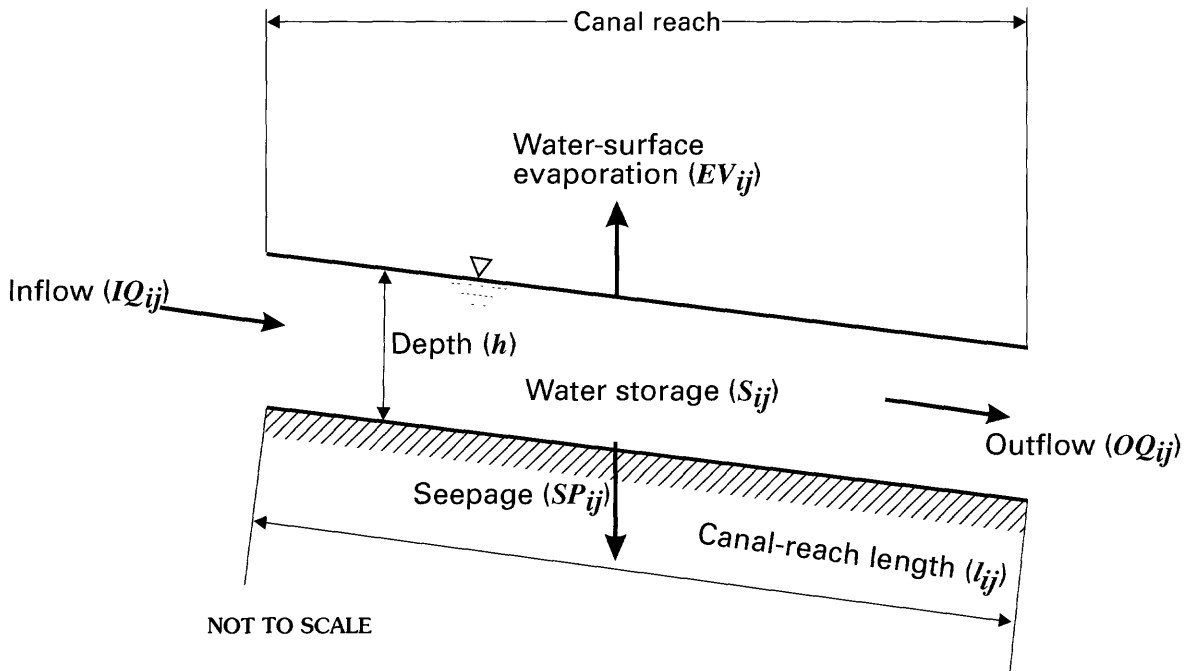
Q_{ij} = flow in arc (i,j) from node i to node j ;
 C_{ij} = cost per unit flow in arc (i,j) , also called the penalty coefficient;

L_{ij} = the lower flow boundary in any arc (i,j) ;
 and

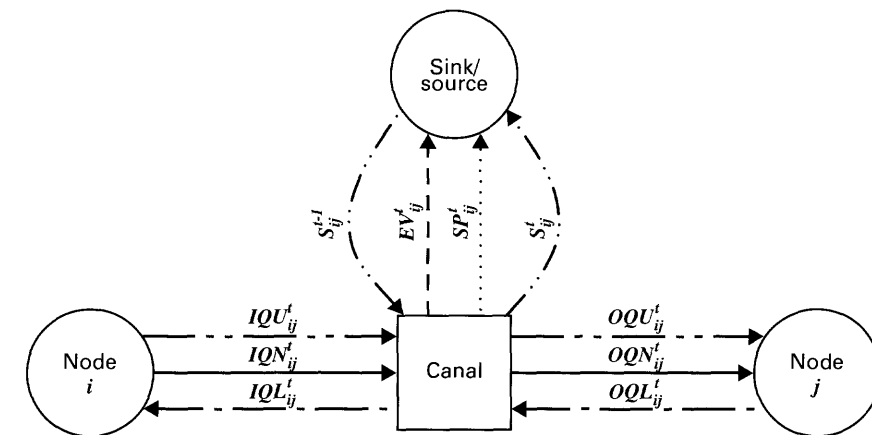
U_{ij} = the upper flow boundary in any arc (i,j) .

Any flow (choice of the Q_{ij} 's) satisfying the constraints in equation 4 was called a conserving flow, accounting for mass conservation at the nodes. A conserving flow that satisfied the remaining constraints in equation 5 was a feasible flow (solution).

The objective of equation 3 for the operation of canals and control ponds was to minimize the total cost due to deviations from specified rule curves and canal flows. Equations 3–5 needed to incorporate the con-

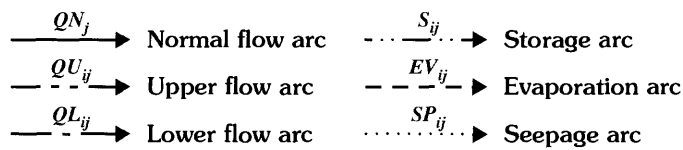


A. Water balance in a canal reach



NOT TO SCALE

EXPLANATION



B. Arc-node representation of canal-flow routing

Figure 7. (A) Water balance in a canal reach and (B) arc-node representation of canal-flow routing.

cepts of rule curve and zoning to obtain the appropriate water-balance equation for any pond or canal reach.

Canal Routing

Considering a canal reach (i, j) with a length l_{ij} , the water balance in the canal (fig. 7A) during time period (t) was expressed as:

$$IQ_{ij}^t - OQ_{ij}^t - (SP_{ij}^t + EV_{ij}^t) = S_{ij}^t - S_{ij}^{t-1}, \quad (6)$$

where

- IQ_{ij}^t = inflow to a canal reach (i, j) during time period t ;
- OQ_{ij}^t = outflow from a canal reach during time period t ;
- SP_{ij}^t = seepage along a canal reach during time period t . The amount of seepage depended on canal flow and hydraulic parameters. See section "Estimation of Canal-Flow Transmission Losses" later in this report.
- EV_{ij}^t = canal surface-water evaporation, which was estimated by:

$$EV_{ij}^t = e_{ij}^t A_{ij}^t, \quad (7)$$

in which e_{ij}^t was the water-surface evaporation coefficient for time period t , and A_{ij}^t was the water-surface area given by $A_{ij}^t = l_{ij} b_{ij}$, where l_{ij} and b_{ij} were the canal length and the width of the water surface, respectively.

S_{ij}^t = water storage in a canal reach at the end of time period t . Canal storage S_{ij}^t depended on canal inflow and outflow, and canal hydrologic parameters. See section "Estimation of Canal Water Storage" using Muskingum's method (McCuen, 1989) later in this report.

S_{ij}^{t-1} = water storage in a canal reach at the beginning of time period t .

In the concept of canal-flow zoning, canal flow may be in the normal, upper, or lower flow zone. The actual flow in a canal was denoted by Q and the normal flow by QN . The range of flow in the normal flow zone was $0 \leq \underline{QN} \leq QN \leq \overline{QN}$, where \underline{QN} and \overline{QN} were the lower and upper boundaries of the normal flow range, respectively. If canal flow (Q) was in the upper flow zone, then the upper flow (QU) was defined as the flow deviation from the upper boundary of the normal

flow range; that is, $QU = Q - \overline{QN}$, and

$0 \leq QU \leq \overline{QU}$, where \overline{QU} was the magnitude of the upper flow zone. In this case, $QN = \overline{QN}$, and $Q = QN + QU$. If canal flow (Q) was in the lower flow zone, then the lower flow (QL) was defined as the flow deviation from the lower boundary of the normal flow zone; that is, $QL = \underline{QN} - Q$, and $0 \leq QL \leq \overline{QL}$, where \overline{QL} was the magnitude of the lower flow zone. In this case, $QN = \underline{QN}$, and $Q = QN - QL$. Therefore, the actual flow Q in a canal reach could be expressed as normal flow (QN), plus the upper flow (QU), and minus the lower flow (QL); that is,

$$Q = QN + QU - QL. \quad (8)$$

If canal flow was in the normal flow zone, both QU and QL were equal to zero. If canal flow was in the upper zone, QL was zero. On the other hand, if canal flow was in the lower flow zone, QU was zero. Therefore, equation 8 represented all flow states in a canal.

Substituting equation 8 into equation 6 gave the canal water-balance equation as follows:

$$(IQN_{ij}^t + IQU_{ij}^t - IQL_{ij}^t) - (OQN_{ij}^t + OQU_{ij}^t - OQL_{ij}^t) - (SP_{ij}^t + EV_{ij}^t) = S_{ij}^t - S_{ij}^{t-1}, \quad (9)$$

where

IQN_{ij}^t = normal inflow ($0 \leq \underline{QN}_{ij} \leq IQN_{ij}^t \leq \overline{QN}_{ij}$),

IQU_{ij}^t = upper inflow ($0 \leq IQU_{ij}^t \leq \overline{QU}_{ij}$),

IQL_{ij}^t = lower inflow ($0 \leq IQL_{ij}^t \leq \overline{QL}_{ij}$),

OQN_{ij}^t = normal outflow

($0 \leq \underline{QN}_{ij} \leq OQN_{ij}^t \leq \overline{QN}_{ij}$),

OQU_{ij}^t = upper outflow ($0 \leq OQU_{ij}^t \leq \overline{QU}_{ij}$), and

OQL_{ij}^t = lower outflow ($0 \leq OQL_{ij}^t \leq \overline{QL}_{ij}$).

Each item in equation 9 could be represented by flow through a distinct arc (fig. 7B). If there was no water loss or storage change along the arc (i, j), then the actual flow between two neighboring nodes i and j was simplified as:

$$IQ_{ij}^t = OQ_{ij}^t = IQN_{ij}^t + IQU_{ij}^t - IQL_{ij}^t. \quad (10)$$

Pond-Storage Routing

Using the concepts of rule curve and zoning of a pond, the actual storage of a pond i at time t , S_i^t , was represented as the sum of the rule-curve storage, RC_i^t , plus the storage deviation from the rule-curve storage, D_i^t ; that is,

$$S_i^t = RC_i^t + D_i^t, \quad (11)$$

where

subscript i was the pond node index, and superscript t was the time period index.

In the concept of pond zoning, the storage deviation, D_i^t , was expressed as:

$$D_i^t = SU_i^t - SL_i^t, \quad (12)$$

where

SU_i^t = the actual storage deviation above the rule curve in the upper zone; that is, $SU_i^t = S_i^t - RC_i^t$, and $0 \leq SU_i^t \leq \overline{SU}_i$, where \overline{SU}_i was the total capacity of the upper zone in control pond i . If there were m upper zones, the upper deviation SU_i^t was calculated by:

$$SU_i^t = \sum_{k=1}^m SU_{i,k}^t, \quad (13)$$

where

$SU_{i,k}^t$ was the water storage in the upper zone k .
 SL_i^t = the actual storage deviation from the rule curve in the lower zone; that is, $SL_i^t = RC_i^t - S_i^t$, and $0 \leq SL_i^t \leq \overline{SL}_i$, where \overline{SL}_i was the total capacity of the lower zone in pond i . If there were n lower zones, the lower deviation SL_i^t was calculated by:

$$SL_i^t = \sum_{k=1}^n SL_{i,k}^t, \quad (14)$$

where

$SL_{i,k}^t$ was the water storage in the lower zone k . Only one of the two terms on the right side of equation 12 could be nonzero. In other words, if the pond water

level was in the upper zone, then $SL_i^t = 0$. On the other hand, if the pond water level was in the lower zone, then $SU_i^t = 0$.

Substituting equation 12 into equation 11 gave:

$$S_i^t = RC_i^t + SU_i^t - SL_i^t. \quad (15)$$

The water-balance equation (equation 3) for pond node i could then be rewritten as:

$$\sum_j OQ_{ji}^t - \sum_j IQ_{ij}^t + I_i^t + RN_i^t - EV_i^t - SP_i^t - W_i^t = S_i^t - S_i^{t-1}, \quad (16)$$

where

OQ_{ji}^t = canal inflow from the upstream node j during time period t .

IQ_{ij}^t = water release to downstream node j during time period t . Release was determined in terms of a downstream flow requirement, pond stage, and outlet control structure. See section "Flow Through Hydraulic Structures" later in this report.

I_i^t = local net inflow to pond i during time period t .

RN_i^t = precipitation falling onto the water surface (P_i^t) plus the direct overland surface runoff (RF_i^t) during time period t , given by:

$$RN_i^t = P_i^t + RF_i^t. \quad (17)$$

Precipitation falling onto the water surface, in acre-feet, P_i^t , was estimated by:

$$P_i^t = 0.0833 r_i^t A_i^t, \quad (18)$$

in which r_i^t was rainfall during time period t , in inches, and A_i^t was the water-surface area, in acres. Direct overland surface runoff (RF_i^t) was estimated using a SCS curve-number method (Soil Conservation Service, 1985). See section "Estimation of Direct Overland Surface Runoff" later in this report.

EV_i^t = water-surface evaporation of pond i during time period t was estimated by:

$$EV_i^t = e_i^t A_i^t, \quad (19)$$

in which e_i^t was the water-surface evaporation coefficient for time period t , and A_i^t was the pond water-surface area at the beginning of time period t . If the evaporation rate a_i^t was in inches per day and the water-surface area A_i^t was in acres, then total surface evaporation EV_i^t , in acre-feet, during time period t with time length of Δt days was calculated as follows:

$$EV_i^t = 0.0833 a_i^t A_i^t \Delta t. \quad (20)$$

SP_i^t = seepage through the pond bottom, in acre-feet. Seepage was estimated using Darcy's equation:

$$SP_i^t = K_i \frac{Zsw_i^t - Zgw_i^t}{d_i} A_i^t \Delta t, \quad (21)$$

in which K_i was the bottom hydraulic conductivity of pond i , in feet per day; Zsw_i^t was the surface-water elevation, in feet above sea level; Zgw_i^t was the ground-water elevation below the pond bottom, in feet above sea level; and d_i was the pond-bottom thickness, in feet.

W_i^t = water withdrawal during time period t , in acre-feet.

S_i^t = pond storage at the end of time period t , in acre-feet.

S_i^{t-1} = pond storage at the beginning of time period t , in acre-feet.

Substituting equations 8 and 15 into equation 16 gave:

$$\sum_j (OQN_{ji}^t + OQU_{ji}^t - OQL_{ji}^t) - \sum_j (IQN_{ij}^t + IQU_{ij}^t - IQL_{ij}^t) + (I_i^t + RN_i^t - SP_i^t - W_i^t - (RC_i^t + SU_i^t) - S_i^{t-1}). \quad (22)$$

Rearranging equation 22 gave:

$$\begin{aligned} & \sum_j (OQN_{ji}^t + OQU_{ji}^t - OQL_{ji}^t) - \sum_j (IQN_{ij}^t + IQU_{ij}^t - IQL_{ij}^t) - SU_i^t + SL_i^t \\ & + (S_i^{t-1} + I_i^t + RN_i^t - RC_i^t - SP_i^t - EV_i^t - W_i^t) = 0. \end{aligned} \quad (23)$$

At the beginning of time t , the values of S_i^{t-1} , I_i^t , RN_i^t , RC_i^t , SP_i^t , EV_i^t , and W_i^t were known or could be estimated using previous time-period data. If $NV_i^t = S_i^{t-1} + I_i^t + RN_i^t - RC_i^t - SP_i^t - EV_i^t - W_i^t$, the pond water-balance equation became:

$$\begin{aligned} & \sum_j (OQN_{ji}^t + OQU_{ji}^t - OQL_{ji}^t) - \sum_j (IQN_{ij}^t + IQU_{ij}^t - IQL_{ij}^t) - SU_i^t + SL_i^t + NV_i^t = 0. \end{aligned} \quad (24)$$

Each term in equation 24 was represented by flow through a distinct arc in the linear-network flow model. Among these arcs, the term NV_i^t was simply called a net-value arc (NV). Upper storage deviation arcs (SU), lower storage deviation arcs (SL), and NV arcs were connected to a sink/source node (fig. 8). The direction of SU arcs was from node i to the sink/source node. The direction of SL arcs was from the sink/source node to node i . The direction of NV arcs depended on the sign of the value of NV_i^t . If the value of NV_i^t was positive, the direction of the NV arc was from the sink/source node to the pond node i , and the reverse was true if the value of NV_i^t was negative (fig. 8).

General Node

A general node was designated where the calculation of water balance was needed (for example, at joints of canals). The difference between a pond node and a general node was that there was no water storage associated with a general node. The water balance at general node i during time t was given by:

$$\sum_j OQ_{ji}^t - \sum_j IQ_{ij}^t + I_i^t - W_i^t = 0, \quad (25)$$

where

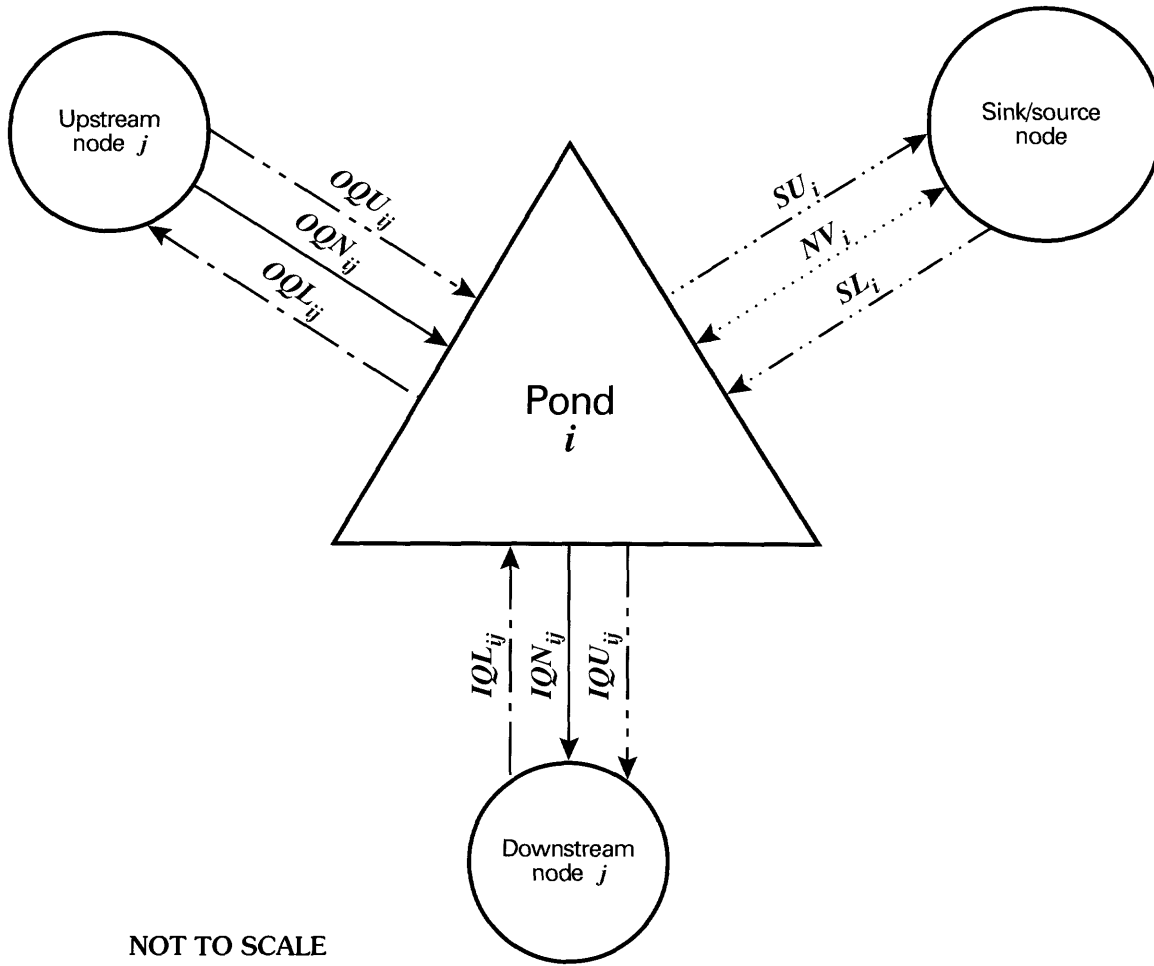
OQ_{ji}^t = inflow from upstream node j to node i during time period t ;

IQ_{ij}^t = outflow from a general node i to the downstream node j during time period t ;

I_i^t = local net incremental inflow to node i , such as surface runoff; and

W_i^t = water withdrawal at node i during time t . W_i^t was expressed as follows:

$$W_i^t = TR_i^t - DW_i^t, \quad (26)$$



EXPLANATION

$\overline{QN_{ij}}$	→ Normal flow arc	$\overline{SU_i}$	→ Upper storage deviation arc
$\overline{QU_{ij}}$	→ Upper flow arc	$\overline{NV_i}$	→ Pond net-value arc
$\overline{QL_{ij}}$	→ Lower flow arc	$\overline{SL_i}$	→ Lower storage deviation arc

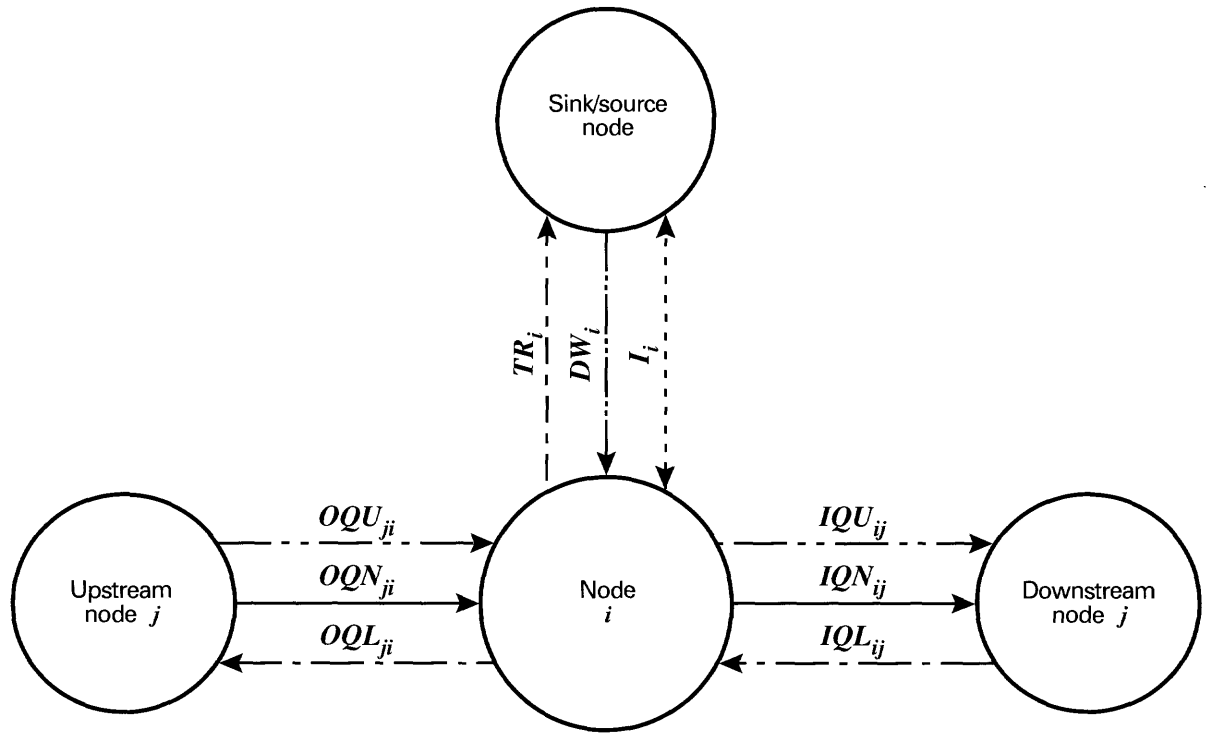
Figure 8. Arc-node representation for pond-storage routing.

in which TR_i^t was the target water-withdrawal from node i during time period t , and DW_i^t was the water withdrawal deviation for node i during time period t ,
 $0 \leq DW_i^t \leq TR_i^t$.

Substituting equations 8 and 26 into equation 25 gave:

$$\sum_j (oQN_{ji}^t + oQU_{ji}^t - oQL_{ji}^t) - \sum_j (IQL_{ij}^t + IQU_{ij}^t - IQN_{ij}^t) + I_i^t - (TR_i^t - DW_i^t) = 0. \quad (27)$$

Equation 27 is consistent with equation 4, and each term of equation 27 was represented by flow through a distinct arc (fig. 9). Among flows through these arcs, flows I_i^t and TR_i^t were known at the beginning of time t . The direction of the I_i^t arc depended on the sign of the value I_i^t . If it was positive, then the arc was directed toward the water demand node from the sink/source node. The reverse was true for a negative I_i^t . Because I_i^t and TR_i^t were known, the penalty coefficients of I_i^t and TR_i^t arcs were assigned to be zero.



NOT TO SCALE

EXPLANATION

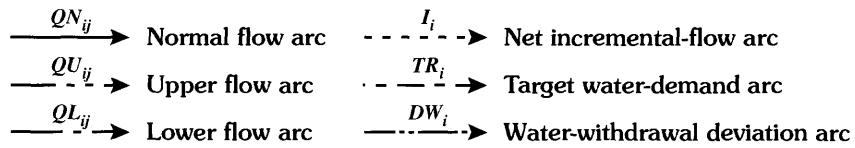


Figure 9. Arc-node representation for a general node.

Sink/Source Node

The sink/source node mentioned in previous sections was an introduced node that made it possible to form a closed-loop network. As a sink, this node accounted for: (1) flows from canal water loss (seepage and evaporation) and final water storage (fig. 7B), (2) flows from the storage deviation above the rule curve (fig. 8), (3) canal flows at the downstream end of the system, and (4) water withdrawal from a node (fig. 9). As a source, the sink/source node accounted for: (1) flows for the canal initial storages (fig. 7B), (2) flows for the storage deviations below the rule curves (fig. 8), (3) net inflows to ponds (fig. 8), and (4) net incremental flows to general nodes (fig. 9). The water balance for canal reaches, pond nodes, and general nodes guaranteed that mass conservation at a sink/source node was satisfied.

Linear-Network Optimization Flow Model

The linear-network flow model given by equations 3–5 was rewritten for the operation of canals and control ponds as follows:

Minimize

$$\sum_i \left(C_i^u S U_i + C_i^l S L_i \right)^t + \sum_i \sum_j \left(C_{ij}^n Q N_{ij} + C_{ij}^u Q U_{ij} - C_{ij}^l Q L_{ij} \right)^t + \sum_i C_i^w D W_i^t, \quad (28)$$

subject to

$$\sum_j (OQN_{ji} + OQU_{ji} - OQL_{ji})^t - \quad (29)$$

$$\sum_j (IQN_{ij} + IQU_{ij} - IQL_{ij}) - (SU_i - SL_i)^t + NV_i^t = 0$$

for all pond nodes i ,

$$\sum_j (OQN_{ji}^t + OQU_{ji}^t - OQL_{ji}^t) - \quad (30)$$

$$\sum_j (IQN_{ij}^t + IQU_{ij}^t - IQL_{ij}^t) + I_i^t - (TR_i^t - DW_i^t) = 0$$

for all general nodes i ,

$$(IQN_{ij}^t + IQU_{ij}^t - IQL_{ij}^t) - (OQN_{ji}^t + OQU_{ji}^t - OQL_{ji}^t) - \quad (31)$$

$$EV_i^t - SP_i^t = S_i^t - S_i^{t-1}$$

for canal flow arcs (i,j) ,

$$0 \leq SU_i^t \leq \overline{SU}_i, \quad (32)$$

$$0 \leq SL_i^t \leq \overline{SL}_i, \quad (33)$$

$$0 \leq \underline{QN}_{ij} \leq QN_{ij}^t \leq \overline{QN}_{ij}, \quad (34)$$

$$0 \leq \underline{QU}_i \leq QU_i^t \leq \overline{QU}_i, \quad (35)$$

$$0 \leq \underline{QL}_i \leq QL_i^t \leq \overline{QL}_i, \quad (36)$$

$$0 \leq \underline{DW}_i \leq DW_i^t \leq \overline{DW}_i, \quad (37)$$

where C_i^u and C_i^l were the penalty coefficients for cost per unit; and the upper bars and lower bars in equations 32–37 were upper and lower flow boundaries of an associated arc.

Water storage deviated from the rule curve at pond node i for the upper zone and lower zone, respectively. C_{ij}^n , C_{ij}^u , and C_{ij}^l denoted the penalty coefficients for cost per unit flow in canal ij for the normal, upper, and lower flow zones, respectively.

The linear-network optimization flow model given by equations 28–37 was a typical minimum-cost flow problem in network analysis. Several algorithms exist for solving a minimum-cost flow problem. One of the

algorithms, called the out-of-kilter algorithm (Fulkerson, 1961; Bazaraa and others, 1990), was used in developing the computer program called OPONDS (the optimal Operation of a system of PONDS) developed for this study (see Appendices for the description and listing of the computer program).

Model Supplements

In the following section, the methods used in OPONDS to estimate canal water storage, canal-flow transmission loss, surface runoff, and flow through hydraulic structures are described.

Estimation of Canal Water Storage

The Muskingum's method (McCuen, 1989) was used to estimate canal storage in this study. The method assumes that, for given a reach, canal storage (S) can be expressed in terms of inflow and outflow rates as follows:

$$S = K[xI + (1-x)O], \quad (38)$$

where

K = the storage constant defined by the ratio of storage to discharge. The storage constant K has the dimension of time; therefore, K is often called traveltime. The coefficients of K and x are generally determined using historical discharge data (Wu and others, 1985; Chow and others, 1988; McCuen, 1989);

x = the dimensionless weighting factor for the storage effect of inflow and outflow. The value of x is usually between 0 and 0.5;

I = inflow rate; and

O = outflow rate.

Estimation of Canal-Flow Transmission Losses

To estimate canal-flow transmission losses to an aquifer, two approximation methods were included in the computer program OPONDS. The first one was based on Darcy's equation given by:

$$q = K_b \frac{(Z_{sw} - Z_{gw})}{d} LB, \quad (39)$$

where

- q = canal seepage rate along canal reach;
- K_b = canal-bottom hydraulic conductivity;
- Z_{sw} = average canal surface-water elevation, which is the average water depth (h) plus the canal-bottom elevation (Z_b);
- Z_{gw} = average ground-water elevation below the canal bottom, which is the average value along the canal;
- d = average canal-bottom thickness;
- L = canal-reach length; and
- B = canal water-surface width.

If a canal gained water from the aquifer, the seepage (q) in equation 39 was a negative number. If the ground-water elevation Z_{gw} was lower than the average canal-bottom elevation, then the seepage rate, q , was simplified as:

$$q = K_b h L B, \quad (40)$$

where h was the average water depth in a canal reach.

The water depth h was estimated iteratively using Manning's equation (Henderson, 1966):

$$v = \frac{1.486 R^{2/3} J^{1/2}}{n}, \quad (41)$$

in which v was the average velocity, in feet per second [$v = Q / A(h)$]; R was the hydraulic radius [$R = A(h) / P(h)$], in feet; $A(h)$ was the cross-section area, in square feet; $P(h)$ was the wetted perimeter in feet; J was the hydraulic slope; and n was the roughness coefficient, which is dependent on canal bottom materials.

The second approximation method (Jordan, 1977) assumed that the rate of canal-flow transmission loss at any point was proportional to the flow at that point and that the canal characteristics were uniform for a given reach; that is,

$$\frac{dQ_x}{dx} = -kQ_x, \quad (42)$$

where x was the distance coordinate, and k was the transmission loss per unit length of canal [$1/L$] and was simply called transmission loss coefficient.

For a given canal reach of length L , the transmission loss then was estimated by:

$$q = (1.0 - e^{-kL})IQ = cIQ, \quad (43)$$

where IQ was the inflow entering a canal, and c was a transmission loss rate for a given canal reach of length L and was estimated using seepage test data with a least-squares technique or other techniques.

Estimation of Direct Overland Surface Runoff

The Soil Conservation Service (1985) developed a method for estimating direct overland surface runoff depth from precipitation. The runoff depth Q generated by precipitation P was given by:

$$Q = \frac{(P - 0.2S)^2}{P + 0.8S}, \quad (44)$$

where S was the potential maximum retention (the amount of rain not converted to runoff after runoff begins) given by:

$$S = \frac{1000}{CN} - 10, \quad (45)$$

in which CN was the SCS curve number. The SCS curve number (CN) is an index that represents the combination of hydrologic soil group and land use. CN is a function of three factors—soil group, land-cover type, and antecedent moisture conditions. The range of CN is from 0 to 100. The curve number for average antecedent soil-moisture conditions (AMC II) can be interpreted for given soil properties and land-cover type (Soil Conservation Service, 1985; McCuen, 1989). For dry conditions (AMC I) and wet conditions (AMC III), equivalent curve numbers can be computed using the following equations (Chow and others, 1988):

$$CN(I) = \frac{4.2CN(II)}{10 - 0.058CN(II)}, \quad (46)$$

and

$$CN(III) = \frac{23CN(II)}{10 + 0.13CN(II)}, \quad (47)$$

where $CN(I)$, $CN(II)$, and $CN(III)$ are the curve numbers for the dry, average, and wet conditions, respectively.

The range of antecedent moisture conditions for each class is shown in table 5 (Chow and others, 1988). The SCS curve numbers for average soil-moisture conditions for the Quivira National Wildlife Refuge are summarized in table 4 in an earlier section.

Flow Through Hydraulic Structures

Water releases from ponds are through hydraulic control structures. The amount of water release depends on several factors, such as the pond water level, hydraulic-structure types and sizes, and the operation of structures. In the following sections, flows through four types of structures are discussed.

Flow Over Sharp-Crested Weir

A sharp-crested weir consists of a vertical plate mounted at right angles to the flow and having a sharp-edged crest (fig. 10A). The discharge equation is:

$$Q = mb\sqrt{2g}H_o^{1.5}, \quad (48)$$

where

Q = discharge over weir, in cubic feet per second;

m = discharge coefficient, which is dimensionless;

b = weir length, in feet;

H_o = total energy head ($= H + v_o^2 / 2g$), in feet. If approaching velocity $v_o \approx 0$, then $H_o = H$, where H is the static water head on a weir, referred to as the weir crest; and

g = gravity acceleration ($= 32.17 \text{ ft/s}^2$).

The discharge coefficient (m) for free discharge is a function of certain dimensionless ratios that describe the geometry of the canal and the weir (Hulsing, 1967). One simple expression for free discharge with no side contraction is (Henderson, 1966):

$$m = 0.4073 + 0.0533 (H/P), \text{ where } 0 < H/P < 5, \quad (49)$$

in which P is the weir height (fig. 10A).

Flow Under Gate on Broad-Crested Weir

Flow under a vertical sluice gate on a broad-crested weir (fig. 10B) was calculated by:

Table 5. Classification of antecedent soil-moisture conditions (AMC) for SCS curve-number method of rainfall abstractions

[From Chow and others, 1988]

AMC	Total 5-day antecedent rainfall (inches)	
	Dormant season	Growing season
I	Less than 0.5	Less than 1.4
II	0.5–1.1	1.4–3.1
III	More than 1.1	More than 3.1

$$Q = mbe\sqrt{2gH_o}, \quad (50)$$

where e was the gate opening height, and the other terms had the same definitions as in equation 48.

If $e/H > 0.65$, flow was not affected by the gate. The discharge coefficient for the free outflow under the gate depended on the relative gate opening height (e/H) and was approximated by (Swamee, 1992):

$$m = 0.611 \left(\frac{1 - \frac{e}{H}}{1 + 15 \frac{e}{H}} \right)^{0.072}, \quad (51)$$

where $e/H < 0.65$.

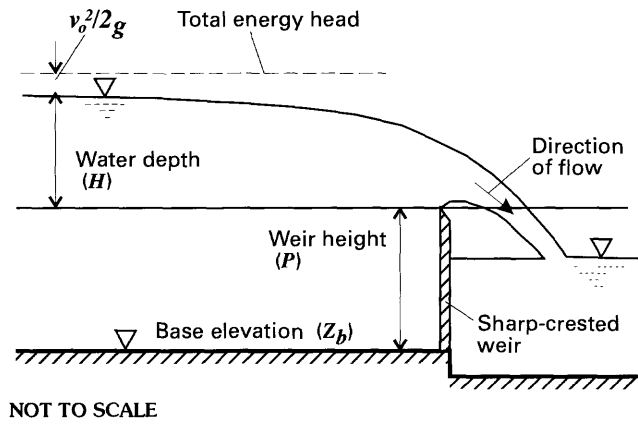
Flow Under Gate on Spillway

Flow under a gate on a spillway was calculated by:

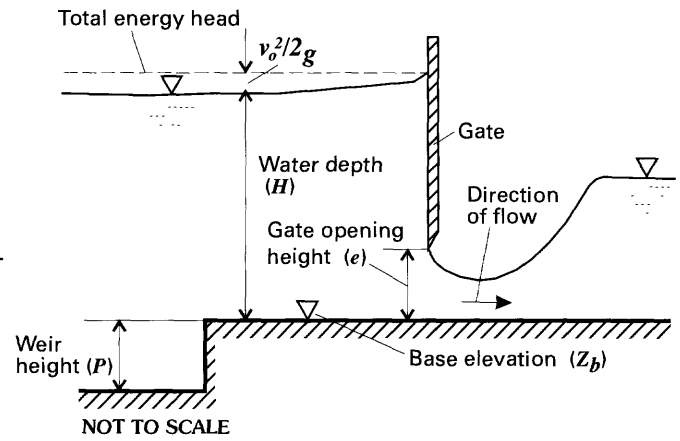
$$Q = mbe\sqrt{2gH_o}. \quad (52)$$

The definition of variables in equation 52 is the same as equation 50. The discharge coefficient (m) for a standard spillway depended not only on the relative gate opening height (e/H) but also on the design water head (H_d) and design discharge coefficient (m_d) (U. S. Army Engineer Waterways Experiments Station, 1972). In the case that design water head and design coefficient were not available, the free outfall flow with a flat gate and sharp-crested edge of the gate facing downstream (fig. 10C) was approximated using the following equation (Chengdu Science and Technology University, 1979):

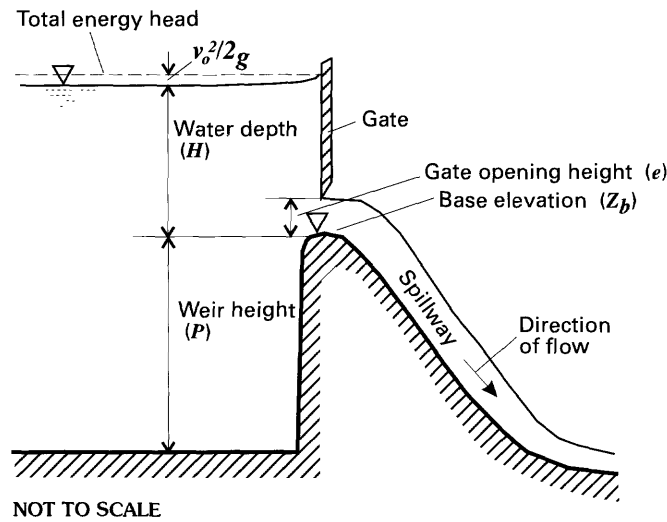
$$m = 0.65 - 0.186 (e/H). \quad (53)$$



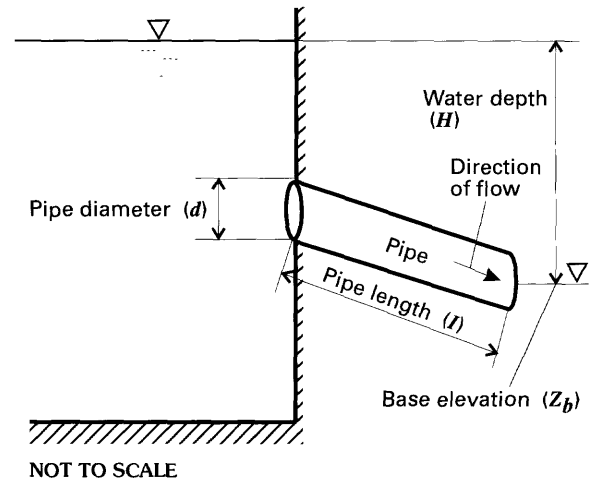
A. Sharp-crested weir.



B. Gate on broad-crested weir.



C. Gate on spillway.



D. Pipe.

Figure 10. Flow through hydraulic structures.

Pipe Outflow

For free flow through a pipe (fig. 10D), the discharge was estimated by:

$$Q = mA\sqrt{2gH} \quad , \quad (54)$$

where A was the area of cross section of the pipe, H was the water depth above the water outlet, and the discharge coefficient (m) was given

by (Chengdu Science and Technology University, 1979):

$$m = \frac{1}{\sqrt{1 + \lambda \frac{l}{d} + \Sigma \zeta}} \quad , \quad (55)$$

where l was the length of the pipe, d was the diameter of the pipe, λ was the pipe friction coefficient that was determined by pipe

materials, and ζ was the entrance loss coefficient that was determined by the shape of the entrance.

SIMULATION OF CANAL AND CONTROL-POND OPERATION FOR 1996

From June 11 through December 11, 1996, personnel at the Quivira National Wildlife Refuge measured water-surface levels about four or five times a month for most ponds. Streamflow discharges at the Rattlesnake Creek near Zenith and Raymond streamflow-gaging stations were measured continuously by USGS; however, the discharges in canals on the refuge were not measured. A simulation was conducted to determine the operation of canals and control ponds under 1996 conditions. The major objective was to determine the operation policy for canals and control ponds on the refuge so that the simulated pond water levels would match well with the measured water levels. The basic approach was to use the measured pond water levels as the pond rule curve, to set up pond zoning and the priority relations of control ponds, to determine pond releases to canals or other ponds to satisfy the measured discharges of Rattlesnake Creek near Raymond, and to examine simulated water levels for those ponds without water-level measurements. In the following sections, the data and the related necessary assumptions needed to conduct the simulation are discussed, the operation policy of ponds is discussed, and the simulation results are presented.

Data Preparation

In this section, data needed for the simulation are discussed. Measurement data were used if available. If some data were not available, reasonable values were estimated from other sources.

Precipitation

The amount of precipitation directly affects the surface runoff to ponds. Daily precipitation measured at the refuge headquarters from June 11 through December 11, 1996, is shown in figure 11A (Marios Sophocleous, Kansas Geological Survey, written commun., 1997). The total amount of precipitation for the period was 13.96 in.

Water-Surface Evaporation

The daily potential evapotranspiration (PET) (Marios Sophocleous, Kansas Geological Survey, written commun., 1997) is shown in figure 11B. It was assumed that the daily water-surface evaporation rates on the refuge were the same as the corresponding daily potential evapotranspiration. The total water-surface evaporation for the simulation period was 25.21 in.

Canal Discharge

Discharge for Rattlesnake Creek measured at the USGS streamflow-gaging stations near Zenith and Raymond from June 11 through December 11, 1996, is shown in figure 11C. The mean daily discharge rates for the simulation period were 48.72 and 47.73 ft³/s for the Zenith and Raymond stations, respectively.

For this simulation, the daily mean discharges observed at the USGS Zenith station were used as water supply from Rattlesnake Creek to Little Salt Marsh. The daily mean discharges observed at the USGS Raymond station were used as the required stream outflow from the refuge through Rattlesnake Creek.

Canal-Flow Transmission Losses

Flow transmission losses from canals on the refuge were difficult to estimate. Personnel from the refuge did four seepage tests (table 6) along a 15,129-ft reach of Rattlesnake Creek downstream from Little Salt Marsh during 1996 (see fig. 2). Applying the least-squares method to equation 43, the estimated transmission loss coefficient (k) (equation 42) was equal to $9.16 \times 10^{-6} \text{ ft}^{-1}$. Due to a lack of data for the remaining canals on the refuge, this value of k was used for the estimation of flow transmission loss rate c (equation 43) for all canals south of the RC Canal. Because canals north of the RC Canal are located in the ground-water discharge area, no canal-flow transmission losses occurred for these canals. The ground-water discharge to these canals was included in discharge to ponds (see table 2).

Ground-Water Discharge to Ponds

Ground-water discharge to ponds on the refuge during the simulation period was not available. Instead, ground-water discharges to ponds based on information provided by Marios Sophocleous (Kansas

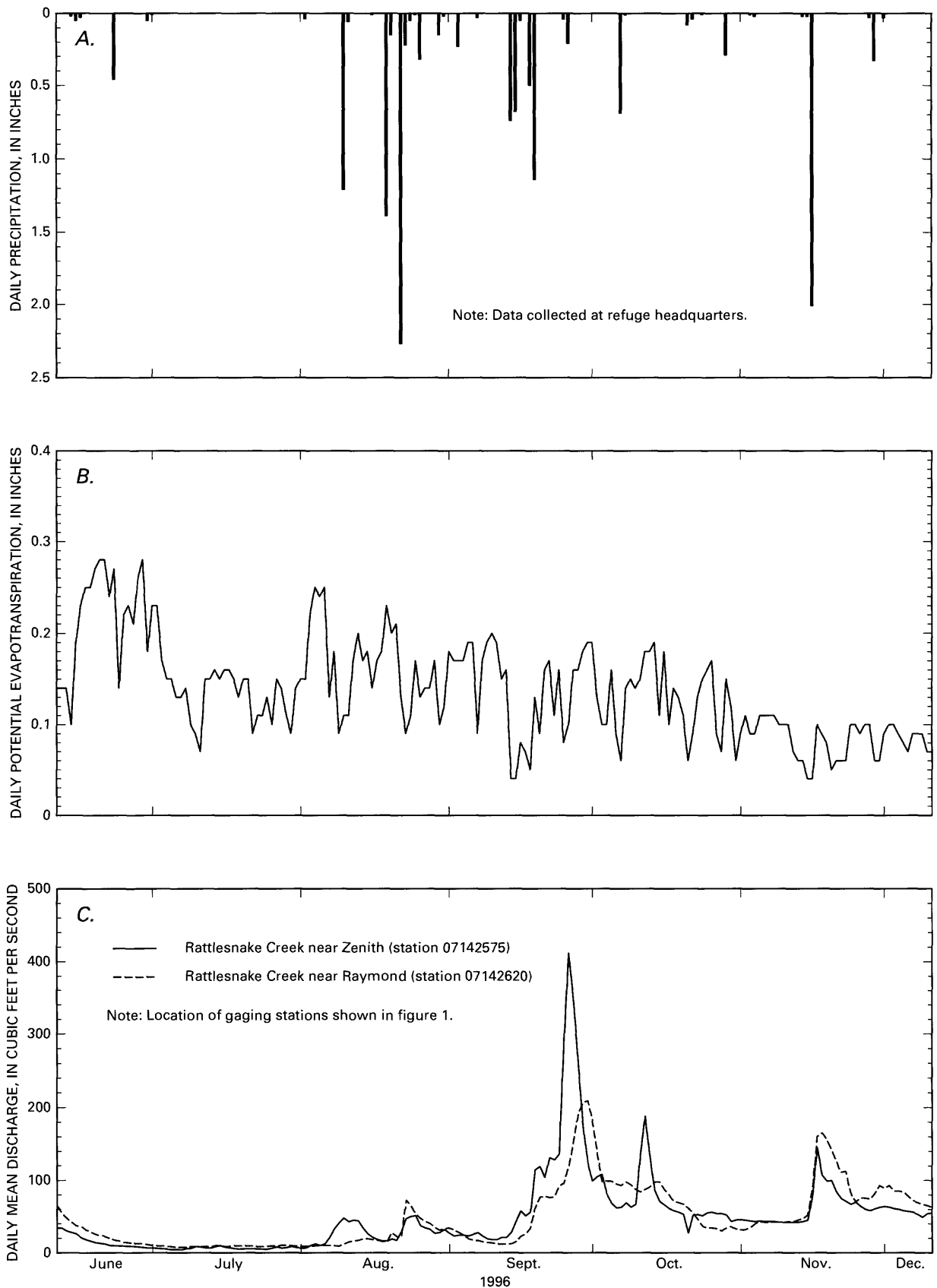


Figure 11. (A) Daily precipitation, (B) daily potential evapotranspiration, (C) daily mean discharge for Rattlesnake Creek, June 11 through December 11, 1996. Precipitation and potential evapotranspiration data from the Kansas Geological Survey (Marios Sophocleous, written commun., 1997).

Table 6. Results of seepage tests along Rattlesnake Creek, 1996, at Quivira National Wildlife Refuge

[Data from U.S. Fish and Wildlife Service (Megan Estep-Johnston, written commun., 1996). ft³/s, cubic feet per second]

Date (month/day/ year)	Discharge (ft ³ /s)	
	Upstream test site 1 (fig. 2)	Downstream test site 2 (fig. 2)
06/26/96	6.93	6.15
07/17/96	8.98	7.46
07/24/96	3.13	2.92
09/09/96	5.52	5.12

Geological Survey, written commun., 1997) for 1994 were used (table 2).

Pond Water-Surface Elevations

Water-surface elevations for selected control ponds were measured during the simulation period. Table 7 lists the ponds with measured water-surface elevations, the number of measurements, and the minimum and maximum water-surface elevations for the ponds. Because the water levels may be at the bottom of ponds or above the staff gage at ponds, the number of observations of water-surface elevations listed in table 7 may be different than the number of measurements listed. The difference between the number of observations and the number of measured elevations is the number of records without measurements. Those water-surface elevations observed outside the range of measurement on pond staff gages were treated accordingly as the pond-bottom elevation or the full-pond elevation in this simulation.

Pond Zoning and Operating Policy

Each control pond was divided into four storage zones—inactive zone, lower zone, upper zone, and extended upper zone as shown in figure 5. Target water levels (rule curves) were set at the top of the lower zone. For ponds with measured water-surface elevations (table 7), the measured water elevations were used as their rule curves, which indicates that the rule curves changed during the simulation period and so did the storage capacity of lower and upper zones. For those ponds without measured water levels, the rule curves were set at 95 percent of their corresponding

full-pond storage capacities. The capacity of the inactive zone of a pond was set at 20 percent of full-pond storage capacity (selected in consultation with the U.S. Fish and Wildlife Service). Some of the rule curves for some ponds with measured water levels were located in the inactive zone during the simulation. In this case, the top boundary of the inactive zone capacity was set at the rule curve, and the capacity of the lower zone was set to zero. The top boundary of the upper zone was set at the full-pond elevation. The top boundary of the extended upper zone was set 0.5 ft higher than its full-pond elevation. For ponds whose maximum measured water levels were higher than the full-pond elevation plus 0.5 ft, the top boundaries of the extended upper zone were set at the maximum measured water level. Pond zoning expressed as pond storage is summarized in table 8.

To operate the system of canals and control ponds on the refuge, it was necessary to establish the priority of the ponds. Because Little Salt Marsh (water unit 5), which is supplied by Rattlesnake Creek, serves as the principal water-storage unit for the entire refuge, the highest operational priorities were given to its storage zones. Water units (75, 78, 80, 81, and 83, see fig. 2) in the north part of the refuge were given the lowest operational priorities because these ponds are at the downstream end of the refuge and control less drainage area. The remaining ponds were given priorities in between the highest and the lowest priorities. Under this operating policy, water to satisfy the downstream water requirements was released first (1) from the lowest priority ponds when water levels at the highest priority ponds were below the rule curve so that high-priority pond water levels were as close as possible to their rule curves, or (2) from the highest priority ponds when their water levels were higher than the rule curves so that the water levels would decrease to as close to their rule curves as possible. To represent priorities of ponds, different penalty coefficients were assigned to each of the storage zones of the ponds. The higher the priority, the higher the penalty coefficient assigned. It should be noted that the relative magnitudes, not the absolute values, of the penalty coefficients determined the optimal operation of the system. Different combinations of assigned values of penalty coefficients were tested for the control ponds on the refuge. Typical values of penalty coefficients used in this simulation are summarized in table 8.

Because there are no flow requirements such as minimum-required flow for canals on the refuge, there

Table 7. Summary of water-surface elevations for selected ponds at Quivira National Wildlife Refuge, June 11 through December 11, 1996

[Data from U.S. Fish Wildlife Service, written commun., 1997]

Water-unit number (fig. 2)	Number of observations	Number of measurements	Maximum measured elevation (feet above sea level)	Minimum measured elevation (feet above sea level)
5	30	30	1,783.30	1,782.62
7	30	23	1,778.96	1,777.33
10A	30	16	1,778.67	1,777.03
10B	30	29	1,778.89	1,777.32
10C	32	32	1,774.86	1,773.22
11	32	14	1,773.91	1,771.95
14A	30	30	1,777.92	1,776.30
14B	30	30	1,777.36	1,774.90
16	30	28	1,774.46	1,772.72
20A	29	29	1,770.84	1,769.56
21	29	17	1,769.09	1,767.00
22	29	29	1,767.17	1,764.91
23	29	27	1,764.78	1,763.02
24	30	30	1,770.46	1,769.61
25	33	20	1,766.92	1,763.16
26	28	28	1,762.06	1,760.14
28	30	17	1,767.81	1,764.10
29	30	26	1,761.83	1,757.20
30	30	17	1,760.01	1,756.48
40	29	22	1,742.59	1,738.58
48	29	25	1,754.28	1,750.88
49	29	29	1,754.13	1,750.25
58	30	30	1,740.90	1,739.59
61	29	29	1,743.89	1,742.54
62	29	29	1,742.64	1,739.55
63	29	29	1,740.73	1,739.17
75	29	7	1,740.17	1,739.55

Table 8. Initial storage, zoning, and penalty coefficients assigned to control ponds at Quivira National Wildlife Refuge, June 11 through December 11, 1996

Water-unit number (fig. 2)	initial storage (acre-feet)	Upper boundary of extended upper zone (acre-feet)	Penalty coefficient for extended upper zone	Upper boundary of upper zone (acre-feet)	Penalty coefficient for upper zone	Lower boundary of lower zone (acre-feet)	Penalty coefficient for lower zone	Lower boundary of inactive zone (acre-feet)	Penalty coefficient for inactive zone
5	1,988.26	2,312.18	2,000	1,866.00	1,500	373.20	1,500	1.00	5,000
7	39.72	72.90	2,000	40.00	1,500	8.00	1,000	1.00	2,000
10A	145.48	180.30	2,000	145.00	1,500	29.00	1,000	1.00	2,000
10B	145.48	180.30	2,000	145.00	1,500	29.00	1,000	1.00	2,000
10C	19.54	21.81	2,000	13.00	1,500	2.60	1,000	1.00	2,000
11	388.37	440.07	2,000	338.00	1,500	67.60	1,000	1.00	2,000
14A	161.70	242.20	2,000	196.00	1,500	39.20	1,000	1.00	2,000
14B	93.40	185.74	2,000	96.00	1,500	19.20	1,000	1.00	2,000
14C	15.51	19.07	2,000	16.00	500	3.20	500	1.00	2,000
16	62.67	96.07	2,000	80.00	1,500	16.00	1,000	1.00	2,000
20A	163.88	268.01	2,000	195.00	1,500	39.00	1,000	1.00	2,000
20B	163.88	268.01	2,000	195.00	1,500	39.00	1,000	1.00	2,000
21	34.34	96.62	2,000	81.00	1,500	16.20	1,000	1.00	2,000
22	2.30	18.81	2,000	13.00	1,500	2.60	1,000	1.00	2,000
23	15.41	19.66	2,000	15.00	1,500	3.00	1,000	1.00	2,000
24	132.55	139.01	2,000	35.00	1,500	7.00	1,000	1.00	2,000
25	18.00	344.05	2,000	296.00	1,500	59.20	1,000	1.00	2,000
26	91.48	142.39	2,000	111.00	1,500	22.20	1,000	1.00	2,000
28	6.11	198.82	2,000	153.00	1,500	30.60	1,000	1.00	2,000
29	0.20	124.51	2,000	91.00	1,500	18.20	1,000	1.00	2,000
30	2.82	161.64	2,000	119.00	1,500	23.80	1,000	1.00	2,000
40	55.91	83.19	2,000	66.00	1,500	13.20	1,000	1.00	2,000
48	3.94	161.19	2,000	113.00	1,500	22.60	1,000	1.00	2,000
49	51.63	209.05	2,000	159.00	1,500	31.80	1,000	1.00	2,000
57	212.22	280.74	2,000	212.00	1,500	42.40	1,000	1.00	2,000

Table 8. Initial storage, zoning, and penalty coefficients assigned to control ponds at Quivira National Wildlife Refuge, June 11 through December 11, 1996—Continued

Water-unit number (fig. 2)	Initial storage (acre-feet)	Upper boundary of extended upper zone (acre-feet)	Penalty coefficient for extended upper zone	Upper boundary of upper zone (acre-feet)	Penalty coefficient for upper zone	Lower boundary of lower zone (acre-feet)	Penalty coefficient for lower zone	Lower boundary of inactive zone (acre-feet)	Penalty coefficient for inactive zone
58	146.39	302.82	2,000	251.00	1,500	50.20	1,000	1.00	2,000
61	212.80	613.17	2,000	498.00	1,500	99.60	1,000	1.00	2,000
62	48.58	145.00	2,000	120.00	1,500	24.00	1,000	1.00	2,000
63	268.98	419.01	2,000	339.00	1,500	67.80	1,000	1.00	2,000
75	2,445.85	3,490.32	1,000	2,446.00	500	489.20	500	1.00	2,000
78	5,270.43	6,091.37	1,000	5,270.00	500	1,054.00	500	1.00	2,000
80	355.20	474.34	1,000	355.00	500	71.00	500	1.00	2,000
81	25.31	60.68	1,000	25.00	500	5.00	500	1.00	2,000
83	314.34	419.31	1,000	314.00	750	62.80	750	1.00	2,000

was only one flow zone for canals designated in this simulation. It was assumed that flow through a canal reach ranged in magnitude from zero to the full capacity of the canal. Because of the complexity of the canal flow network on the refuge, flows could reach the same location through different routes of canals. Different penalty coefficients were assigned to the flow zones of canals so that the most efficient route could be determined by minimizing the total penalty applied to canal flows. However, costs of transporting water through canals were not available. Because Rattlesnake Creek is used as the major route to distribute water to the refuge and because other canals are used only when necessary, flows through Rattlesnake Creek and canals downstream from control ponds were assigned penalty coefficients of zero, and the remaining canals were assigned nonzero penalty coefficients as shown in table 9 (see figures 2 and 4 for nodal names, location, and flow network).

Results

The simulation of canal and control-pond operation at the Quivira National Wildlife Refuge for June 11 through December 11, 1996, was conducted using the following specifications for pond zoning, operating policy, and canal outflow from the refuge: (1)

four storage zones for each pond, with the inactive storage of 20 percent of full-pond storage capacity; (2) rule curves set at the measured water levels for ponds with measurements, otherwise at 95 percent of full-pond storage capacity; (3) initial storage in ponds interpreted from the water levels measured on June 10, 1996, for ponds with measurements, otherwise set at 95 percent of full-pond storage capacity; and (4) outflows from the refuge through Rattlesnake Creek near the USGS streamflow-gaging station near Raymond equal to the observed discharges at the streamflow-gaging station (fig. 11C).

Figures 12A–D show the water-budget components simulated for the operation of water unit 5. Similar figures also can be generated for other control ponds. Inflows shown in figure 12A are upstream inflows from Rattlesnake Creek, which are equal to the discharges observed at the USGS streamflow-gaging station near Zenith. Total downstream releases shown in figure 12C are the summations of releases to all downstream nodes (water units 7 and 10A, and nodes C–2 and JE–1, see figure 4). Ground-water seepage during the simulation period shown in figure 12B is almost the same for the whole simulation period (the values were estimated for 1994, see table 2). Figure 12E shows the simulated and measured water stages and depths. From July 9 to August 8, even though there were no releases from the pond, the simulated water stages were lower than the

Table 9. Penalty coefficients for canal flows at Quivira National Wildlife Refuge, June 11 through December 11, 1996

Canal			Canal			Canal		
From-node name (fig. 4)	To-node name (fig. 4)	Penalty coefficient	From-node name (fig. 4)	To-node name (fig. 4)	Penalty coefficient	From-node name (fig. 4)	To-node name (fig. 4)	Penalty coefficient
Zenith	Unit 5	0	Unit 24	Unit 21	10	Unit 61	JE-4	0
Unit 5	Unit 7	10	Unit 24	Unit 20B	1,000	Unit 63	JE-5	0
Unit 5	Unit 10A	10	Unit 25	JE-2	10	Unit 63	JE-6	0
Unit 5	JE-1	0	Unit 25	Unit 26	10	Unit 75	Unit 78	10
Unit 5	C-2	10	Unit 26	48E	10	Unit 78	Unit 81	10
Unit 7	Unit 10B	10	48E	Unit 48	0	Unit 81	Unit 80	10
Unit 10A	Unit 10B	10	48E	Unit 49	0	Unit 80	JN-1	0
Unit 10B	Unit 10C	10	Unit 28	Unit 29	10	Unit 83	JN-1	0
Unit 10C	Unit 11	10	Unit 29	Unit 30	10	JN-1	JE-7	0
Unit 11	SKSC ¹	1,250	Unit 30	WCE	10	DCC	DCF	100
C-2	F-1	0	WCE	Unit 48	0	DCC	SKSC ¹	5,000
C-2	D-1	0	WCE	RCD	0	DCF	40C	100
F-1	F-2	0	Unit 48	Unit 49	10	DCF	JE-0	100
F-1	Unit 14B	0	Unit 48	Unit 55	10	JE-0	37	100
D-1	Unit 14A	0	Unit 49	RCB	10	37	39	100
D-1	WCA	0	Unit 49	JE-3	10	39	JE-9	100
Unit 14A	UNIT 16	10	Unit 55	RCC	0	40C	Unit 40	0
Unit 14A	J14	10	Unit 55	RCF	0	40C	Unit 62	0
Unit 14B	J14	10	JE-1	Unit 24	0	Unit 40	JE-8	10
Unit 14B	Unit 20B	10	JE-2	JE-3	0	Unit 62	JE-5	10
J14	Unit 20A	0	JE-3	RCA	0	Unit 62	Unit 40	10
F-2	Unit 14C	0	RCA	RCB	0	JE-4	JE-5	0
F-2	Unit 20B	0	RCA	JE-4	0	JE-5	JE-6	0
Unit 14C	JE-1	0	RCB	Unit 61	0	JE-6	JE-7	0
Unit 16	WCA	10	RCB	RCC	0	JE-7	JE-8	0
WCA	Unit 28	10	RCC	RCF	0	JE-8	JE-9	0
Unit 20A	Unit 21	10	RCF	Unit 57	0	JE-9	JE-10	0
Unit 20B	Unit 20A	10	RCF	RCD	0	JE-10	RAYMOND	0
Unit 21	Unit 22	10	RCD	Unit 58	0	RAYMOND	SKSC ¹	0
Unit 22	Unit 23	10	Unit 57	Unit 78	10			
Unit 23	Unit 26	10	Unit 58	Unit 75	10			
Unit 24	Unit 25	10	Unit 58	Unit 78	10			
Unit 24	JE-2	0	Unit 61	Unit 57	10			
Unit 24	DCC	100	Unit 61	Unit 63	10			

¹Nodal name SKSC is used to specify that the end node of a canal is outside the refuge.

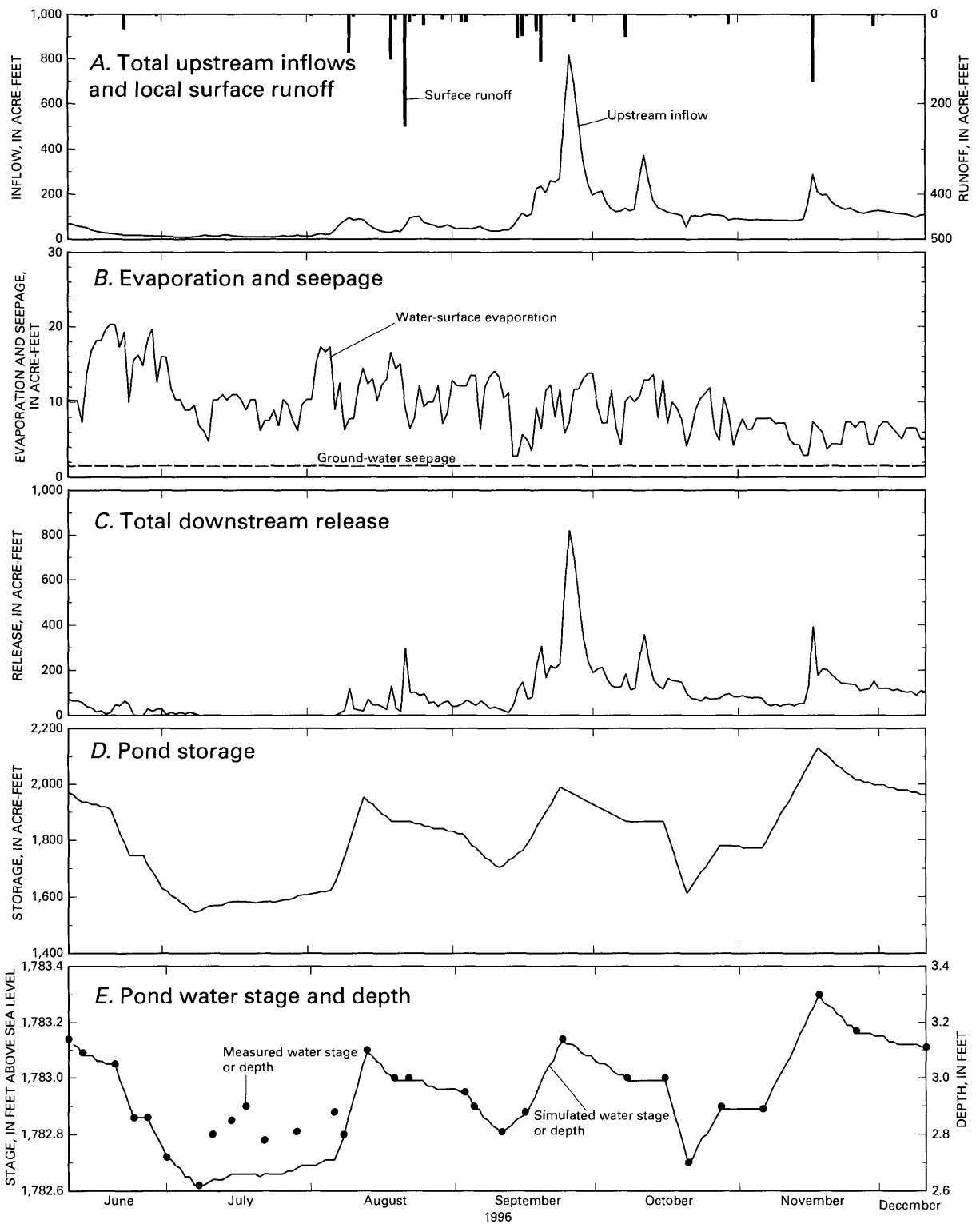


Figure 12. Water budget simulated for water unit 5, June 11 through December 11, 1996.

measured ones. The differences in stages were about 0.1 to 0.2 ft. The cause of these differences might be errors in reading stage and in estimating water-surface evaporation. The simulated water levels matched well with measured ones for the simulation period. The root mean square error (RMSE) between the simulated (\hat{Z}) and measured (Z) water levels was 0.08 ft for water unit 5 (see equation 56; n is the number of comparisons):

$$RMSE = \sqrt{\frac{\sum_{i=1}^n (\hat{Z}_i - Z)^2}{n}}. \quad (56)$$

Similar results were found for other ponds. The RMSEs for other ponds were less than 0.13 ft except water units 24 and 30 for which RMSEs were 0.49 and 0.40 ft, respectively. In other words, the current specification for pond zoning and rule curves simulated the operation of ponds well.

Table 10 summarizes water-budget components of the ponds for the entire simulation period. For each pond, the water-balance equation was:

$$\begin{aligned} & \text{Initial water storage} + \text{upstream inflow} + \\ & \text{surface runoff} - \text{water-surface evaporation} - \\ & \text{ground-water seepage} - \text{total downstream release} = \\ & \text{final storage}, \end{aligned} \quad (57)$$

where *upstream inflow* was the total inflow to a pond from upstream canals; *surface runoff* was the total runoff calculated using the measured precipitation data and SCS curve numbers; *water-surface evaporation* was the total surface-water evaporation loss, which was estimated in terms of the water-surface area and potential evapotranspiration coefficients; *ground-water seepage* was the total water loss to an aquifer (positive values) or total water gain from an aquifer (negative values); *downstream release* was the total amount of water released to downstream canals from a pond; and *final storage* was the water stored in a pond at the end of the simulation period. It was shown that equation 57 was satisfied for all ponds.

Another way to examine the water budget is by viewing a whole flow system as a "system node," which combines canals and control ponds with inflow from the Zenith node and outflows from nodes Raymond, water unit 11, and DCC (see figure 4). The over-

all water budget for the entire canal and control-pond system is summarized in table 11 for the entire simulation period. In table 11, initial storage was the summation of pond storage at the beginning of the simulation (13,102.68 acre-ft), which was interpreted from the measured water levels on June 10, 1996. Total stream inflow was the inflow from Rattlesnake Creek to water unit 5 (17,782.21 acre-ft), which was measured at the USGS streamflow-gaging station near Zenith. Local water gain to the system included surface runoff due to precipitation (6,559.04 acre-ft, 6,499.77 acre-ft of which were to ponds) and ground-water seepage to ponds (3,035.56 acre-ft) and was equal to 9,594.60 acre-ft. The outflow was the summation of outflows released from node Raymond (17,421.22 acre-ft) and from node water unit 11 (400.85 acre-ft). Total stream inflow to the system was almost the same as the stream outflow from the system. Although the total inflow to the system (stream inflow, runoff, and ground-water seepage to ponds) was much larger than the stream outflow from the system, the final water storage in the system was significantly reduced from the initial storage of 13,102.68 acre-ft to 9,211.88 acre-ft due to a large amount of local water loss through water-surface evaporation from ponds (10,683.74 acre-ft) and canal-flow transmission losses (2,761.79 acre-ft). The water loss due to water-surface evaporation was larger than the total local water gain within the refuge.

SIMULATION OF CANAL AND CONTROL-POND OPERATION FOR 1991 WATER YEAR

A simulation was conducted to determine the operation of the system of canals and control ponds under drought flow conditions as occurred during the 1991 water year (October 1, 1990, through September 30, 1991) with different rule curves. Discharge during the 1991 water year was used as simulated discharge because this water year was the driest in terms of total discharges in Rattlesnake Creek for water years 1973 through 1995 (Putnam and others, 1996). In the following sections, the data needed to conduct the simulation and the necessary assumptions about these data are discussed, and then the simulation results are presented.

Table 10. Water budgets simulated for selected control ponds at Quivira National Wildlife Refuge, June 11 through December 11, 1996

[All values are in acre-feet; --, not applicable]

Water-unit number (fig. 2)	initial storage	+ Total upstream inflow	+ Total surface runoff	- Water- surface evaporation	- Ground- water seepage	- Total downstream release	= Final storage
5	1,988.26	17,782.22	1,117.84	1,795.73	286.00	16,844.87	1,961.72
7	39.72	486.93	31.98	57.25	0	431.67	69.71
10A	145.48	169.66	57.79	109.78	0	117.67	145.48
10B	145.48	531.21	55.60	98.40	34.04	467.15	132.70
10C	19.54	459.95	9.25	20.34	0	449.93	18.47
11	388.37	448.42	54.08	101.65	0	400.85	388.37
14A	161.70	214.52	96.82	152.05	0	152.25	168.74
14B	93.40	197.43	150.80	120.57	-9.20	179.99	150.27
14C	15.51	102.95	6.88	12.81	18.09	91.24	3.20
16	62.67	114.02	29.02	51.88	0	92.57	61.26
20A	163.88	493.65	142.78	250.73	0	353.34	196.24
20B	163.88	433.68	141.17	249.69	7.36	285.45	196.23
21	34.34	531.79	31.32	48.36	0	494.16	54.93
22	2.30	489.96	13.63	22.24	0	460.62	23.03
23	15.41	458.46	10.26	17.77	0	448.68	17.68
24	132.55	12,741.83	63.05	93.63	103.47	12,666.91	73.42
25	18.00	676.23	55.85	72.37	40.89	494.09	142.73
26	91.48	862.43	63.69	93.92	7.25	829.71	86.72
28	6.11	635.03	44.60	69.63	0	497.53	118.58
29	.20	482.72	38.38	53.35	0	404.38	63.57
30	2.82	396.78	62.17	97.04	0	196.68	168.05
40	55.91	77.90	14.03	34.19	-36.27	83.58	66.34
48	3.94	316.42	73.47	72.85	0	245.36	75.62
49	51.63	567.52	64.72	102.76	11.87	437.18	132.06
57	212.22	1,193.02	156.57	257.91	0	1,102.51	201.39
58	146.39	1,388.52	83.47	135.49	-86.07	1,429.80	139.16
61	212.80	340.11	123.31	209.47	-50.68	380.83	136.60
62	48.58	59.57	17.64	33.97	-31.89	57.62	66.09
63	268.98	132.23	129.33	232.08	-76.44	243.73	131.17
75	2,445.85	1,177.96	1,445.34	2,043.86	-2,484.88	4,004.79	1,505.38
78	5,270.43	5,359.15	1,728.99	3,161.14	-291.63	7,252.18	2,236.88
80	355.20	7,542.65	155.93	363.20	-85.77	7,705.34	71.01
81	25.31	7,252.18	41.67	94.73	-323.22	7,542.65	5.00
83	314.34	0	188.34	352.91	-68.48	14.14	204.11
Total	13,102.68	--	6,499.77	10,683.75	-3,035.56	--	9,211.91

Table 11. Water budget simulated for entire canal and control-pond system at Quivira National Wildlife Refuge, June 11 through December 11, 1996

[All values are in acre-feet; --, not applicable]

Water-budget component	Storage	Inflow	Outflow
Initial storage	13,102.68	--	--
Stream inflow	--	17,782.21	--
Surface runoff	--	6,559.04	--
Water-surface evaporation	--	--	10,683.74
Net ground-water seepage	--	3,035.56	--
Canal-flow transmission loss	--	--	2,761.79
Outflow from Raymond node	--	--	17,822.07
Final storage	9,211.88	--	--

Data Preparation

In this section, data needed for the simulation are discussed. Measurement data were used if available. If some data were not available, reasonable values were interpreted on the basis of other related data.

Precipitation

One of the major factors affecting the generation of direct overland surface runoff to ponds is the amount of precipitation. Figure 13A shows the daily precipitation measured at the Sandyland Experiment Station and at the USGS streamflow-gaging station near Zenith (fig. 1). Precipitation data from October 1, 1990, through May 20, 1991, were measured at the Sandyland Experiment Station. Precipitation data from May 21 through September 30, 1991, were measured at the USGS streamflow-gaging station near Zenith. The total amount of precipitation during the 1991 water year was 13.43 in.

Water-Surface Evaporation

The daily potential evapotranspiration (PET) estimated with the Penman method using the climatic data collected at the Sandyland Experiment Station (Marios Sophocleous, Kansas Geological Survey, written commun., 1996) is shown in figure 13B. The total amount of PET was 61.23 in. for the 1991 water year. For the

1991 water year simulation, it was assumed that the daily water-surface evaporation rate for ponds on the refuge was equal to the corresponding daily potential evapotranspiration at the Sandyland Experimental Station.

Canal Discharge

Discharges for Rattlesnake Creek measured at the USGS streamflow-gaging stations near Zenith and Raymond (fig. 1) from October 1, 1990, through September 30, 1991, are shown in figure 13C (Geiger and others, 1992). The mean daily discharges for the Zenith and Raymond stations during the 1991 water year were 6.59 and 2.77 ft³/s, respectively, which are much smaller than the long-term means of 50.6 ft³/s (1973–95 water years) and 48.8 ft³/s (1960–95 water years), respectively. As shown in the figure 13C, there was almost no flow during late September 1991.

For this simulation, the daily mean discharge observed at the USGS streamflow-gaging station near Zenith station was used as daily inflows to Little Salt Marsh from Rattlesnake Creek. The daily mean discharge observed at the USGS streamflow-gaging station near Raymond was used as the streamflow requirement for Rattlesnake Creek near Raymond.

Canal-Flow Transmission Losses

Canal-flow transmission loss was difficult to estimate. Because there were no data available to estimate the canal-flow transmission loss coefficient for the canals on the refuge during the simulation period, the estimated transmission loss coefficient (k in equation 42) of $9.16 \times 10^{-6} \text{ ft}^{-1}$ for the 1996 simulation period was used for this simulation. Similar to 1996, canal-flow transmission losses occurred only in canals south of the RC Canal.

Ground-Water Discharge to Ponds

No monthly data for ground-water discharge to ponds were available for the simulation period. The study conducted using MODFLOW by Marios Sophocleous (Kansas Geological Survey, written commun., 1996) shows that the amount of annual ground-water discharge to ponds on the refuge was almost the same from 1975 through 1990. Consequently, the monthly ground-water-discharge data obtained from Marios Sophocleous (Kansas Geological Survey, written commun., 1997) for 1994 were used (see table 2).

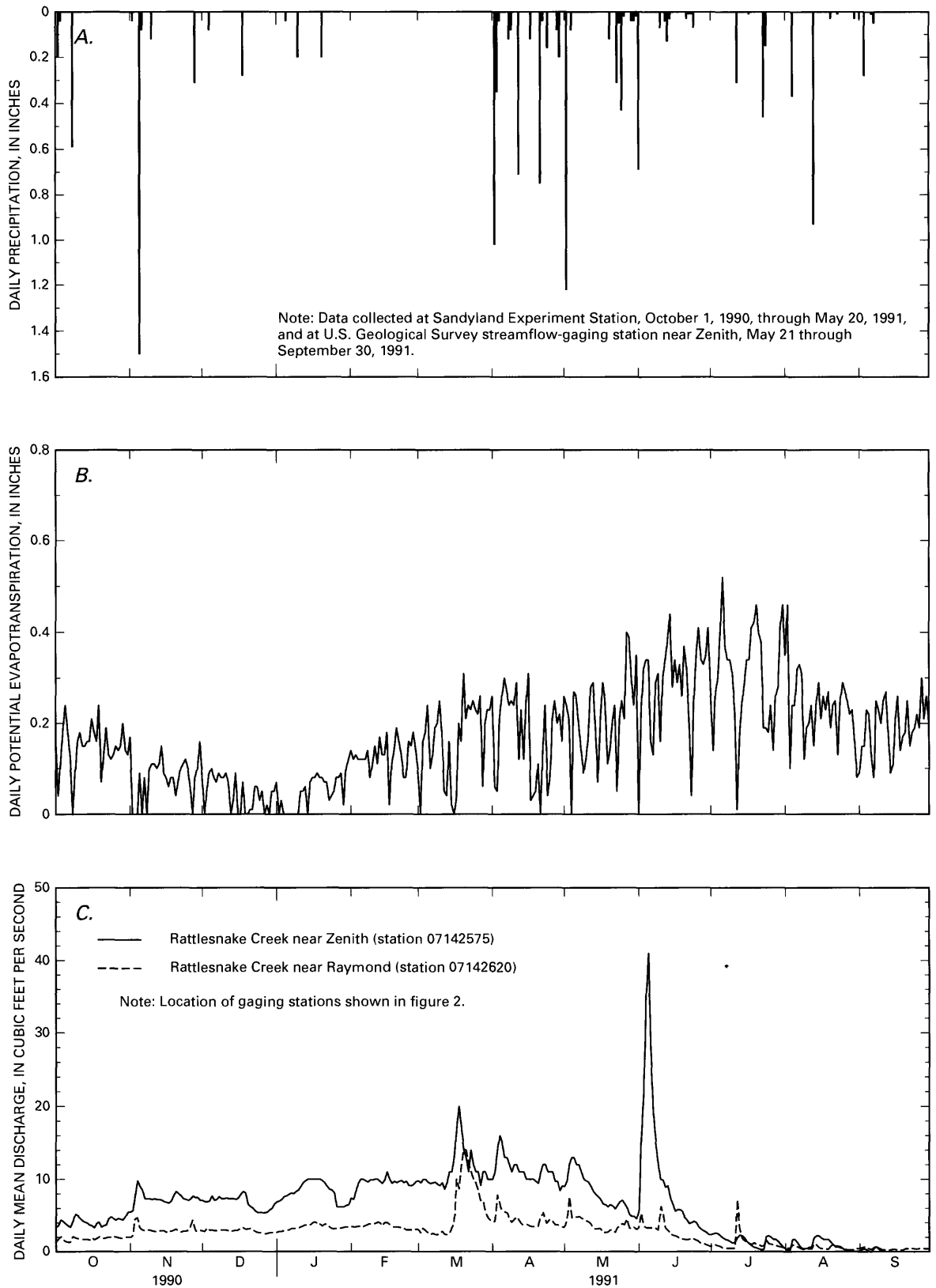


Figure 13. (A) Daily precipitation, (B) daily potential evapotranspiration, and (C) daily mean discharge for Rattlesnake Creek, 1991 water year. Part of the precipitation data and potential evapotranspiration data are from the Kansas Geological Survey (Marios Sophocleous, written commun., 1996), and the discharge data are from Geiger and others (1992).

Initial Water Storage in Ponds

The amount of initial water storage in the control ponds affects the operation of the ponds and the final water budgets. The amount of water stored in the control ponds on September 30, 1990, was not known. The 1991 water year simulation was used to evaluate the daily operation of ponds with different rule curves during drought conditions. Therefore, the initial water storage in a pond was simply set at 80 percent of full-pond capacity for the 1991 simulation (see table 12) to be consistent with the study by Marios Sophocleous (Kansas Geological Survey, written commun., 1996).

Pond Zoning and Operating Policy

Each control pond was divided into four storage zones—inactive zone, lower zone, upper zone, and extended upper zone as shown in figure 5. Normal operating storage of a pond consisted of water stored between the lower zone and upper zone and was set between 20 and 100 percent of full-pond storage capacity for this simulation. In the other words, the lower boundary of the lower storage zone was set at 20 percent of full-pond capacity, and the top boundary of the upper storage zone was set at 100 percent of full-pond capacity. The rule curve was set within this operating storage range. Four different rule curves corresponding to different simulations were set at 60, 70, 80, and 90 percent of full-pond capacity, respectively. The capacity of the inactive zone of a pond was set at 20 percent of full-pond storage capacity (selected in consultation with the U.S. Fish and Wildlife Service). The top boundary of the extended upper zone was set 0.5 ft higher than corresponding full-pond capacity. Pond zoning expressed as pond storage along with the rule curve at 90 percent of full-pond capacity are summarized in table 12.

The priority of pond operation for the 1991 water year was the same as for 1996 (see the discussion of pond priority for the 1996 simulation). Typical values for penalty coefficients used in the 1991 water year simulation are also summarized in table 12. The canal-flow zoning and the assignment of penalty coefficients were the same as those used in the simulation for 1996 (see table 9).

Results

Four different simulations of canal and control-pond operation at the refuge were conducted with the rule curve of a pond set at 60, 70, 80, and 90 percent of full-pond capacity, respectively. Other specifications for pond zoning and canal outflows from the refuge were (1) the initial storage of a pond was set at the 80 percent of full-pond capacity; (2) the inactive storage of a pond was set at the 20 percent of full-pond capacity; and (3) outflows of Rattlesnake Creek near the USGS streamflow-gaging station near Raymond were fixed and equal to the discharges observed for the 1991 water year.

Results of operating the canals and control ponds using a rule curve of 90 percent of full-pond capacity are described first. The simulated water budget for water unit 5 is shown in figure 14. Figure 14A shows the inflows from the upstream Zenith node to water unit 5 (also see figure 4), which are equal to the discharges observed at the USGS streamflow-gaging station near Zenith. The total releases to all downstream nodes (water units 7 and 10A, and canal joints C-2 and JE-1) from unit 5 are shown in figure 14C. The simulated pond water stage corresponding to water storage (fig. 14D) is shown in figure 14E. Similar figures could also be generated for the remaining control ponds. These figures reflect the operation of a single pond during an entire simulation period with the current operating policy. These figures also can be used to evaluate whether some specifications, such as the target water level, in the operating policy are satisfied. Water storage after mid-June 1991 decreased and reached the inactive zone (fig. 14D) and could not be maintained at the target level due to insufficient inflow and water-surface evaporation. In other words, if the target level in water unit 5 was set too low, water unit 5 could be dry at the end of the period under inflow conditions that were simulated.

To show the water budget of a control pond during the simulation period, table 13 summarizes water-budget components of ponds with the rule curve at 90 percent of full-pond capacity. It is seen from table 13 that the final storage value for all ponds at the end of the simulation period was much smaller than the initial storage values. Many small ponds were dry at the end of the simulation period. Total water-surface evaporation for all ponds was much larger than other water-budget components (runoff, ground-water seepage).

Table 12. Initial storage, rule curve, zoning, and penalty coefficients assigned to control ponds at Quivira National Wildlife Refuge, 1991 water year

[acre-ft, acre-feet]

Water-unit number (fig. 2)	Initial storage at 80 percent of full-pond capacity (acre-ft)	Rule curve at 90 percent of full-pond capacity (acre-ft)	Upper boundary of extended upper zone (acre-ft)	Penalty coefficient for extended upper zone	Upper boundary of upper zone (acre-ft)	Penalty coefficient for upper zone	Lower boundary of lower zone (acre-ft)	Penalty coefficient for lower zone	Lower boundary of inactive zone (acre-ft)	Penalty coefficient for inactive zone
5	1,492.80	1,679.40	2,312.18	3,000	1,866.00	2,000	373.20	3,000	0	6,000
7	32.00	36.00	54.49	3,000	40.00	1,500	8.00	1,500	0	3,000
10A	116.00	130.50	180.30	3,000	145.00	1,500	29.00	1,500	0	3,000
10B	116.00	130.50	180.30	3,000	145.00	1,500	29.00	1,500	0	3,000
10C	10.40	11.70	14.61	3,000	13.00	1,500	2.60	1,500	0	3,000
11	270.40	304.20	413.90	3,000	338.00	1,500	67.60	1,500	0	3,000
14A	156.80	176.40	242.20	3,000	196.00	1,500	39.20	1,500	0	3,000
14B	76.80	86.40	144.01	3,000	96.00	1,500	19.20	1,500	0	3,000
14C	12.80	14.40	19.07	3,000	16.00	1,500	3.20	1,500	0	3,000
16	64.00	72.00	96.07	3,000	80.00	1,500	16.00	1,500	0	3,000
20A	156.00	175.50	268.01	3,000	195.00	1,500	39.00	1,500	0	3,000
20B	156.00	175.50	268.01	3,000	195.00	1,500	39.00	1,500	0	3,000
21	64.80	72.90	96.62	3,000	81.00	1,500	16.20	1,500	0	3,000
22	10.40	11.70	18.81	3,000	13.00	1,500	2.60	1,500	0	3,000
23	12.00	13.50	19.66	3,000	15.00	1,500	3.00	1,500	0	3,000
24	28.00	31.50	53.04	3,000	35.00	1,500	7.00	1,500	0	3,000
25	236.80	266.40	344.05	3,000	296.00	1,500	59.20	1,500	0	3,000
26	88.80	99.90	142.39	3,000	111.00	1,500	22.20	1,500	0	3,000
28	122.40	137.70	198.82	3,000	153.00	1,500	30.60	1,500	0	3,000
29	72.80	81.90	124.51	3,000	91.00	1,500	18.20	1,500	0	3,000
30	95.20	107.10	161.64	3,000	119.00	1,500	23.80	1,500	0	3,000
40	52.80	59.40	83.19	3,000	66.00	1,500	13.20	1,500	0	3,000
48	90.40	101.70	161.19	3,000	113.00	1,500	22.60	1,500	0	3,000
49	127.20	143.10	209.05	3,000	159.00	1,500	31.80	1,500	0	3,000
57	169.60	190.80	280.74	3,000	212.00	1,500	42.40	1,500	0	3,000
58	200.80	225.90	302.82	3,000	251.00	1,500	50.20	1,500	0	3,000
61	398.40	448.20	613.17	3,000	498.00	1,500	99.60	1,500	0	3,000
62	96.00	108.00	145.00	3,000	120.00	1,500	24.00	1,500	0	3,000
63	271.20	305.10	419.01	3,000	339.00	1,500	67.80	1,500	0	3,000
75	1,956.80	2,201.40	3,490.32	3,000	2,446.00	1,500	489.20	1,500	0	3,000
78	4,216.00	4,743.00	6,091.37	2,000	5,270.00	1,000	1,054.00	1,000	0	3,000
80	284.00	319.50	474.34	2,000	355.00	1,000	71.00	1,000	0	3,000
81	20.00	22.50	60.68	2,000	25.00	1,000	5.00	1,000	0	3,000
83	251.20	282.60	419.31	3,000	314.00	1,500	62.80	1,500	0	3,000

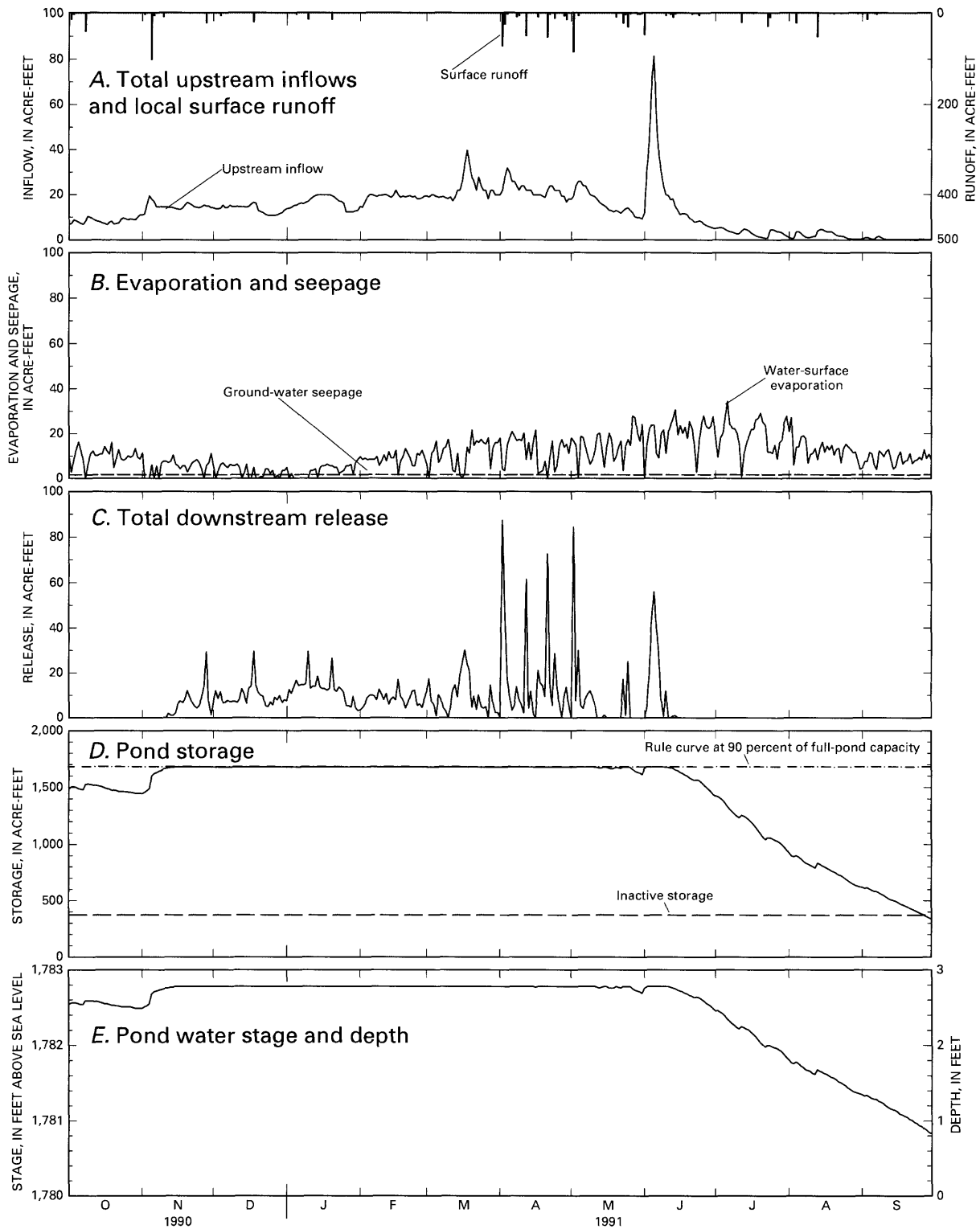


Figure 14. Water budget simulated for water unit 5, 1991 water year.

Table 13. Water budgets simulated for control ponds at Quivira National Wildlife Refuge with rule curve at 90 percent of full-pond capacity, 1991 water year

[All values are in acre-feet; --, not applicable]

Water-unit number (fig. 2)	Initial storage at 80 percent of full-pond capacity	+ Total upstream inflow	+ Total surface runoff	- Water- surface evaporation	- Ground- water seepage ¹	- Total downstream release	= Final storage
5	1,492.80	4,772.63	900.16	3,970.32	545.66	2,312.24	337.37
7	32.00	27.62	17.03	60.87	0	15.78	0
10A	116.00	81.59	48.37	196.59	0	41.33	8.04
10B	116.00	56.16	31.97	132.51	64.84	6.78	0
10C	10.40	6.68	5.53	22.61	0	0	0
11	270.40	0	41.11	175.91	0	0	135.60
14A	156.80	117.93	67.12	252.34	0	80.85	8.66
14B	76.80	64.16	44.27	171.41	-18.25	27.51	4.56
14C	12.80	56.63	5.13	17.70	24.97	31.89	0
16	64.00	48.64	19.21	72.22	0	54.60	5.03
20A	156.00	100.37	78.10	303.93	0	29.97	.57
20B	156.00	196.85	96.66	380.01	13.88	55.62	0
21	64.80	51.83	22.11	85.13	0	50.50	3.11
22	10.40	50.07	6.45	23.09	0	43.83	0
23	12.00	43.62	6.40	23.31	0	38.71	0
24	28.00	1,541.96	17.28	62.83	149.33	1,375.08	0
25	236.80	94.39	73.72	268.01	75.04	61.85	.01
26	88.80	99.09	30.52	121.74	13.50	83.17	0
28	122.40	61.71	44.38	172.26	0	53.03	3.20
29	72.80	51.45	31.35	123.71	0	30.83	1.06
30	95.20	30.25	42.70	167.16	0	.99	0
40	52.80	0	30.79	134.70	-86.83	8.19	27.53
48	90.40	35.50	43.20	167.99	0	.12	.99
49	127.20	45.13	52.38	205.68	19.03	0	0
57	169.60	343.12	114.11	485.65	0	102.75	38.43
58	200.80	362.88	92.46	394.82	-173.34	390.50	44.16
61	398.40	565.28	208.54	902.49	-109.08	283.94	94.87
62	96.00	0	40.37	173.96	-70.19	3.15	29.45
63	271.20	189.92	150.50	650.00	-161.39	59.07	63.94
75	1,956.80	254.24	1,478.91	6,103.74	-5,002.59	1,642.84	945.96
78	4,216.00	1,881.86	1,262.39	5,406.83	-587.57	1,486.99	1,054.00
80	284.00	2,028.75	147.51	646.47	-172.74	1,915.53	71.00
81	20.00	1,486.99	37.92	162.60	-651.44	2,028.75	5.00
83	251.20	0	133.69	522.11	-137.75	0	.53
Total	11,525.60	--	5,422.34	22,760.70	-6,264.92	--	2,883.07

¹The positive values of ground-water seepage indicate that ponds lost water to the aquifer. The negative values of ground-water seepage indicate that ponds gained water from the aquifer.

Table 14. Water budget simulated for entire canal and control-pond system at Quivira National Wildlife Refuge with rule curve at 90 percent of full-pond capacity, 1991 water year

[All values are in acre-feet; --, not applicable]

Water-budget component	Storage	Inflow	Outflow
Initial storage	11,525.60	--	--
Stream inflow	--	4,772.63	--
Surface runoff	--	5,422.34	--
Water-surface evaporation	--	--	22,760.70
Net ground-water seepage	--	6,264.92	--
Canal-flow transmission loss	--	--	336.51
Outflow from Raymond node	--	--	2,005.24
Final storage	2,883.07	--	--

To examine the water budget for the whole flow system at the refuge, table 14 summarizes the overall water budget for the entire canal and control-pond system with the rule curve set at 90 percent of full-pond capacity. It can be seen from this table that although there were total inflows of 16,459.89 acre-ft, of which 4,772.63 acre-ft were from Rattlesnake Creek at Zenith node, 5,422.34 acre-ft from direct surface runoff, and 6,264.92 acre-ft from the ground-water seepage to ponds, the final water storage in the system was substantially reduced from the initial storage of 11,525.60 acre-ft, which was set at 80 percent of full-pond capacity, to 2,883.07 acre-ft due to the outflows from the Raymond node, water-surface evaporation, and canal-flow transmission loss. Total water out of the system (outflow, evaporation, and canal-flow transmission loss) from the system was 25,102.45 acre-ft, of which 22,760.70 acre-ft (or 91 percent of water outflow from the system) was due to water-surface evaporation. At the end of simulation period, 30 out of 34 ponds, including water unit 5, had water stored only in the inactive zone or were dry due to the large amount of water-surface evaporation.

To compare the operation of canal and control ponds with the rule curve at 90 percent of full-pond capacity, simulations were also conducted with the rule curves at 80, 70, and 60 percent of full-pond capacity. All of simulations were conducted with the same model specification except for the rule curves.

Figure 15 shows the change in water storage for water units 5 and 78, respectively, with different rule curves. As the rule curve was reduced from 90 to 60 percent of full-pond capacity, water storage in water unit 5 during the simulation period decreased, and the final pond storage was also reduced from 337 to 48 acre-ft (fig. 15A). Because water unit 5 had the highest priority and because the initial storage was higher than the rule curve, water was released immediately downstream as shown in figure 15A. On the other hand, water storage in water unit 78 increased during the simulation period (fig. 15B). Because water unit 78 had the lowest priority and because water storage in the upstream higher priority ponds was in the upper zone, water was released from these higher priority ponds to maintain their rule curves, and water released from the upstream pond was stored in the unit 78, which caused the water storage to reach full-pond capacity (fig. 15B). After mid-June 1991, there were not enough inflow (upstream inflow plus surface runoff) to water unit 5 to maintain water levels at the rule curve, and water levels decreased due to water-surface evaporation. At the end of simulation, the water level in water unit 5 was located in the inactive zone (figs. 14 and 15A). Similar changes in water storages were also observed for other control ponds.

The simulated water budget for the entire canal and control-pond system for the 1991 water year with different rule curves is summarized in table 15. As the rule curves were reduced from 90 to 60 percent of full-pond capacity, surface runoff, water-surface evaporation, and ground-water seepage from ponds were reduced, and stream outflow and final storage increased (see table 15). The reduction of the rule curve of a pond generally caused a lower pond water level to be maintained for the higher priority ponds. In other words, the total water-surface evaporation and rainfall onto the water-surface area of a pond were reduced for the same evaporation rate and precipitation depth. When initial pond storage was higher than the rule curve (initial storage was set at 80 percent of full-pond capacity), water was released from the ponds with higher priority to meet the rule-curve water level, which caused more canal-flow transmission losses along the canals in the south part of the refuge and increased outflows from water unit 11. The final pond water storage also increased due to storage increases in water unit 78 (fig. 15B) and other ponds in the north part of the refuge.

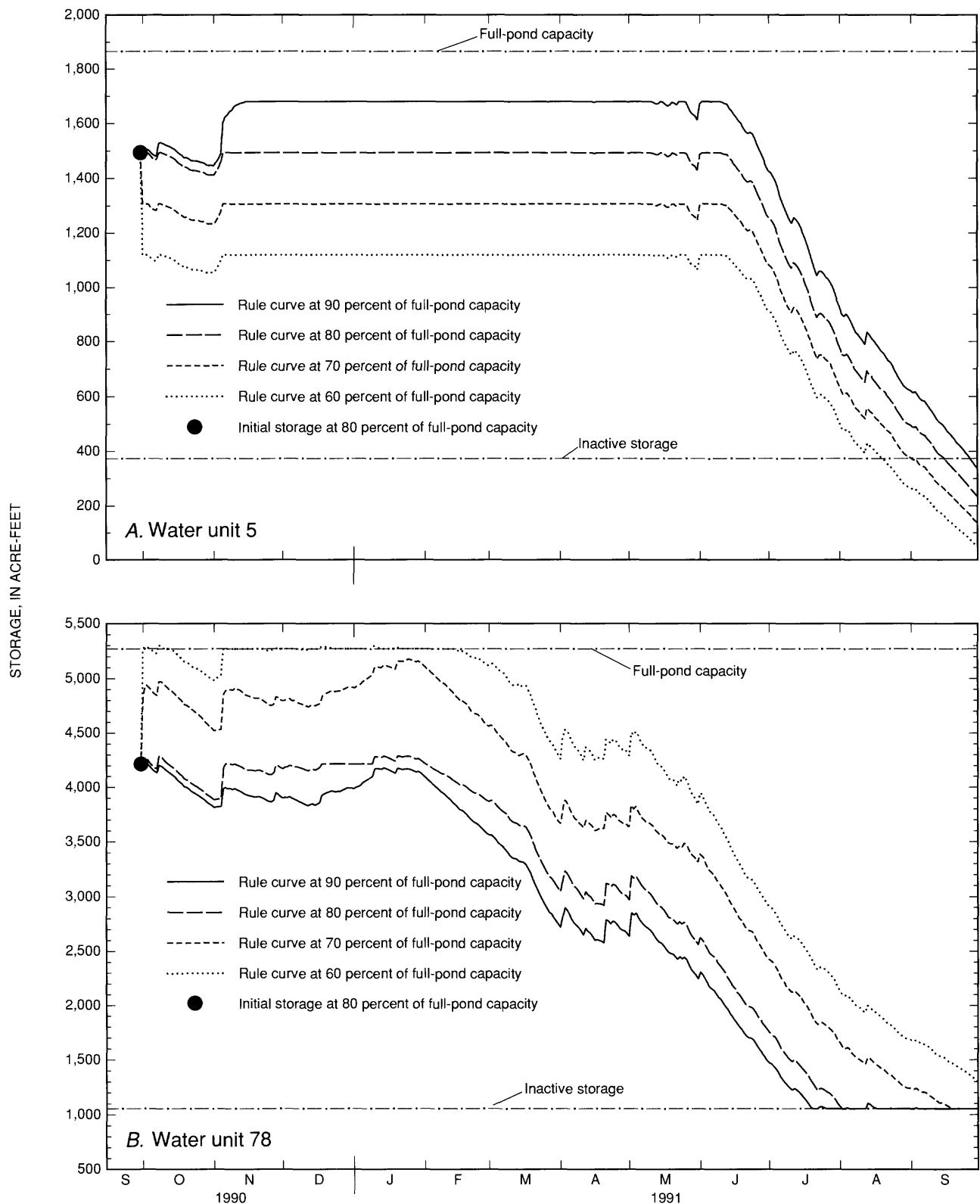


Figure 15. Simulated pond water storage with different rule curves for (A) water unit 5 and (B) water unit 78, with initial storage at 80 percent of full-pond capacity, 1991 water year.

Table 15. Water budgets simulated for entire canal and control-pond system at Quivira National Wildlife Refuge using different rule curves, 1991 water year

[all values are in acre-feet]

Water-budget component	Rule curve set at 90 percent of full-pond capacity	Rule curve set at 80 percent of full-pond capacity	Rule curve set at 70 percent of full-pond capacity	Rule curve set at 60 percent of full-pond capacity
Initial pond storage	11,525.60	11,525.60	11,525.60	11,525.60
Inflow:				
Stream inflow from Rattlesnake Creek	4,772.63	4,772.63	4,772.63	4,772.63
Surface runoff	5,422.34	5,393.31	5,260.83	4,981.23
Net ground-water seepage, including canal-flow transmission loss	-5,928.41	-5,900.37	-5,854.23	-5,854.04
Outflow:				
Water-surface evaporation	22,760.70	22,694.41	22,238.25	21,299.74
Total outflow	2,005.24	2,005.28	2,073.30	2,547.26
Final pond storage	2,883.07	2,892.22	3,101.74	3,286.50

Simulation results for the 1991 water year indicate that water-surface evaporation was the major factor in lowering water storage in ponds. Storing more water in the ponds in the north part of the refuge by reducing the rule curve for higher priority ponds may reduce the overall water-surface evaporation. However, this will also cause water unit 5 to dry out quickly if there is not enough upstream inflow as was the case during the 1991 water year. Maintaining high water levels in water unit 5 depends upon the rule curve in water unit 5 being set at a high level. The simulation results discussed for the 1991 water year were obtained based on a number of assumptions, such as the initial storage in ponds. If the specifications for the simulation model change, the results may be much different.

SUMMARY

In 1995, a 3-year study was undertaken to develop a water budget and flow-routing model to assist the U.S. Fish and Wildlife Service in determining the outcome of possible water-management options at the Quivira National Wildlife Refuge, south-central Kansas. The study was done by the U.S. Geological Survey in cooperation with the Kansas Geological Survey. A computer program OPONDS, written in FORTRAN,

was developed using network flow analysis to determine the optimal operation of a system of canals and control ponds. Applications of the model are presented that investigate the daily operation of canals and control ponds on the refuge using historical discharge and pond water levels.

The daily operation of a system of canals and control ponds at the refuge in the Rattlesnake Creek Basin was simulated for June 11 through December 11, 1996, using a linear-network flow model. In this simulation, some management requirements included the measured water levels of control ponds as the target management pond levels and the observed stream discharges in Rattlesnake Creek near Raymond as the outflow requirement from the refuge. Measured precipitation and calculated potential evapotranspiration were used to compute the surface runoff to ponds and water-surface evaporation, respectively. The operating policy was determined by using selected storage zones within a pond and prioritization of the ponds by using the relative magnitude of penalty coefficients within the computer model to adjust pond storages and canal flows. Results of the 1996 simulation indicate that the current specification for pond zoning and rule curves, with water unit 5 given the highest priority and ponds in the north part of the refuge given the lowest priori-

ties, simulated pond levels that matched well with observed ones. Root mean square errors between simulated and measured water levels were less than 0.13 ft except for water units 24 and 30. Water storage in ponds during the simulation period was substantially reduced due to water-surface evaporation and canal-flow transmission losses.

Simulation of canal and control-pond operation under drought conditions during the 1991 water year was also conducted with different target pond water levels. This simulation used 1991 measured stream discharges, precipitation, and potential evapotranspiration data and 1994 ground-water seepage to ponds to investigate the operation of the canals and control ponds. The operating policy used four pond storage zones and the prioritization of ponds, with water unit 5 having the highest priority and ponds in the north part of the refuge having the lowest priority. Results showed that under the same initial water storage of 80 percent of full-pond capacity lowering target pond water levels reduced water-surface evaporation, resulted in more water stored in ponds in the north part of the refuge, and caused a substantial decrease in the final water storage in water unit 5. In other words, to maintain high water storage in water unit 5, the target water level in this unit should be high. To reduce the total water-surface evaporation loss, the target water level should be low for unit 5 so that water is stored in the ponds in the north part of the refuge. It should be noted that results of the 1991 water year simulation were obtained with the same initial storage of ponds and measured discharges of Rattlesnake Creek near Raymond as the 1996 simulation. The optimal operation of a system of canals and control ponds depends on having a well-defined operating policy and accurate data and may require several combinations of model specifications to obtain optimum results.

The OPONDS model can be applied to other operations at the Quivira Refuge simply by modifying the conceptual flow-network configuration and changing the operating policy through pond-storage and canal-flow zoning and corresponding penalty coefficients. The OPONDS model can be applied to operational matters such as the determination of target water levels and pond water releases, the operation of the outlet structures, and canal flow and routing.

The OPONDS model is a simplification of a complex canal-pond network flow system and is limited in simulating the operation of the flow system by the accuracy of data used in the model and some assump-

tions. Nonetheless, the OPONDS model is a useful tool for estimating the effects of possible water-management options for the Quivira National Wildlife Refuge.

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APPENDICES

APPENDIX A. GENERAL DESCRIPTION OF OPONDS COMPUTER PROGRAM

The computer program OPONDS (The optimal Operation of a system of PONDS) is written in FORTRAN 77. The main purpose of the program is to simulate the operation of a system of canals and ponds using various management requirements. Some examples of management requirements are target water levels (rule curves) of ponds, target releases from ponds, minimum required canal flow, maximum allowed canal flow, target water withdrawals, prioritization of ponds, and so forth. The program combines the concepts of pond zoning and rule curves together with the prioritization of ponds to determine operation of a system of canals and ponds using a linear programming technique. The resulting model is very flexible and can be easily adapted to any configuration of a canal-pond system. The introduction of penalty coefficients to the model allows model users to switch easily from one policy of operation to another by simply altering values of the penalty coefficients assigned to prioritize the various ponds.

The modeling approach converts the canal-pond operation into a minimum-cost network flow problem. Some management requirements become constraints in the network flow problem. After the minimum-cost flows are determined, these flows are transferred back to their corresponding pond-storage or canal-flow values.

The overall OPONDS program structure is shown in figure 16. In terms of functions, the whole program can be divided into three parts: (1) build and modify a flow network, (2) determine the flow in the network, and (3) output water budgets in nodes and arcs.

The first part of the program builds a basic flow network. The arcs in this network do not change throughout the simulation period and include pond-storage arcs and canal-flow arcs. For each time period, time-dependent contribution data, such as the net incremental inflow to nodes, precipitation, target water demand, water-surface evaporation, and ground-water seepage, are needed, and arcs representing these contribution data are generated and added into the basic network. If time-dependent management requirements such as seasonal flow boundary and pond rule curves are needed, the basic network can be expanded to represent these time-dependent data.

After the flow network is built, the flows in the network can be determined using a linear network flow algorithm called the out-of-kilter algorithm (Fulkerson, 1961). If no flows can be determined, the program execution terminates.

Once flows are determined for a network, water budgets for canals and ponds are computed. These water budgets are output for each time step. Time-series output of water budgets for selected canals and ponds are also provided.

The program source codes are listed in Appendix E of this report. The electronic form of the source codes may be obtained by contacting the U.S. Geological Survey in Lawrence, Kansas.

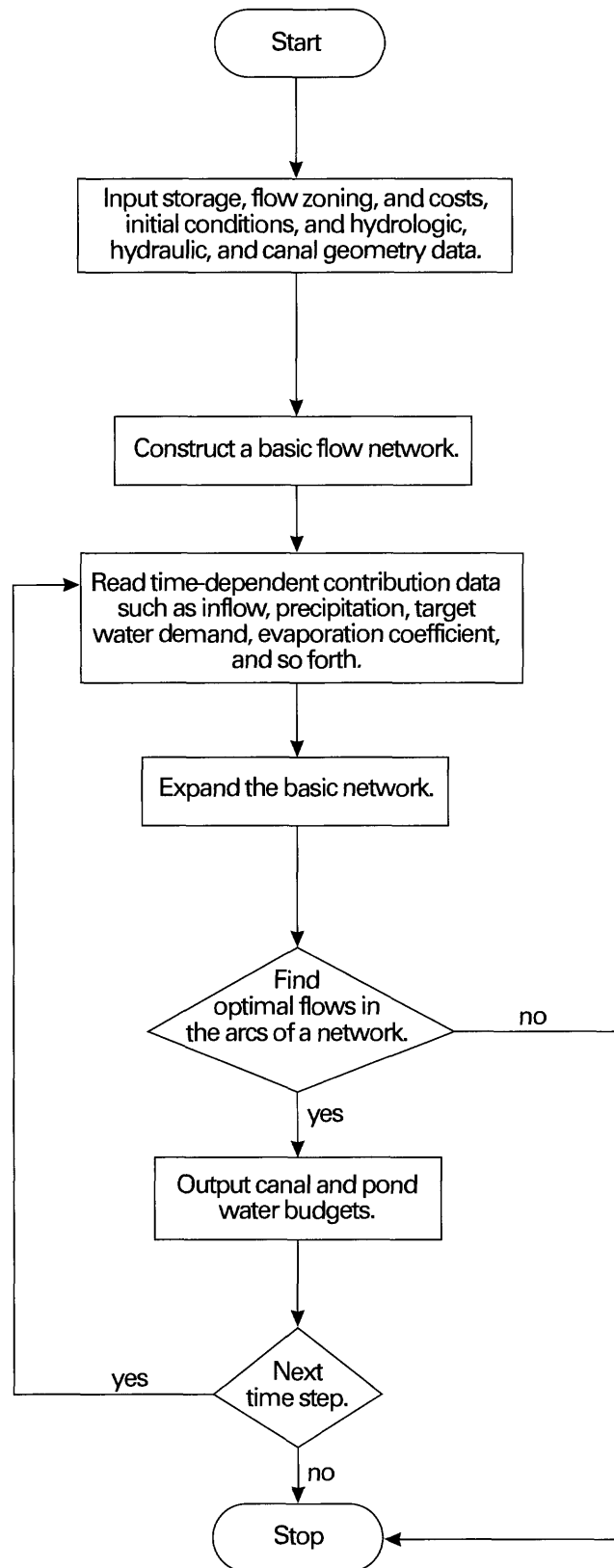


Figure 16. Overall structure of OPONDS computer program.

APPENDIX B. INPUT/OUTPUT INSTRUCTIONS FOR OPONDS COMPUTER PROGRAM

Because there are more than 20 input and output data files, all input- and output-data file names and associated file-identification codes are listed in the master data file. File-identification codes here are used to distinguish data files. Table 16 lists all available file-identification codes and descriptions of associated data files. The master data-file format is listed in table 17. Instruction file formats for different input data are summarized in tables 18 through 36. Most input data files consist of four parts of information—title area (five title lines), data unit code, nodal name list, and data matrix. Data are input with free format; that is, the data are delimited by spaces.

The number of input data files is dependent on the study need. The essential files to run the program are the master data file, the general network configuration and parameter file, and the network flow-configuration file if data in this file are not included in the general network configuration and parameter file. The other data files are added only if needed. For example, if a study involves the operation of pond(s), then files for relations of elevation-volume-area of ponds and pond-storage zoning are needed.

Most input data are related to a nodal name. Nodal names are limited to 12 characters and are not case sensitive. For example, POND_1 and pump_1 are valid nodal names. Commas and spaces are not allowed in a nodal name.

Table 16. List of file codes and descriptions for OPONDS computer program

[--, not applicable]

File code	File description	File format
Input data files		
0	General network configuration and parameter file	See table 18
1	Pond zoning file	See table 19
2	Network flow-configuration file	See table 20
3	Canal geometry file	See table 21
4	Outlet hydraulic-structure file	See table 22
5	Surface-runoff parameter file	See table 23
9	Pond elevation-volume-area file	See tables 24 and 25
10	Seasonal target water-demand file	See table 26
11	Seasonal water-surface evaporation file	See table 27
12	Seasonal flow-boundary file	See table 28
13	Seasonal rule-curve file	See table 29
16	Local net incremental inflow file	See table 30
17	Precipitation file	See table 31
18	Time-dependent, evaporation file	See table 32
19	Time-dependent, target water-demand file	See table 33
20	Time-dependent, rule-curve elevation file	See table 34
21	Time-dependent, flow-boundary file	See table 35
22	Ground-water-elevation file	See table 36
23	Fixed-flow file	See table 37
Output files		
26	Network configuration output	--
27	Nodal budget output	--
28	Arc budget output	--
29	Operation of hydraulic-structure output	--
30	File for listing nodal names for nodal water-budget output in time-series format	--
31	File for listing canal upstream and downstream nodal names for canal water-budget results in time-series format	--

Table 17. File format for master file in OPONDS computer program

[--, not applicable]

State- ment number	Informa- tion at state- ment	Vari- able	Definition	Vari- able type	State- ment number	Informa- tion at state- ment	Vari- able	Definition	Vari- able type
1-5	Title lines	--	Title and variable descriptions.	--	6	File code and file names	CD FILN M	File code (see table 16). File name.	integer character

The time-series output of water budgets for a node will use the nodal name as a part of the output file name. If the program is running on a personal computer (PC) and time-series output for a nodal water budget is needed, the corresponding nodal name is limited to five characters because a file name, not including the file extension, on PC MS-DOS systems is limited to eight characters.

Seasonal data here mean that values change seasonally (monthly, weekly, even daily) within a year and do not change over years. Some examples of seasonal data are target water demands, pond rule curves (target water level), and evaporation coefficients. These data can be input either as seasonal data or nonseasonal data depending on the length of the simulation period. If the whole simulation period is multiple years, the seasonal data can be specified in the seasonal data file. However, if the simulation period is less than 1 year, seasonal data can be input as nonseasonal data because this may result in smaller input files.

General Network-Configuration and Parameter File

The general network-configuration and parameter file is used for specifying the basic simulation information, such as length of the simulation period, the number of seasons of a year, and accuracy of output results (see items 1 through 7 in table 18). In addition to the simulation information, data for constructing a basic network, such as pond zoning, canal zoning, canal-flow directions, canal hydrologic and geometry data, and seasonal input data, also can be included in this file. The part numbers are designed to input these data (see instructions in table 18). Note that these data may be specified in separate files (see tables 19 through 23 and 26 through 29).

Table 18. File format for general network configuration and parameters (file-identification code 0 used in OPONDS computer program)

[<, less than; SCS, Soil Conservation Service; ft³/s, cubic feet per second; ft, feet; ft³/d, cubic feet per day; ft/d, feet per day; acre-ft, acre-feet; in/d, inch per day; in., inch; mm/d, millimeter per day; mm, millimeter; --, not applicable; >, greater than; <, less than]

Statement number	Information at statement	Variable name	Definition	Variable type	Default value	Unit
1-2	Title lines	SYSNAM	Canal-pond system name.	character		
3	Time step and seasons	PERD	Number of days in a time period.	real		day
		NPER	Number of periods in a year.	integer	--	--
4	Starting season	STMO	Starting season.	integer		--
		YR	Starting year.	integer	--	--
5	Length of simula- tion periods	NSPS	Number of simulation periods.			--
6	Convergence	RTERM	Flow convergence criterion.	real		ft ³ /s
		LDIRT	Maximum number of iterations.	integer	--	--
7	Output budget accuracy	XP	Number of decimal points in acre-ft.	integer		

Table 18. File format for general network configuration and parameters (file-identification code 0 used in OPONDS computer program)—Continued

Statement number	Information at statement	Variable name	Definition	Variable type	Default value	Unit
8	Save options	SAVOPT	Save options for general output (0—all; 1—input data; 2—network configuration; 9—none).	integer	0	
9	Part 1: Pond zones and bottom information	PART	Part.	character	PART	
		N	Part number.	integer	1	--
10	List of variables	--	--	string		
11	Data	NAME	Pond node name.	character		
		UNITCD	Elevation or storage unit code (0—ft; 1—acre-ft).	integer	--	--
		INST	Initial pond elevation or storage.	real	--	--
		BOT	Bottom elevation.	real	--	ft
		KY	Bottom-bed hydraulic conductivity.	real	--	ft/d
		B	Bottom-bed thickness.	real	--	ft
		RC	Rule-curve elevation or storage.	real	--	--
		Z(I)	Zone elevation or storage.	real	--	--
		COST(I)	Penalty coefficients. NZONE is the number of operational storage zones of a pond.	integer	--	--
		I = 1, NZONE)				
12	Finish	FINISH	Finish.	character	finish	--
13	Part 2: Flow network	PART	Part.	character	PART	--
		N	Part number.	integer	2	--
14	List of variables	--	--	string	--	--
15	Data	F_NODE	From-node name.	character	--	--
		T_NODE	To-node name.	char	--	--
		LBND	Lower flow boundary.	real	--	ft ³ /s
		UBND	Upper flow boundary.	real	--	ft ³ /s
		COST	Penalty coefficient for flow zone.	real	--	--
		INST	Initial canal storage.	real	--	acre-ft
		K	Traveltime through routing canal.	real	--	day
		X	Weighting factor between 0 and 0.5.	real	--	--
		SP	Canal-seepage coefficient (<1.0); if < 0, use Darcy's law.	real	--	--
		EV	Evaporation coefficient.	real	--	in/d
16	Finish	FINISH	Finish.	character	finish	--
17	Part 3: Canal geometry data	PART	Part.	character	PART	--
		N	Part number.	integer	3	--
18	List of variables	--	--	character	--	--

Table 18. File format for general network configuration and parameters (file-identification code 0 used in OPONDS computer program)—Continued

Statement number	Information at statement	Variable name	Definition	Variable type	Default value	Unit
19	Data	F_NODE	From-node name.	character		--
		T_NODE	To-node name.	character		--
		N	Canal roughness coefficient.	real		--
		L	Canal length.	real		ft
		J	Canal average slope.	real		--
		W	Canal width.	real		ft
		M	Canal side slope.	real	0	--
		D	Canal maximum depth.	real		ft
		KY	Canal riverbed hydraulic conductivity.	real		ft/d
						--
		THICK	Riverbed thickness.	real		ft
		ELEV	Riverbed elevation at entry of canal.	real		ft
20	Finish	FINISH	Finish.	character	finish	--
21	Part 4: Hydraulic structure	PART	Part.	character	PART	--
		N	Part number.	integer	4	--
22	List of variables	--	--	string	--	--
23	Information for outlet hydraulic structure—pipe	NAME	Structure name.	character	--	--
		F_NODE	From-node name.	character	--	--
		T_NODE	To-node name.	character	--	--
		TYPE	Structure type code (1—sharp-crested weir; 2—gate on spillway; 3—gate on broad-crested weir).	integer	--	--
		BELEV	Base elevation.	real	--	ft
		WLEN	Weir length.	real	--	ft
		WHITE	Weir height if sharp-crested and broad-crested weir, or the design water head if standard spillway.	real	--	ft
		GHITE	Gate opening height.	real	--	ft
		WB	Weir width (thickness).	real	--	ft
23	Information for outlet hydraulic structure—pipe	NAME	Structure name.	character	--	--
		F_NODE	From-node name.	character	--	--
		T_NODE	To-node name.	character	--	--
		TYPE	Structure type code (= 6).	integer	--	--
		BELEV	Base elevation.	real	--	ft
		WIDTH	Pipe diameter.	real	--	ft
		LENG	Pipe length.	real	--	ft
		FRIC	Pipe-friction factor.	real	0.025	--
		ENLOS	Pipe-entrance loss factor.	real	0.5	--
24	Finish	--	--	character	--	--
25	Part 5: Surface-runoff parameters	PART	Part.	character	PART	--
		N	Part number.	integer	5	--
26	List variable	--	--	--	--	--

Table 18. File format for general network configuration and parameters (file-identification code 0 used in OPONDS computer program)—Continued

Statement number	Information at statement	Variable name	Definition	Variable type	Default value	Unit
27	Data	NAME	Nodal name.	character	--	--
		A5DR0	Initial total antecedent 5-day rainfall.	real	--	in.
		A5DRI	Antecedent 5-day rainfall for dry conditions.	real	--	in.
		A5DRIII	Antecedent 5-day rainfall for wet conditions.	real	--	in.
		SCSCN	SCS curve number for average condition.	real	--	--
		AREA	Drainage area.	real	--	acres
28	Finish	--	--	--	--	--
29	Part 10: Seasonal target water demands	PART	Part.	character	PART	--
		N	Part number.	integer	10	--
30	Unit	WSUNIT	Target water-demand unit code (0—acre-ft; 1—ft ³ /s; 2—ft ³ /d).	integer	0	--
31	List of variables	TIME	Time step.	character	--	--
		(NAME (I), I = 1, NWSND)	Nodal names. NWSND is the number of nodes with seasonal target water demands.	character	--	--
32	Target water demands for each season N, N = 1, NPER	TIME (WSTB (N,J), J = 1, NWSND)	Season. Seasonal target water demands.	character real	-- --	-- --
33	Finish	FINISH	Finish.	character	finish	--
34	Part 11: Seasonal water-surface evaporation coefficient	PART	Part.	character	PART	--
		N	Part number.	integer	11	--
35	Unit	EVUNIT	Surface-water evaporation coefficient unit code (0—mm/d; 1—in/d; 2—ft/d).	integer	--	--
36	List of variables	TIME (NAME (I), I = 1, NEV)	Time step. Nodal names. NEV is the number of nodes with surface-water evaporation coefficients.	character character	-- --	-- --
37	Water-surface evaporation coefficients for each season N, N = 1, NPER	TIME (EVTB (N,J), J=1, NEV)	Season. Evaporation coefficients.	char real	-- --	-- --
38	Finish	FINISH	Finish.	character	finish	--
39	Part 12: Seasonal flow boundaries	PART	Part.	character	PART	--
		N	Part number.	integer	12	--
40	Unit	FBUNIT	Flow unit code (0—acre-ft; 1—ft ³ /s; 2— ft ³ /d).	integer	--	--

Table 18. File format for general network configuration and parameters (file-identification code 0 used in OPONDS computer program)—Continued

Statement number	Information at statement	Variable name	Definition	Variable type	Default value	Unit
41	List of nodal names	(NAME (J), J = 1, NFBAR)	From-nodal names. NFBAR is the number of arcs.	character	--	--
42	List of nodal names	(NAME (J), J = 1, NFBAR)	To-nodal names.	character	--	--
43	Zone index	TIME, (FBIDX (J), J = 1, NFBAR)	Season. Flow-zone index (-1, lower boundary of normal flow zone; +1, upper boundary of normal flow zone, <-2, lower extended flow zone; >+2, upper extended flow zone; that is, $ndex = zone \pm$).	character integer	-- --	-- --
44	Flow boundaries for each season N, N = 1, NPER	TIME, (FBTB (N,J), J = 1, NFBAR)	Season. Seasonal flow boundaries.	character real	-- --	-- --
45	Finish	FINISH	Finish.	character	finish	--
46	Part 13: Seasonal rule curve	PART N	Part. Part number.	character integer	PART 13	-- --
47	Unit	RCUNIT	Rule-curve elevation unit code (0—ft; 1— in.; 2—mm).	integer	--	--
48	List of variables	TIME (NAME (I), I = 1, NRCND)	Time step. Pond nodal names. NRCND is the number of pond nodes with seasonal rule curves.	character character	-- --	-- --
49	Rule curves for each season N, N = 1, NPER	N (RCTB (N,J), J=1, NRCND)	Season. Rule-curve elevation.	character real	-- --	-- --
50	Finish	FINISH	Finish.	character	finish	--

Pond-Zoning File

The pond-zoning file is used to specify pond-storage zones and bottom hydraulic parameters. Initial storage, rule curve, pond zoning, and penalty coefficients are specified in this file. The bottom hydraulic and geometry data include hydraulic conductivity, average thickness of bottom layer, and bottom elevation. Input instructions and explanations of variables are listed in table 19. The file-identification code is 1. All data specified in this file can also be included in the general network-configuration and parameter file (table 18).

Table 19. File format for initial pond condition and zoning (file-identification code 1 used in OPONDS computer program)

[ft, feet; acre-ft, acre-feet; ft/d, feet per day; --, not applicable]

State- ment num- ber	Information at statement	Variable name	Definition	Variable type	Unit
1–5	Title lines		Specify data information about source, etc.		
6	Pond zoning and bottom hydrau- lic parameter for each pond	NAME	Pond node name.	character	--
		UNITCD	Stage unit code (0—ft, 1—acre-ft).	integer	--
		INST	Pond initial elevation or storage.	real	--
		BOT	Bottom elevation.	real	ft
		KY	Bottom layer hydraulic conductivity.	real	ft/d
		B	Bottom layer thickness.	real	ft
		RC	Rule-curve stage.	real	--
		(Z(I)	Zone stage (see note 1).	real	--
		COST(I)	Penalty coefficients (see note 1). NZONE is	integer	--
		I = 1, NZONE)	the number of storage zones of a pond.		

Note:

1. If there is more than one zone, Z and COST must be specified for each zone. Zone boundaries (that is, elevations) or corresponding storage are specified at the beginning of the zones next to the rule curve.

Network Flow-Configuration File

The network flow-configuration file contains data for determining the directions of canal-flow arcs and canal-routing coefficients. Each record in the file represents an arc in a network. Because each flow zone in the canal is represented by an arc, there is one record for each extended flow zone. Canal-routing coefficients (initial canal storage, traveltime, weighting factor, and seepage coefficient) are optional. If canal routing is not needed, routing coefficients do not need to be specified or the values are set equal to zero. Input instructions and explanations of variables are listed in table 20. The file-identification code is 2. All data specified in this file also can be included in the general network-configuration and parameter file (table 18). Network-flow configuration data need to be specified in order to run the OPONDS program.

Table 20. File format for network flow configuration (file-identification code 2 used in OPONDS computer program)

[ft³/s, cubic feet per second; acre-ft, acre-feet; in/d, inches per day; --, not applicable; <, less than]

State- ment num- ber	Information at statement	Variable	Definition	Variable type	Unit
1–5	Title lines	--	--	--	--
6	Canal-flow zon- ing and routing coefficients for each flow zone	F_Node	From-node name.	character	--
		T_Node	To-node name (see note 1).	character	--
		LBND	Lower flow boundary (see note 2).	real	ft ³ /s
		UBND	Upper flow boundary (see note 2).	real	ft ³ /s
		COST	Penalty coefficient of flow zone.	integer	--
		INST	Initial canal storage.	real	acre-ft
		K	Traveltime through routing canal .	real	day
		x	Weighting factor between 0 to 0.5.	real	--
		SP	Canal-seepage coefficient (<1.0) (see note 3).	real	--
		EV	Evaporation coefficient (see note 4).	real	in/d

Notes:

1. If there is not a physical downstream node (that is, the downstream node is SINK), use node name SKSC.
2. If there are more than one flow zone, the normal flow zone must be specified first.
One record is specified for each flow zone, including normal, lower, and upper zones.
3. If SP < 0, the seepage will be estimated using Darcy's law. The average depth of water is estimated using Manning's equation. The hydraulic and geometry parameters used in Manning's equation must be specified.
4. Only the evaporation (EV) occurring in the normal flow range will be estimated.
If EV > 0, this value will be used for entire simulation period.
If EV < 0, the evaporation coefficient will be interpreted in terms of coefficients in upstream and downstream nodes if available. To estimate the surface-water evaporation for canals, canal geometry parameters (length, width, and side slopes) must be specified in the geometry and riverbed hydraulic parameter file. If the canal geometry data are not specified, the canal water-surface evaporation will not be calculated.

Canal-Geometry and Riverbed Hydraulic Parameter File

The canal-geometry and riverbed hydraulic parameter file is used for specifying canal cross-section data, riverbed hydraulic parameters, canal lengths, and canal-entry bottom elevations. These data are used mainly in canal routing for determining canal seepage and surface-water evaporation. These data are optional. Input instructions and explanations of variables are listed in table 21. The file-identification code is 3. All data specified in this file also can be included in the general network-configuration and parameter file (table 18).

Table 21. File format for canal geometry and riverbed hydraulic parameters (file-identification code 3 used in OPONDS computer program)

[ft, feet; ft/d, feet per day; --, not applicable]

State- ment num- ber	Information at statement	Variable	Definition	Variable type	Unit
1-5	Title lines	--	--	--	--
6	Canal geometry data	F_NODE	From-node name.	character	--
		T_NODE	To-node name.	character	--
		N	Canal roughness coefficient.	real	--
		L	Canal length.	real	ft
		J	Canal average slope.	real	--
		W	Canal bottom width.	real	ft
		M	Canal side slope.	real	--
		D	Canal maximum depth.	real	ft
		KY	Canal riverbed hydraulic conductivity.	real	ft/d
		THICK	Riverbed thickness.	real	ft
		ELEV	Riverbed elevation at entry.	real	ft

Outlet Hydraulic-Structure Parameter File

Outlet hydraulic structures included in the OPONDS program are rectangular sharp-crested weirs, vertical sluice gates on broad-crested weirs, vertical flat gates on spillways, and pipes. Flow through gated weirs and sharp-crested weirs is assumed to be controllable by adjusting gate opening heights or sharp-crested weir heights. The data needed and input instructions and explanations of variables are listed in table 22. The file-identification code is 4. All data specified in this file also can be included in the general network-configuration and parameter file (table 18).

Table 22. File format for outlet hydraulic-structure parameters (file-identification code 4 used in OPONDS computer program)

[ft, feet; --, not applicable]

State- ment num- ber	Information at statement	Variable	Definition	Variable type	Default value	Unit
1-5	Title lines	--	--	--	--	--
6	Information for each weir	NAME	Structure name,	character	--	--
		F_NODE	Nodal name (see note 4).	character	--	--
		T_NODE	Downstream nodal name (see note 4).	character	--	--
		TYPE	Structure type code (see note 1).	integer	--	--
		BELEV	Base elevation (see note 2).	real	--	ft
		WLEN	Weir length.	real	--	ft
		WHITE	Weir height if sharp-crested and broad-crested weir, or the design water head if standard spillway.	real	--	ft
		GHTE	Gate opening height.	real	--	ft
		WB	Weir width (thickness).	real	--	ft
	Information for each outlet pipe	NAME	Structure name.	character	--	--
		F_NODE	Nodal name (see note 4).	character	--	--
		T_NODE	Downstream nodal name (see note 4).	character	--	--
		TYPE	Structure type code (see note 1).	integer	--	--
		BELEV	Base elevation (see note 2).	real	--	ft
		WIDTH	Pipe diameter.	real	--	ft
		LENG	Pipe length (see note 3).	real	--	ft
		FRIC	Pipe-friction factor.	real	0.025	--
		ENLOS	Pipe-entrance loss factor.	real	0.5	--

Notes:

- Hydraulic-structure type code:
 - Rectangular sharp-crested weir ($0 < H/P < 5$).
 - Vertical flat gate on spillway. Gate edge is facing downstream.
 - Vertical sluice gate on broad-crested weir.
 - Pipe.
- Base elevation: (1) bottom of a weir if sharp-crested weir, (2) top of a weir if spillway or broad-crested weir, and (3) center of a pipe at entry if pipe.
- If sharp-crested weir height is less than zero (< 0), the weir height is adjustable, and the absolute value is the maximum height allowed.
- There is only one flow zone downstream from the structure. No extended flow zones are allowed.

Surface-Runoff Parameter File

Surface runoff is calculated using the SCS curve-number method (Soil Conservation Service, 1985). Data needed are drainage area, curve number for average condition, initial antecedent 5-day rainfall, and criteria for wet/dry conditions. Input instructions and explanations of variables are listed in table 23. The file-identification code is 5. All data specified in this file also can be included in the general network-configuration and parameter file (table 18).

Table 23. File format for surface-runoff parameters (file-identification code 5 used in OPONDS computer program)

[in., inches; --, not applicable]

State- ment num- ber	Information at statement	Variable	Definition	Variable type	Default value	Unit
1-5	Title lines	--	--	--	--	--
6	Data	NAME	Nodal name.	character	--	--
		A5DR0	Initial antecedent 5-day rainfall.	real	--	in.
		A5DRI	Antecedent 5-day rainfall for dry conditions (I) (see note 1).	real	0.5	in.
		A5DRIII	Antecedent 5-day rainfall for wet conditions (III) (see note 2).	real	1.1	in.
		SCSCN	SCS curve number for average conditions (II).	real	--	--
		AREA	Drainage area.	real	--	acres

Notes:

1. The suggested values are less than 0.5 in. for dormant season and less than 1.4 in. for growing season (Soil Conservation Service, 1985; McCuen, 1989).

2. The suggested values are greater than 1.1 in. for dormant season and greater than 2.1 in. for growing season (Soil Conservation Service, 1985; McCuen, 1989).

Pond Elevation-Volume-Area Relation File

If there is any pond operation involved, the pond elevation-volume-area relation file is used. The relations among water-surface elevation, volume, and water-surface area of ponds can be expressed either in tabular form or in the regression equations for the Quivira National Wildlife Refuge (tables 24 and 25). The file-identification code is 9.

Table 24. File format for relations among water-surface elevation (Z), volume (V), and water-surface area (A) of ponds (file-identification code 9 used in OPONDS computer program)

[ft; feet; acre-ft, acre-feet; --, not applicable]

State- ment num- ber	Information at statement	Variable	Definition	Variable type	Unit
1-5	Title lines	--	--	--	--
6	Data source	ZVAMTH	Data source index for Z-V-A data (= 0).	integer	--
7	Pond name	NAME	Pond nodal name.	character	--

Table 24. File format for relations among water-surface elevation (Z), volume (V), and water-surface area (A) of ponds (file-identification code 9 used in OPONDS computer program)—Continued

State- ment num- ber	Information at statement	Variable	Definition	Variable type	Unit
8	Pond character- istic curves among eleva- tion, capacity, and area	ELE	Water-surface elevation.	real	ft
		CAP	Water volume of a pond at the current ele- vation.	real	acre-ft
		AREA	Water-surface area.	real	acre
9	Empty line	--	Move to next pond node.		--
10	Finish	FINISH	Finish reading pond table.	character	--

Table 25. File format for regression relations of water-surface elevation (Z), volume (V), and water-surface area (A) of ponds (file-identification code 9 used in OPONDS computer program)

[ft, feet; --, not applicable]

State- ment num- ber	Information at statement	Variable	Definition	Variable type	Unit
1–5	Title lines	--	--	--	--
6	Data source	ZVAMTH	Data source index for Z-V-A data (= 1).	integer	--
7	Pond name	NAME	Pond nodal name.	character	--
8	Base and coeffi- cients for each regression equation	N	Equation sequential number.	integer	--
		ZB	Zonal elevation base.	real	ft
		A1	Coefficient A1.	real	--
		A2	Coefficient A2.	real	--
		A3	Coefficient A3.	real	--
9	Empty line	--	Move to next pond node.	--	--
10	Finish	FINISH	Finish reading coefficients of regression equations.	character	--

Seasonal Target Water-Demand File

Target water demand in the OPONDS program means that water will be withdrawn from a node; that is, water will be taken out of the canal and control-pond system. Input instructions and explanations of variables are listed in table 26. The file-identification code is 10. All data specified in this file also can be included in the general network-configuration and parameter file (table 18).

Table 26. File format for seasonal target water demands (file-identification code 10 used in OPONDS computer program)

[acre-ft, acre-feet; ft³/s, cubic feet per second; ft³/d, cubic feet per day; --, not applicable]

State- ment num- ber	Information at statement	Variable	Definition	Variable type
1–5	Title lines	--	--	--
6	Water-demand unit	WSUNIT	Water-demand unit code (0—acre-ft, 1—ft ³ /s, 2—ft ³ /d).	integer
7	List of nodal names	TIME NAME(I), I = 1, NWSND	Time step. Nodal names (see note 1). NWSND is the number of nodes with water demands.	character character
8	Target water demands for each season N, N = 1, NPER	TIME (WSTB (N,J), J = 1, NWSND)	Season. Seasonal target water demands.	character real

Note:

1. Use the nodal name DEFAULT for nodes with the same target water demands. The DEFAULT node must follow the other specified nodes (that is, in last column).

Seasonal Water-Surface Evaporation File

Seasonal water-surface evaporation from a pond node or canal segment is calculated using evaporation coefficients and water-surface area. Input instructions and explanations of variables for evaporation coefficients are listed in table 27. The file-identification code is 11. All data specified in this file also can be included in the general network-configuration and parameter file (table 18).

Table 27. File format for seasonal water-surface evaporation coefficients (file-identification code 11 used in OPONDS computer program)

[mm/d; millimeters per day; in/d, inches per day; ft/d, feet per day; --, not applicable]

State- ment num- ber	Information at statement	Variable	Definition	Variable type
1–5	Title lines	--	--	--
6	Unit	EVUNIT	Water-surface evaporation coefficient (0—mm/d; 1—in/d; 2—ft/d).	integer
7	List of nodal names	TIME (NAME(J), J = 1, NEV)	Time. Nodal names (see note 1). NEV is the number of nodes with evaporation coefficients.	character character

Table 27. File format for seasonal water-surface evaporation coefficients (file-identification code 11 used in OPONDS computer program)—Continued

State- ment num- ber	information at statement	Variable	Definition	Variable type
8	Evaporation coefficients for each sea- son N, N = 1, NPER	TIME (EVTB (N,J), J=1, NEV)	Season index. Evaporation coefficients.	character real

Note:

1. Use the nodal name DEFAULT for nodes with the same coefficients. The DEFAULT node must follow the other specified nodes (that is, in last column).

Seasonal Flow-Boundary File

The flow requirements in canals may be different for different seasons. In the linear network setting, these flow requirements are represented by flow boundaries in the associated arcs. Input instructions and explanations of variables for seasonal flow boundaries are listed in table 28. The file-identification code is 12. All data specified in this file can also be included in the general network-configuration and parameter file (table 18).

Table 28. File format for seasonal flow boundaries (file-identification code 12 used in OPONDS computer program)

[acre-ft, acre-feet; ft³/s, cubic feet per second; ft³/d, cubic feet per day; --, not applicable; >, greater than; <, less than]

State- ment num- ber	information at statement	Variable	Definition	Variable type
1-5	Title lines	--	--	--
6	Unit	FBUNIT	Flow unit code (0—acre-ft; 1—ft ³ /s; 2—ft ³ /d).	integer
7	List of upstream nodal names	(NAME(J),J = 1, NFBAR)	From-node names of arcs. NFBAR is the number of arcs with seasonal flow bound- aries.	character
8	List of down- stream nodal names	(NAME(J),J = 1, NFBAR)	To-node names of arcs.	character
9	Zone index	TIME, (FBIDX(J), J = 1, NFBAR)	Season. Flow-zone index (-1, lower boundary of normal flow zone; +1, upper boundary of normal flow zone; <-2, lower extended flow zone; >+2, upper extended flow zone; that is, $ndex = zone \pm$).	character integer
10	Flow boundaries for each season N, N = 1, NPER	TIME, (FBTB (N,J), J = 1, NFBAR), N = 1, NPER	Season. Seasonal flow boundary (see note 1).	character real

Note:

1. For lower or upper extended flow zones, flow boundaries are equal to flow-zone capacities.

Seasonal Pond Rule-Curve File

Management water levels in a pond may change seasonally. The seasonal water levels are represented by the different rule curves in the linear-network flow model. Input instructions and explanations of variables for seasonal rule curves are listed in table 29. The file-identification code is 13. All data specified in this file also can be included in the general network-configuration and parameter file (table 18).

Table 29. File format for seasonal pond rule curves (file-identification code 13 used in OPONDS computer program)

[ft, feet; in., inches; mm, millimeters; --, not applicable]

State- ment num- ber	Information at statement	Variable	Definition	Variable type
1–5	Title lines	--	--	--
6	Unit	RCUNIT	Rule-curve elevation unit code (0—ft; 1—in.; 2—mm).	integer
7	List of nodal names	TIME (NAME(J), J = 1, NRCND)	Time. Pond nodal names (see note 1). NRCND is the number of pond nodes with seasonal rule curves.	character character
8	Rule curves for each season N, N = 1, NPER	TIME (RCTB (N,J), J=1, NRCND)	Season. Rule-curve elevations.	character real

Note:

1. Use the nodal name DEFAULT for pond nodes with the same rule curves. The DEFAULT node must follow the other specified nodes (that is, in last column).

Local Net Incremental Inflow File

The local net incremental inflow file is used to specify nodes and their local net inflows. Local net incremental inflow is the water locally attributed to a node. Input instructions and explanations of variables for local net inflows are listed in table 30. The file-identification code is 16.

Table 30. File format for local net incremental inflows (file-identification code 16 used in OPONDS computer program)

[acre-ft, acre-feet; ft³/s, cubic feet per second; ft³/d, cubic feet per day; --, not applicable]

State- ment num- ber	Information at statement	Variable	Definition	Variable type
1–5	Title lines	--	--	--
6	Flow unit	IFWCD	Flow unit code (0—acre-ft; 1—ft ³ /s; 2—ft ³ /d).	integer
7	List of nodal names	TIME (NAME(J), J = 1, NIFW)	Time. Nodal names (see note 1). NIFW is the number of nodes with net incremental inflows.	character character

Table 30. File format for local net incremental inflows (file-identification code 16 used in OPONDS computer program)—Continued

State- ment num- ber	Information at statement	Variable	Definition	Variable type
8	Net inflows to node for each time period	TIME QIN(I), I = 1, NIFW	Time. Net inflow to nodes (see note 2).	character real

Notes:

1. Use the nodal name DEFAULT for nodes with the same net inflows. The DEFAULT node must follow the other specified nodes (that is, in last column).
2. Net inflow to a node can be either a positive or a negative value.

Precipitation File

The precipitation file is used to assign precipitation data to nodes. The precipitation data are used to calculate surface runoff to nodes. Input instructions and explanations of variables for precipitation data are listed in table 31. The file-identification code is 17.

Table 31. File format for precipitation data (file-identification code 17 used in OPONDS computer program)

[ft, feet; in., inches; mm, millimeters; acre-ft/d, acre-feet per day; ft³/s, cubic feet per second; ft³/d, cubic feet per day; --, not applicable]

State- ment num- ber	Information at statement	Variable	Definition	Variable type
1–5	Title lines	--	--	--
6	Data unit	RNUNIT	Data unit code (if rntype = 1, 0—ft, 1— in., 2—mm; if rntype = 2, 0—acre-ft/d, 1—ft ³ /s, 2—ft ³ /d).	integer
		RNTYPE	Date type index (1—depth; 2—flux).	integer
7	List of nodal names	TIME (NAME(J), J = 1, NRAIN)	Time. Nodal names (see note 1). NRAIN is the number of nodes with precipitation.	character character
8	Precipitation data for each time period	TIME RAIN(I), I = 1, NRAIN	Time. Precipitation depth or flux rate to nodes.	character real

Note:

1. Use the nodal name DEFAULT for nodes with the same amount of precipitation. The DEFAULT node must follow the other specified nodes (that is, in last column).

Time-Dependent, Water-Surface Evaporation File

In general, water-surface evaporation coefficients change with time. Even for the same season and the same place, the evaporation coefficients may be significantly different for different years. This file is designed to input time-dependent, water-surface evaporation coefficients for selected nodes. Input instructions and explanations of variables are listed in table 32. The file-identification code is 18.

Table 32. File format for time-dependent, water-surface evaporation coefficients (file-identification code 18 used in OPONDS computer program)

[mm/d; millimeters per day; in/d, inches per day; ft/d, feet per day; --, not applicable]

State- ment num- ber	information at statement	Variable	Definition	Variable type
1–5	Title lines	--	--	--
6	Date unit	EVUNIT	Evaporation coefficient unit code (0—mm/d, 1—in/d, 2 —ft/d).	integer
7	List of nodal names	TIME (NAME(J), J = 1, NEV)	Time. Nodal names (see note 1). NEV is the num- ber of nodes with water-surface evapora- tion coefficients.	character character
8	Water-surface evaporation for each time period	TIME (EVTB (0,J), J=1, NEV)	Time. Evaporation coefficients (see note 2).	character real

Notes:

1. Use the nodal name DEFAULT for nodes with the same coefficients. The DEFAULT node must follow the other specified nodes (that is, in last column).
2. If both seasonal and nonseasonal coefficients are specified for the same node, only nonseasonal coefficients are used.

Time-Dependent, Target Water-Demand File

The time-dependent, target water-demand file is used to input target water demands for selected nodes for which water withdrawals change with time. Input instructions and explanations of variables are listed in table 33. The file-identification code is 19.

Table 33. File format for time-dependent, target water demand (file-identification code 19 used in OPONDS computer program)

[acre-ft, acre-feet; ft³/s, cubic feet per second; ft³/d, cubic feet per day; --, not applicable]

State- ment num- ber	information at statement	Variable	Definition	Variable type
1–5	Title lines	--	--	--
6	Units of water demand	WSUNIT	Water-demand unit code (0—acre-ft; 1—ft ³ /s; 2—ft ³ /d).	integer

Table 33. File format for time-dependent, target water demand (file-identification code 19 used in OPONDS computer program)—Continued

State- ment num- ber	Information at statement	Variable	Definition	Variable type
7	List of nodal names	TIME NAME(I), I = 1, NWSND	Time step. Nodal names (see note 1). NWSND is the number of nodes with target water demands.	character character
8	Target water demand for each time period	TIME (WSTB (0,J), J = 1, NWSND)	Time. Target water demands (see note 2).	character real

Notes:

1. Use the nodal name DEFAULT for nodes with the same target water demands. The DEFAULT node must follow the other specified nodes (that is, in last column).
2. If both seasonal and nonseasonal values are specified for the same node, only nonseasonal values are used.

Time-Dependent, Pond Rule-Curve File

The target water-surface elevation of a pond changes not only seasonally but also with time (nonseasonal). It is assumed in the OPOND program that the top level of the upper zone and bottom level of the lower zone are kept unchanged. The rule curve of a pond changes between the top level of the upper zone and the bottom level of the lower zone with time. Input instructions and explanations of variables for rule-curve elevations are listed in table 34. The file-identification code is 20.

Table 34. File format for time-dependent, pond rule-curve elevations (file-identification code 20 used in OPONDS computer program)

[ft, feet; in., inches; mm, millimeters; --, not applicable]

State- ment num- ber	Information at statement	Variable	Definition	Variable type
1–5	Title lines	--	--	--
6	Unit	RCUNIT	Rule-curve elevation unit code (0—ft; 1—in.; 2—mm).	integer
7	List of nodal names	TIME NAME(J), J = 1, NRCND	Time. Pond nodal names (see note 1). NRCND is the number of nodes with time-dependent rule curves.	character character
8	Rule-curve ele- vations for each time period	TIME (RCTB (0,J), J=1, NRCND)	Time. Rule-curve elevations (see note 2).	character real

Notes:

1. Use the nodal name DEFAULT for nodes with the same pond rule curves. The DEFAULT node must follow the other specified nodes (that is, in last column).
2. If both seasonal and nonseasonal values are specified for the same node, only nonseasonal values are used.

Time-Dependent, Flow-Boundary File

The time-dependent, flow-boundary file is used to specify flow boundaries that change with time for selected flow zones in canals. Input instructions and explanations of variables are listed in table 35. The file-identification code is 21.

Table 35. File format for time-dependent flow boundaries (file-identification code 21 used in OPONDS computer program)

[acre-ft, acre-feet; ft³/s, cubic feet per second; ft³/d, cubic feet per day; --, not applicable; >, greater than; <, less than]

State- ment num- ber	Information at statement	Variable	Definition	Variable type
1-5	Title lines	--	--	--
6	Unit	FBUNIT	Flow unit code (0—acre-ft; 1—ft ³ /s; 2—ft ³ /d).	integer
7	List of upstream nodal names	NAME(J), J = 1, NFBAR	From-node names. NFBAR is the number of arcs with flow boundaries.	character
8	List of down- stream nodal names	NAME(J), J = 1, NFBAR	To-node names.	character
9	Flow-zone index	TIME, (FBIDX(J), J = 1, NFBAR)	Time. Flow-zone index (-1, lower boundary of normal flow zone; +1, upper boundary of normal flow zone; <-2, lower extended flow zone; >+2, upper extended flow zone; that is, $index = zone \pm 1$).	character integer
10	Flow boundaries for each time period	TIME, (FBTB(0,J), J = 1, NFBAR),	Time. Flow boundaries (see notes 1 and 2).	character real

Notes:

1. For lower or upper extended flow zone, set flow boundaries equal to corresponding flow-zone capacities.
2. If both seasonal and nonseasonal values are specified for the same flow zone in the same flow arc, only nonseasonal values are used.

Ground-Water Elevation File

Ground-water data are used to estimate seepage from ponds and canals. Ground-water elevations are conceptually specified at nodes. Input instructions and explanations of variables for ground-water data are described in table 36. The file-identification code is 22.

Table 36. File format for ground-water elevations (file-identification code 22 used in OPONDS computer program)

[ft, feet; in., inches; mm, millimeters; acre-ft/d, acre-feet per day; ft³/s, cubic feet per second; ft³/d, cubic feet per day; --, not applicable]

State- ment num- ber	Information at statement	Variable	Definition	Variable type
1-5	Title lines	--	--	--
6	Data unit and type	GWUNIT	Data unit code (if gwtype = 1, 0—ft; 1—in.; 2—mm; if gwtype = 2, 0—acre-ft/d; 1—ft ³ /s; 2—ft ³ /d).	integer
		GWTYPE	Data type code (1—level; 2—flux).	integer
7	List of nodal names	TIME (NAME(J), J = 1, NGWND)	Time. Nodal names (see note 1). NGWND is the number of nodes with ground-water data.	character character
8	Ground-water level or flux for each time period	TIME, GWLVL(I), I = 1, NGWND	Time. Ground-water elevation or flux (see note 2).	character real

Notes:

1. Use the nodal name DEFAULT for nodes with the same values. The DEFAULT node must follow the other specified nodes (that is, in last column).
2. If the flux is a negative value, it means the pond gains water from an aquifer.

Fixed-Flow File

One special case of canal flows is when flows are fixed to a certain amount for a given time period. This implies that there are no extended flow zones and that flow in the normal flow zone is constant in the network model. The fixed-flow file is used to specify fixed flows for selected canals. Input instructions and explanations of variables are described in table 37. The file-identification code is 23.

Table 37. File format for fixed flows (file-identification code 23 used in OPONDS computer program)

[acre-ft, acre-feet; ft³/s, cubic feet per second; ft³/d, cubic feet per day; --, not applicable]

State- ment num- ber	Information at statement	Variable	Definition	Variable type
1-5	Title lines	--	--	--
6	Flow unit	FIXCD	Flow unit code (0—acre-ft; 1—ft ³ /s; 2—ft ³ /d).	integer
7	List of upstream nodes	(NAME(J), J = 1, NFIX)	Upstream nodal names. NFIX is the number of arcs with fixed flows.	character
8	List of down- stream nodes	TIME NAME(J), J = 1, NFIX	Time. Downstream nodal names.	character character

Table 37. File format for fixed flows (file-identification code 23 used in OPONDS computer program)—Continued

State- ment num- ber	Information at statement	Variable	Definition	Variable type
9	Fixed flow for each time period	TIME Q(I), I = 1, NFIX	Time. Fixed flow.	character real

Network-Configuration Output File

The network-configuration output file is used to summarize input data and the basic network configuration and to store error messages during program execution. The file-identification code is 26. The output file name with the file-identification code 26 must be specified in the master data file. See a sample output in Appendix D for the sample problem.

Nodal-Budget Output File

The nodal-budget output file is used to store nodal budgets for each time period. The output for general nodes includes upstream inflows, local net inflows, surface runoff, water withdrawals, and total outflows from nodes. In addition to these budget terms, budget items for initial storage, surface-water evaporation, bottom seepage, and final storage are added for the pond-budget output. The file-identification code is 27. The output file name with the file-identification code 27 must be specified in the master data file. If the file name is not specified, no nodal water-budget output will be generated.

Canal-Routing Output File

The canal-routing output file summarizes the canal-routing results, which include canal initial storage, inflow, canal seepage, water-surface evaporation, canal final storage, and outflow from each canal. The file-identification code is 28. The output file name with the file-identification code 28 must be specified in the master data file (see tables 16 and 17). If the file name is not specified, no canal water-budget output will be generated.

Outlet Hydraulic-Structure Output File

The output file includes outlet hydraulic-structure information, flow through structure, and structure operation. The structure information includes structure name, type, location, and size. Structure operation means the gate opening height or sharp-crested weir height. The file-identification code is 29. The output file name with the file-identification code 29 must be specified in the master data file (see tables 16 and 17).

List of Nodes for Time-Series Output of Water Budget

This file is used to list selected nodes for which time-series output of water budgets are needed. For the nodal listing, the file-identification code is 30. To specify a node, one nodal name is one record in the file. Two time-series output files for each of the specified nodes are generated. The first output file is for the nodal water budget with the output file as “nb_{nodal_name}.dat”, where {nodal_name} is the specified nodal name. The second output file is for downstream releases from a specified node. The name of this output file is “nr_{nodal_name}.dat”. If the pro-

gram runs on PC MS-DOS, nodal names are limited to five characters because a file name in PC DOS is limited to eight characters.

List of Canals for Time-Series Output of Canal-Routing Results

This file is used to list selected canals for which time-series output of routing results are needed. For the canal listing, the file-identification code is 31. A canal is represented by upstream and downstream nodal names. One record is specified for each selected canal in the file. The time-series output of the water budget will be saved in separated files for each selected canal. The output file name is of the form “arbud###.out”, where ### is the sequential number of selected canals in the file.

APPENDIX C. LIST OF SELECTED VARIABLES AND THEIR DEFINITIONS

Selected variables from the OPONDS program and their definitions are listed along with the variable type, which is presented in parentheses. If a variable is an array, the array dimension is also presented in parentheses.

ARBFLG—Logical indicator for arc budget output (LOGICAL).

ARC—Index variable for the current arc (INTEGER).

ARCBUD—List of budget terms for canal routing (unsigned) (**LDARC X 0:6**; INTEGER).

0—Downstream node of a canal reach.

1—Inflow.

2—Initial storage.

3—Canal seepage.

4—Canal water-surface evaporation.

5—Final storage.

6—Outflow.

ARCS—Number of arcs in a basic network (that is, number of physical arcs in the original network) (INTEGER).

ARTYP—List of arc types (**LDARC X 1**; INTEGER).

1—Canal-flow arc: abcd.

a = + if normal or upper extended flow zone;

- if lower extended flow zone.

b = 1.

c = 0 if not a stream arc;

1 if a stream arc.

d = 0 if normal flow zone:

> 0 if extended flow zone number.

2—Pond-storage arc:

a = + upper storage zone;

- lower storage zone.

b = 2.

c = 0.

d = zone number (1, 2, ...).

3—Pond net value arc.

4—Pond evaporation arc or seepage arc.

5—Surface-runoff arc.

6—Water-demand arc.

7—Local inflow arc.

8—Canal initial-storage arc.

9—Canal final-storage arc.

10—Canal-seepage arc.

11—Canal-evaporation arc.

CARBT—Cumulative arc budgets for whole system (6 x 1; REAL).

1—Inflow.

2—Initial storage.

3—Canal seepage.

4—Canal water-surface evaporation.

5—Final storage.

6—Outflow from the system.

CNDBT—Cumulative nodal budget for whole system (0:10 x 1; REAL).

0—Initial storage (acre-feet).

1—Upstream inflow (acre-feet).

2—Local inflow (acre-feet).

3—Evaporation loss for a pond node (acre-feet).

4—Precipitation gain (acre-feet).
 5—Seepage loss for pond node (acre-feet).
 6—Water withdrawal (acre-feet).
 7—Total downstream release (acre-feet).
 8—Final storage for pond node (acre-feet).
COLSTR—Array of strings for general purposes (**LDCOL** X 1; CHARACTER*30).
CONST—Constant coefficient converting acre-feet per month to cubic feet per second ($= 86,400 / 43,560 = 1.98347$) (REAL).
COST—Array of penalty coefficients of arcs (**LDARC** X 1; INTEGER).
CSARBT—Array of cumulative water budgets for arcs (**LDARC** x 0:6; REAL).
 0—Downstream node of a canal reach.
 1—Inflow.
 2—Initial storage.
 3—Canal seepage.
 4—Canal water-surface evaporation.
 5—Final storage.
 6—Outflow.
CSNDBT—Array of cumulative nodal budgets for nodes (**LDND** x 0:10; REAL).
 0—Initial storage (acre-feet).
 1—Upstream inflow (acre-feet).
 2—Local inflow (acre-feet).
 3—Evaporation loss for pond node (acre-feet).
 4—Precipitation gain (acre-feet).
 5—Seepage loss for pond node (acre-feet).
 6—Water withdrawal (acre-feet).
 7—Total downstream release (acre-feet).
 8—Final storage for pond node (acre-feet).
CTARFW—List of control arc and lower and upper flow boundaries (**LDCTAR** X 3; INTEGER).
 0—Control arc number.
 1—Lower flow boundary.
 2—Upper flow boundary.
CTERM—A temporary string (char*500).
DEBUG—Logical variable for optimal solution (LOGICAL).
ERR—Error flag (LOGICAL).
EVFIL—Logical indicator for time-dependent evaporation from a file (LOGICAL).
EVFLAG—Logical indicator for pond water-surface evaporation (LOGICAL).
EVND—List of nodes with water-surface evaporation (**LDEV** X 3; INTEGER).
 1—Nodal number.
 2—Data unit code (0—millimeter per day; 1—inches per day; 2—feet per day).
 3—Sequential number for time-dependent data.
 If zero, seasonal data will be used. If negative, the default value is used.
EVTB—Array of seasonal water-surface evaporation coefficients (0:**LDP** X **LDEV**; REAL).
EVUNIT—Evaporation coefficient unit code (INTEGER).
FBAR—List of flow-boundary arc information (0:7 X **LDFBAR**; INTEGER).
 0—Signed flow-boundary arc.
 1—Upstream node.
 2—Downstream node.
 3—Flow-zone index. index = zone number ± 1 .
 (-1 lower boundary of normal flow range;
 +1 upper boundary of normal flow range;
 ≤ -2 lower extended flow zone;

- >= 2 upper extended flow zone).
- 4—Next extended arc 1.
- 5—Next extended arc 2.
- 6—Data unit code (0—acre-feet; 1—cubic feet per second; 2—cubic feet per day).
- 7—Seasonal or time-dependent index (0—seasonal, >0—time dependent).

FBFIL—Logical indicator for time-dependent flow boundary from a file (LOGICAL).

FBFLAG—Logical indicator for existing flow-boundary arcs (LOGICAL).

FBTB—List of seasonal or time-dependent flow boundaries for selected arcs (0:**LDFBTB** x **LDFBAR**; REAL).

FBUNIT—Unit code for flow boundaries (INTEGER).

- (0—acre-feet; 1—cubic feet per second; 2—cubic feet per day).

FCRIT—Flow-convergence criterion (INTEGER).

FILNAM—List of data and output file names (0:**LDFIL** X 1; CHAR*30).

FLAG—Logical indicator (LOGICAL).

FLOW—Array of flows in arcs (**LDARC** X 1; INTEGER).

FLWARC—Logical indicator for canal routing (LOGICAL).

FWFLAG—Indicator for existing local incremental inflow (LOGICAL).

FXAR—List of fixed-flow arc and associated upstream and downstream nodes (0:2 X **LDFXAR**; INTEGER).

- 0—Arc number.
- 1—Upstream node.
- 2—Downstream node.

FXFLAG—Logical variable for fixed flow (LOGICAL).

FXUNIT—Fixed-flow unit code (INTEGER).

- (0—acre-feet; 1—cubic feet per second; 2—cubic feet per day).

GWFLAG—Logical indicator for ground-water discharge (LOGICAL).

GWLVL—List of ground-water elevations (**LDGWND** X 1; REAL).

GWND—List of nodes with ground-water seepage (**LDGWND** X 3; INTEGER).

- 1—Nodal number.
- 2—Data unit code (0—feet; 1—inches; 2—millimeters).
- 3—Sequential number for time-dependent data. If zero, seasonal data will be used.

GWTYPE—Ground-water data type (INTEGER).

- (1—elevation; 2—flux).

GWUNIT—Ground-water-data unit (0:2 x2; CHARACTER*6).

HI—Array of upper flow boundaries of arcs (**LDARC** X 1; INTEGER).

HYBFLG—Logical indicator for outputting water budgets through outlet hydraulic structures (LOGICAL).

HYDAT—List of hydraulic-structure parameters (**LDHY** X 5; REAL).

- 1—Base elevation (feet).
- 2—Weir length or pipe diameter (feet).
- 3—Weir height or pipe length (feet).
- 4—Gate opening height (feet) or pipe-friction factor.
- 5—Pipe-loss factor in constrictions and entrances.

HYDIR—Array for structure and associated upstream and downstream nodal names (**LDHY** X 0:2; CHAR*12).

- 0—Structure name.
- 1—Upstream nodal name.
- 2—Downstream nodal name.

HYOUT—Array for results for hydraulic-structure operation (**LDHY** X 3; REAL).

- 1—Flow-through structure (cubic feet per second).
- 2—Water level of an upstream node (feet).
- 3—Gate opening height or weir height (feet).

HYTP—Array of names of hydraulic-structure types available (0:**LDHYTP** X 1; CHAR*20).

HYTPCD—Array of structure type codes and downstream arcs (**LDHY** X 0:1; INTEGER).

- 0—Structure type code.
- 1—Rectangle sharp-crested weir.
- 2—Vertical flat gate on spillway.

- 3—Vertical flat gate on broad-crested weir.
- 6—Pipe.
- 1—Downstream arc number.

I—Index variable (INTEGER).

IFault—Index variable for fault code (INTEGER).

IFWCD—Local net inflow unit code (INTEGER).

- 0—Acre-feet.

- 1—Cubic feet per second (cubic feet per second).

- 2—Cubic feet per day (cubic feet per day).

IFWND—List of nodes with local incremental flows (**LDIFW** X 3; INTEGER).

- 1—Nodal number.

- 2—Data unit code (0—acre-feet; 1—cubic feet per second; 2—cubic feet per day).

- 3—Sequential number for time-dependent data. IF < 0, use default value.

II—An array of upstream nodes for directed arcs (**LDARC** X 1; INTEGER).

IN—FORTRAN file unit for general network data file (INTEGER).

INST—Array of initial storages at each time step (**LDRES** X 1; REAL).

IN_EV—FORTRAN file unit for evaporation coefficient (INTEGER).

IN_FB—FORTRAN file unit for time-dependent flow boundaries (INTEGER).

IN_FX—FORTRAN file unit for fixed-flow data (INTEGER).

IN_GW—FORTRAN file unit for ground-water-level data (INTEGER).

IN_IFW—FORTRAN file unit for local incremental inflow (INTEGER).

IN_RC—FORTRAN file unit for time-dependent rule curve (INTEGER).

IN_RN—FORTRAN file unit for precipitation data (INTEGER).

IN_WS—FORTRAN file unit for target water-demand data (INTEGER).

ISGN—INTEGER function for the sign of an INTEGER (INTEGER).

J—Index variable (INTEGER).

JJ—Array of downstream nodes of directed arcs (**LDARC** X 1; INTEGER).

KARC—The current out-of-kilter arc (INTEGER).

LAST—Logical indicator at the ending of a file (LOGICAL).

LDARC—Leading dimension for number of arcs in a network (INTEGER).

LDCOL—Leading dimension for the array **COLSTR** (INTEGER).

LDCTAR—Leading dimension for number of control arcs (INTEGER).

LDEV—Leading dimension for number of nodes with water-surface evaporation (INTEGER).

LDFBAR—Leading dimension for flow-boundary arcs (INTEGER).

LDFBTB—Leading dimension for flow-boundary data matrix (INTEGER).

LDFIL—Leading dimension for number of input-output files (INTEGER).

LDFXAR—Leading dimension for number of fixed-flow arcs (INTEGER).

LDGWND—Leading dimension for number of nodes with ground-water data (INTEGER).

LDHY—Leading dimension for number of hydraulic structures (INTEGER).

LDHYTP—Leading dimension for number of outlet-structure types (INTEGER).

LDIFW—Leading dimension for number of nodes with local incremental inflow (INTEGER).

LDITR—Leading dimension for number of iterations (INTEGER).

LDND—Leading dimension for number of nodes (INTEGER).

LDP—Leading dimension for number of seasons in a year (INTEGER).

LDPL—Leading dimension for pool space (INTEGER).

LDRAIN—Leading dimension for number of nodes with precipitation (INTEGER).

LDRC—Leading dimension for number of nodes with time-dependent rule curves (INTEGER).

LDRCTB—Leading dimension for rule-curve table (INTEGER).

LDREAR—Leading dimension for number of pond arcs (INTEGER).

LDRES—Leading dimension for number of pond nodes (INTEGER).

LDRETB—Leading dimension for number of pond-characteristic table (INTEGER).

LDRFTB—Leading dimension for runoff data table **RNOFTB** (INTEGER).
LDRNOF—Leading dimension for number of nodes with the surface-runoff data (INTEGER).
LDSABL—Leading dimension for number of single-arc budget lists (INTEGER).
LDSNBL—Leading dimension for number of single-node budget lists (INTEGER).
LDSTR—Leading dimension for number of stream arcs (INTEGER).
LDSTRM—Leading dimension for number of stream-routing arcs (INTEGER).
LDUNIT—Leading dimension for number of units used (INTEGER).
LDWS—Leading dimension for number of water-demand nodes (INTEGER).
LO—Array of lower flow boundaries (**LDARC X 1**; INTEGER).

MNTH—Current season in characters (CHARACTER*5).
MTH—Current season in numbers such as 1, 2, ... (INTEGER).
MXCST—Maximum value of penalty coefficients (INTEGER).

NARCND—Number of arc nodes (that is, stream nodes) (INTEGER).
NARCS—Number of arcs in a network (INTEGER).
NDBFLG—Logical indicator for outputting the nodal water budgets (LOGICAL).
NDDWAR—Array of downstream flow arcs from nodes (**LDARC X 1**; INTEGER).
NDNAM—List of nodal names (**LDND X 1**; CHAR*12).
NDSEQ—List of nodal sequential numbers (**LDND X 1**; INTEGER).
NDTYP—List of nodal types (**LDND X 1**; INTEGER).

- 1—Pond node.
- 2—General node.

NDWAR—Number of downstream arcs (INTEGER).
NDXAR—List of signed extensional arcs associated with a node (**LDND X 6**; INTEGER).

- 1—Pond net-value arc or local net inflow arc.
- 2—Water-surface-evaporation arc.
- 3—Precipitation arc.
- 4—Pond-seepage arc.
- 5—Target water-demand arc.
- 6—Target water-demand deviation arc.

NEV—Number of evaporation nodes (INTEGER).
NFBAR—Number of flow-boundary arcs (INTEGER).
NFXAR—Number of arcs with time-dependent, fixed flows (INTEGER).
NGWND—Number of ground-water nodes (INTEGER).
NHY—Number of hydraulic structures (INTEGER).
NHYTP—Number of outlet structure types supported (INTEGER).
NIFW—Number of nodes with local incremental inflows (INTEGER).
NITR—Number of iteration steps (INTEGER).
NNODS—Number of active nodes in a network (that is, original nodes) (INTEGER).
NODBUD—List of budget terms for nodal budgets (**LDND X 0:10**; INTEGER).

- 0—Initial storage for a pond node (acre-feet).
- 1—Upstream inflow (acre-feet).
- 2—Local inflow (acre-feet).
- 3—Evaporation loss for pond node (acre-feet).
- 4—Precipitation runoff (acre-feet).
- 5—Seepage loss for pond node (acre-feet).
- 6—Water withdrawal (acre-feet).
- 7—Total downstream release (acre-feet).
- 8—Final storage for pond node (acre-feet).

NOP—Index for the current time period (INTEGER).
NOTCOV—Logical convergence indicator (LOGICAL).
NPER—Number of seasons in a year (INTEGER).
NRRAIN—Number of nodes with precipitation (INTEGER).
NRCND—Number of nodes with time-dependent, rule curves (INTEGER).

NRES—Number of pond nodes (INTEGER).
NRNOF—Number of surface-runoff nodes (INTEGER).
NSABL—Number of arcs needed to output single-arc budget (INTEGER).
NSNBL—Number of nodes needed to output nodal budget (INTEGER).
NSPS—Number of simulation periods (INTEGER).
NSTR—Number of stream reaches with geometrical data (INTEGER).
NSTRM—Number of stream-routing arcs (INTEGER).
NTLS—Number of non-data lines in a data file (INTEGER).
NWSND—The number of water-demand nodes (INTEGER).

OARCS—Old arc (INTEGER).
OCF—Array of old stream coefficients (canal storage) (**LDSTRM** X 1; REAL).
OFLOW—Array of old flows (**LDARC** X 1; INTEGER).
OHI—Array of old high boundaries of arcs (**LDARC** X 1; INTEGER).
OINST—List of old initial pond storages (**LDRES** X 1; REAL).
OU_AR—FORTRAN unit for outputting arc budgets (INTEGER).
OU_HY—FORTRAN unit for outputting operating hydraulic structures (INTEGER).
OU_ND—FORTRAN unit for outputting nodal budgets (INTEGER).
OU_NT—FORTRAN unit for outputting network configuration (INTEGER).
OU_SA—FORTRAN unit for file listing arcs for which single-arc budgets are needed (INTEGER).
OU_SN—FORTRAN unit for file listing nodes for which single-node budgets are needed (INTEGER).

PERD—Number of days for each time period (REAL).
PN—Part number (INTEGER).
PTDWAR—Pointer of downstream arcs from a node (**LDND** X 2; INTEGER).
 1—Pointer for first downstream arc.
 2—Pointer for the last downstream arc.
PTRE—Pointer of pond-storage arcs for pond nodes (**LDRES** X 1; INTEGER).
PTRES—Pointer of pond nodes (that is, list of pond nodes) (**LDRES** X 1; INTEGER).

RAINND—List of nodes with precipitation (**LDRAIN** X 3; INTEGER).
 1—Nodal number.
 2—Data unit code (0—feet; 1—inches; 2—millimeters).
 3—Sequential number for time-dependent data. If zero, seasonal data will be used.

RAINTY—Surface-runoff data type (INTEGER).
 (1—precipitation; 2—flux)

RC—Array of pond rule curves in volume (**LDRES** X 1; REAL).
RCFIL—Logical indicator for time-dependent, rule curves from a file (LOGICAL).
RCFLAG—Logical indicator for time-dependent, rule curves (LOGICAL).
RCND—List of nodes with time-dependent, rule curves (**LDRC** X 3; INTEGER).
 1—Nodal number.
 2—Data unit code (0—feet; 1—inches; 2—millimeters).
 3—Sequential number for time-dependent data. If zero, seasonal data will be used.

RCTB—Array of time-dependent, rule curves (0:**LDRCTB** X **LDRC**; INTEGER).
RCUNIT—Rule-curve unit code (INTEGER).
REAR—Array of pond-storage arcs (**LDREAR** X 1; INTEGER).
RESDAT—Pond-bottom information (**LDRES** X 0:2; REAL).
 0—Pond-bottom elevation (feet).
 1—Pond-bed hydraulic conductivity (feet per day).
 2—Pond-bed thickness (feet).

REZN—Array of pond-storage zone capacities (**LDREAR** X 1; REAL).
RNFLAG—Indicator for existing rain data (LOGICAL).
RNOFND—List of nodes with surface runoff (**LDRNOF** X 1; INTEGER).
RNOFTB—Array of surface-runoff data (**LDRNOF** X 0:**LDRFTB**; REAL).
 0—Initial 5-day antecedent rainfall, in inches.

- 1—5-day antecedent rainfall for dry condition, in inches.
- 2—5-day antecedent rainfall for wet condition, in inches.
- 3—Drainage area, in acres.
- 4—SCS curve number for average condition (II).

RNUNIT—Precipitation unit code (INTEGER).

(0—feet; 1—inches; 3—millimeters)

RPOOL—Pool space (**LDPL X 1**; REAL).

RTERM—Temporary REAL number (REAL).

SABLFG—Logical indicator for single-arc budget list (LOGICAL).

SABLND—List of arcs with single-arc budget list (**LDSABL X 3**; INTEGER).

- 1—Upstream node number.
- 2—Downstream node number.
- 3—FORTRAN unit for output budget.

SAVOPT—Save option for general output (INTEGER).

- 0—Both input data-file information and network configuration.
- 1—Input data-file information.
- 2—Network configuration.

SKSC—Sink/source node (INTEGER).

SNBLFG—Logical indicator for single-node budget list (LOGICAL).

SNBLND—List of nodes with single-node budget list (**LDSNBL X 3**; INTEGER).

- 1—Nodal number.
- 2—FORTRAN unit for water-budget output.
- 3—FORTRAN unit for downstream water releases.

STMO—Starting season of simulation period in number (INTEGER).

STRDAT—List of canal-geometry data (**LDSTR X 9**; REAL).

- 1—Canal-bed roughness (dimensionless).
- 2—Canal-reach length (feet).
- 3—Average canal-bed slope (dimensionless).
- 4—Canal-base width (feet).
- 5—Canal-side slope (dimensionless).
- 6—Canal depth (feet).
- 7—Canal-bed hydraulic conductivity (feet per day).
- 8—Thickness of canal bed (feet).
- 9—Canal-bed elevation at reach entry (feet above sea level).

STRDIR—List of nodes associated with stream arcs (**LDSTR X 3**; INTEGER).

- 1—Upstream node.
- 2—Downstream node.
- 3—Middle routing node.

STRMAR—List of arcs for canal routing. Arc number is signed (**LDSTRM X 0:6**; INTEGER).

- 0—Stream nodal number.
- 1—Signed inflow arc number.
- 2—Signed initial canal-storage arc number.
- 3—Canal-seepage arc number.
- 4—Evaporation arc.
- 5—Final storage arc.
- 6—Outflow arc.

STRMCF—List of canal-routing coefficients (**LDSTRM X 0:4**; REAL).

- 0—Canal initial storage (acre-feet).
- 1—Traveltime (*K*) (days).
- 2—Weighting factor (*x*) (0–0.5, dimensionless).
- 3—Seepage coefficient (dimensionless).
 - > 0, in percentage of inflow (<1.0, 0.01 ~ 0.02).
 - < 0, seepage will be calculated by Darcy's law.
- 4—Evaporation coefficient (inches per day).

SYSNAM—Name of a system of ponds (CHAR*30).

UNITNM_1—Unit name for discharge (0:2 X 1; CHAR*6).

0—acre-feet.

1—cubic feet per second.

2—cubic feet per day.

UNITNM_2—Unit name for length (0:2 X 1; CHAR*6).

0—feet.

1—inches.

2—millimeters.

UNITNM_3—Unit name for rate (0:2 X 1; CHAR*6).

0—millimeters per day.

1—inches per day.

2—feet per day.

WSFIL—Logical indicator for target water demand from a file (LOGICAL).

WSFLAG—Logical indicator for target water demand (LOGICAL).

WSND—List of nodes with target water demand (LDWS X 3; INTEGER).

1—Nodal number.

2—Data unit code (0—acre-feet; 1—cubic feet per second; 2—cubic feet per day).

3—Sequential number for time-dependent data. If zero, seasonal data will be used.

WSTB—Array of time-dependent or seasonal target water demands (0:LDP X LDWS; REAL).

WSUNIT—Target water-demand unit code (INTEGER).

(0—acre-feet; 1—cubic feet per second; 2—cubic feet per day).

XF—Exaggerator factor (REAL).

XP—Number of decimal points for water budget, in acre-feet (INTEGER).

YEAR—Current year (INTEGER).

YES—Logical variable for yes or no (LOGICAL).

YR—Year (INTEGER).

ZEROFG—Logical indicator for zero (LOGICAL).

APPENDIX D. CONCEPTUAL SAMPLE PROBLEM

The conceptual sample problem is intended to illustrate the use of the computer program OPONDS for operation of a system of canals and control ponds, especially input data files and output from the program. The conceptual flow system used in this sample problem contains three ponds, three pumping stations for water supply, and three hydraulic structures to control water releases from the ponds (fig. 17). The sample problem specifies that three ponds should be operated to satisfy the downstream water demands at three pumping stations and the minimum flow requirements in channels and to keep pond water levels as close to target levels as possible. In the following section, the input data files for the sample problem are discussed first, and then output files are presented. The data in this sample problem are hypothetical.

Input Data for Conceptual Sample Problem

There are 18 input data files for this sample problem. Contents of these files are described in the following section. For the file format and variable definitions, refer to the input/output instructions in Appendix B.

Master Data File (mdf.dat)

The master data file is used to list all file names for input and output and their associated file-identification codes. The contents of master data file for the sample problem are listed as follows:

List of sample data files for the sample problem

File code	File name	File Description
0	nsim.dat	: Network-configuration file
1	pdzn.dat	: Pond-zoning file
2	ntwk.dat	: Network flow-configuration file
3	geom.dat	: Canal-geometry file
4	hydr.dat	: Hydraulic-structure file
5	rnof.dat	: Runoff-related data file
9	pzva.dat	: Pond-characteristics table file
10	tws.dat	: Pond seasonal target water-demand file
11	pet.dat	: Pond seasonal water-surface evaporation file
13	rc.dat	: Seasonal rule-curve file
16	ifw.dat	: Local incremental flow file
17	rain.dat	: Precipitation file
18	pet2.dat	: Water-surface evaporation file
19	tws2.dat	: Target water-demand file
20	rc2.dat	: Rule-curve data
22	gw1.dat	: Ground-water-level file
26	nsim.out	: Network-configuration output
27	ndbt.out	: Nodal-budget output
28	arbt.out	: Arc-budget output
29	hydr.out	: Flow-through hydraulic-structure output
30	snbl.dat	: List of nodes for time-series output
31	sabl.dat	: List of arcs for time-series output

General Network-Configuration and Parameter File (nsim.dat)

This file is used to specify the simulation period, accuracy of results, and so forth. It is noted that data for network construction and seasonal data also can be included in this file.

Operation of a river system

This is a sample problem.

```
30.42 12      ! Time interval, and number of time steps in a year
10 1996      ! Starting time period and year.
12          ! Number of simulation periods.
0.1 100      ! Flow-convergence criterion and maximum iteration steps.
2          ! Output accuracy. Number of decimal point in acre-feet.
0          ! Output option.
```

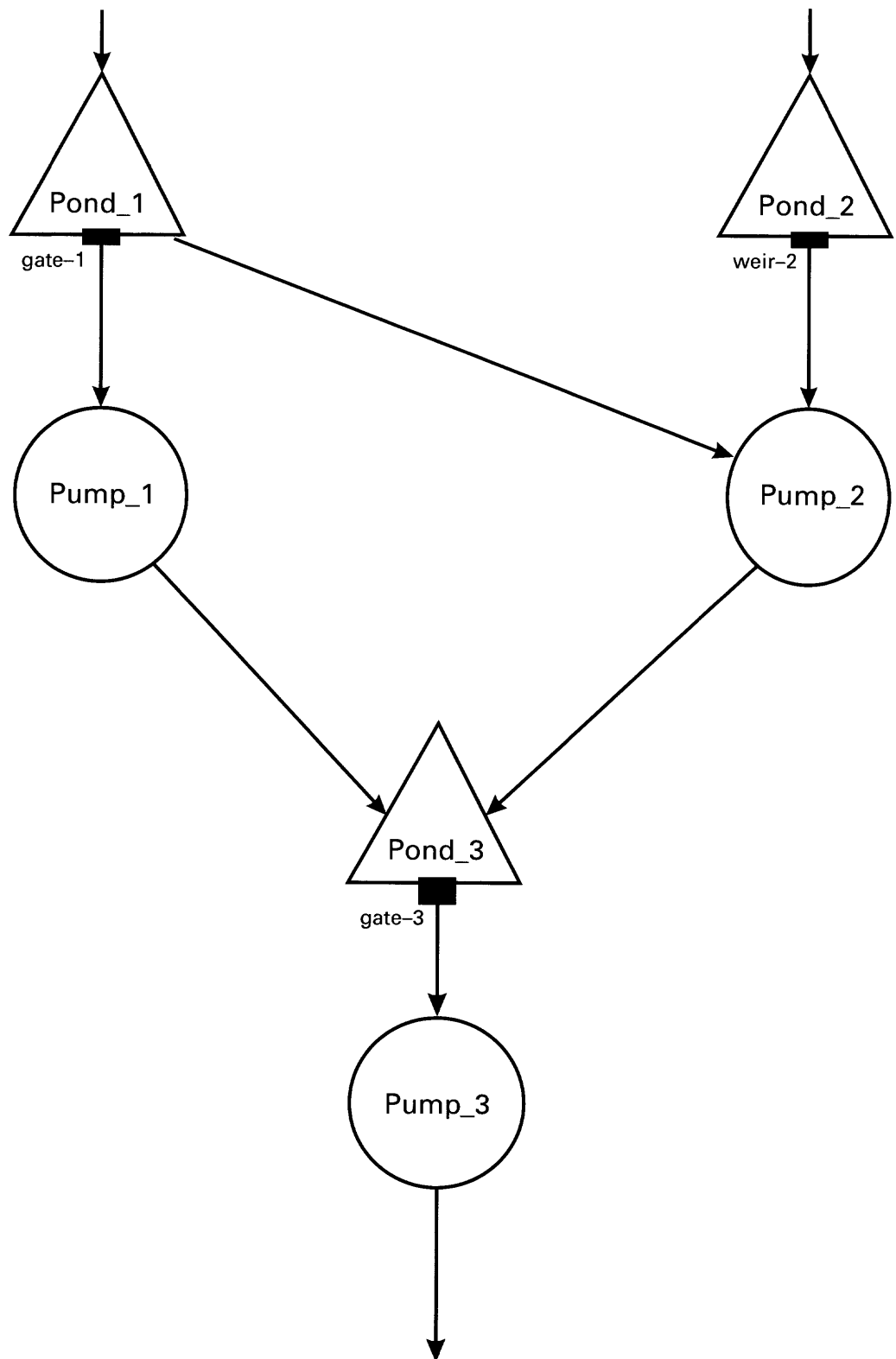


Figure 17. Network representation of flow system for sample problem.

Pond Rule-Curve and Zoning File (pdzn.dat)

Storages of pond_1 and pond_2 are divided into two storage zones (lower zone and upper zone). However, the storage of pond_3 is divided into three zones (lower zone, upper zone, and extended upper zone). The rule curves for the three ponds are located at the top of the lower zones. Pond-bottom hydraulic data also are included in this data file. It is noted that units of storage zone, rule curve, and initial storage must be the same. The units can be either feet or acre-feet and are specified in the unit code.

Sample pond rule curve and zoning.

Name	Unit	Initial elevation (feet)	Bottom Hydraulic elevation (feet)	cond. (ft/d)	Bottom thickness (feet)	Rule-curve elev. (feet)	Zone_1 elev. (feet)	Zone_1 coef.	Zone_2 elev. (feet)	Zone_2 coef.	Zone_3 elev. (feet)	Zone_3 coef.
pond_1	0	1274.00	1239.0	0.	1.0	1274.00	1240.00	100	1280.00	1000		
POND_2	0	1350.50	1308.0	0.0001	1.0	1350.50	1320.00	100	1365.00	1000		
POND_3	0	1039.00	1010.0	0.0001	1.0	1039.00	1020.00	100	1050.00	1000	1060.00	2000

Network Flow Configuration File (ntwk.dat)

There are three flow zones from pond_1 node to pump_1 node. Canal-routing coefficients, seepage coefficients, and water-surface-evaporation coefficients also are included in this file.

Network flow configuration for sample problem

F_node	T_node	Lower boundary (ft ³ /s)	Upper boundary (ft ³ /s)	Cost coef.	Initial storage (acre-ft)	Travel time (day)	Weight factor	Seepage coef.	Evap. coef. (in/d)
pond_1	PUMP_1	0.	16000.	10	0.0	1	0.2	0.01	0.0
pond_1	PUMP_2	0.	16000.	100	0.0	1	0.2	0.01	0.0
POND_2	PUMP_2	0.	16000.	10	0.0	1	0.2	0.01	0.0
POND_3	PUMP_3	0.	16000.	10	0.0	1	0.2	0.01	0.0
PUMP_1	POND_3	10.0	16000.	10	0.0	1	0.2	-0.01	-1.0
PUMP_2	POND_3	10.0	16000.	10	0.0	1	0.2	0.01	0.0
PUMP_3	sksc	10.0	16000.	10					

Canal Geometry Data File (geom.dat)

Canal geometry data are used for estimating water-surface evaporation and water depth of a selected canal.

Canal geometry for the sample problem

F-Node	T-Node	Rough. coef.	Canal length (feet)	Channel slope	Canal width (feet)	Side slope	Canal depth (feet)	Hydr. cond. (ft/d)	Riverbed thickness (feet)	Bottom elevation (feet)
pond_1	pump_1	0.025	1000	0.01	50	0	15	1.0	1.0	1240.0
pump_1	pond_3	0.025	5000	0.001	50	0	10	1.0	1.0	1140.0
pump_2	pond_3	0.025	500	0.01	50	0	30	1.0	1.0	1140.0

Outlet Hydraulic-Structure File (hydr.dat)

Three kinds of hydraulic structures—sharp-crested weir, gate on spillway, and gate on broad-crested weir—are include in the sample problem.

Hydraulic-structure parameters for the sample problem

Name	F_Node	T_Node	Type	Base elevation (feet)	Weir length or pipe diameter (feet)	Weir height or pipe length (feet)	Gate opening or pipe friction (feet)
Gate-1	Pond_1	Pump_1	2	1240.00	2.0	0	1.0
Weir-2	Pond_2	Pump_2	1	1340.00	2.0	-20	
Gate-3	Pond_3	Pump_3	3	1020.00	10.0	0	

Surface-Runoff File (rnof.dat)

Surface-runoff data for the sample problem

Name	Antecedent 5-day rainfall (inches)	AMC for dry conditions (inches)	AMC for wet conditions (inches)	SCS curve number	Drainage area (acres)
pump_3	1.0	0.5	1.1	85	10000

Elevation-Volume-Area Relation File (pzva.dat)

Elevation-capacity-area relations of three ponds for the sample problem

File format: pond_name

elevation volume area

Elevation is in feet, volume is in acre-feet; and area is in acres.

0 !Z-V-A type(0--table; 1 -- QNWR regression equations)

```
pond_1
1239.0      0.0      0.0
1240.0      14.0     42.0
1241.0      87.3    110.0
1242.0     200.3    116.0
1243.0     349.0    184.0
1244.0     536.0    190.0
1245.0     762.0    264.0
1246.0    1032.0    276.0
1247.0    1351.0    364.0
1248.0    1724.9    384.0
1249.0    2181.9    534.0
1250.0    2784.5    600.0
1251.0    3428.9    764.0
1252.0    4210.8    800.0
1253.0    5076.9    934.0
1254.0    6028.9    970.0
1255.0    7062.2   1104.0
1256.0    8187.1   1140.0
1257.0    9393.5   1274.0
1258.0   10700.4   1340.0
1259.0   12121.6   1504.0
1260.0   13668.4   1590.0
1261.0   15339.7   1754.0
1262.0   17146.4   1860.0
1263.0   19087.9   2024.0
1264.0   21164.6   2130.0
1265.0   23376.1   2294.0
1266.0   25722.9   2400.0
1267.0   28204.5   2560.0
1269.0   33513.1   2774.0
1270.0   36315.1   2830.0
1271.0   39211.8   2964.0
1272.0   42210.7   3034.0
1273.0   45308.5   3162.0
1274.0   48506.5   3234.0
1275.0   58118.1   3390.0
1276.0   55258.0   3490.0
1277.0   58827.7   3650.0
1278.0   62532.6   3760.0
1279.0   66372.3   3920.0
1280.0   70347.0   4030.0
1281.0   74456.0   4190.0
1282.0   78701.8   4300.0
1283.0   83076.0   4450.0
1284.0   87578.0   4560.0
1285.0   92213.0   4710.0
1286.0   96978.0   4820.0
1287.0  101892.0   5010.0
1288.0  106982.0   5170.0
1289.0  112266.0   5400.0
1290.0  117764.0   5560.0
1290.0  123380.0   5710.0
1292.0  129165.0   5860.0
1293.0  135095.0   6000.0
1294.0  141174.0   6160.0
1295.0  147414.0   6320.0
1296.0  153823.0   6500.0
1297.0  160413.0   6680.0
1298.0  167207.0   6910.0
1299.0  174212.0   7100.0
1300.0  181438.0   7340.0
1301.0  188867.8   7520.0
```

1302.0	196507.5	7760.0
1303.0	204347.0	7920.0
1304.0	212397.0	8228.0
1305.0	220656.8	8340.0
1306.0	229126.0	8600.0
1307.0	237831.3	8810.0
1308.0	246776.0	9080.0
1309.0	255960.0	9290.0
1310.0	265405.8	9601.0

pond_2

1308.0	0.0	0.0
1309.0	1.0	3.0
1310.0	4.5	4.0
1311.0	9.0	5.0
1312.0	14.5	6.0
1313.0	21.4	8.0
1314.0	31.4	12.0
1315.0	45.3	16.0
1316.0	64.3	22.0
1317.0	93.0	36.0
1318.0	142.3	64.0
1319.0	227.3	108.0
1320.0	363.3	166.0
1321.0	567.1	244.0
1322.0	853.0	330.0
1323.0	1240.5	448.0
1324.0	1744.4	562.0
1325.0	2384.7	722.0
1326.0	3175.7	862.0
1327.0	4137.9	1066.0
1328.0	5271.2	1202.0
1329.0	6573.9	1406.0
1330.0	8037.5	1522.0
1331.0	9660.4	1726.0
1332.0	11451.0	1856.0
1333.0	13419.0	2082.0
1334.0	15561.7	2204.0
1335.0	17898.0	2472.0
1336.0	20442.1	2616.0
1337.0	23199.8	2902.0
1338.0	26183.4	3066.0
1339.0	29401.2	3372.0
1340.0	32865.8	3558.0
1341.0	36583.7	3880.0
1342.0	40528.5	4010.0
1343.0	44707.3	4350.0
1344.0	49103.2	4442.0
1345.0	53730.0	4814.0
1346.0	58594.9	4916.0
1347.0	63712.6	5322.0
1348.0	69090.5	5434.0
1349.0	74733.2	5854.0
1350.0	80864.1	5976.0
1351.0	86849.7	6430.0
1352.0	93343.6	6558.0
1353.0	100140.2	7038.0
1354.0	107256.0	7094.0
1355.0	114710.5	7718.0
1356.0	122532.2	7926.0
1357.0	130752.5	8518.0
1358.0	139402.1	8782.0
1359.0	148449.8	9316.0
1360.0	157889.5	9584.0
1361.0	167748.3	10116.0
1362.0	178022.9	10434.0
1363.0	188741.0	11006.0
1364.0	199911.2	11334.0
1365.0	211534.5	11915.0
1366.0	223636.1	12249.0

pond_3

1008.0	0.0	0.0
1009.0	0.0	0.0
1010.0	3.0	8.4
1011.0	20.0	27.4
1012.0	50.0	32.7
1013.0	84.0	35.3
1014.0	122.0	40.8
1015.0	164.0	43.3
1016.0	212.0	52.9
1017.0	269.0	61.2

1018.0	335.0	70.9
1019.0	411.0	81.2
1020.0	505.0	107.4
1021.0	666.0	221.4
1022.0	966.0	386.2
1023.0	1451.0	591.0
1024.0	2171.0	857.2
1025.0	3167.0	1141.5
1026.0	4467.0	1465.2
1027.0	6089.0	1784.1
1028.0	8054.0	2151.7
1029.0	10517.0	2788.0
1030.0	13576.0	3338.2
1031.0	17332.0	4189.9
1032.0	21788.0	4727.5
1033.0	27038.0	5790.4
1034.0	32947.0	6028.4
1036.0	46511.0	7193.9
1037.0	54164.0	8125.5
1038.0	62433.0	8417.4
1039.0	71284.0	9291.8
1040.0	80723.0	9587.0
1041.0	90745.0	10463.4
1042.0	101314.0	10795.4
1043.0	112626.0	11714.8
1044.0	124525.0	12084.1
1045.0	137072.0	13015.6
1046.0	150271.0	13383.2
1047.0	164118.0	14316.0
1048.0	178617.0	14682.8
1049.0	193774.0	15636.2
1050.0	209603.0	16022.6
1051.0	226110.0	16996.2
1052.0	243309.0	17402.6
1053.0	261221.0	18426.3
1054.0	279875.0	18882.6
1055.0	299292.0	19956.3
1056.0	319495.0	20450.7
1057.0	340497.0	21558.2
1058.0	362316.0	22080.9
1059.0	384963.0	23217.9
1060.0	408461.0	23779.2
1061.0	432828.0	24959.5
1062.0	458087.0	25559.6
1063.0	484273.0	26817.4
1064.0	514131.0	27500.0
1065.0	539588.0	28819.1
1066.0	568766.0	29538.4
1067.0	598982.0	30898.7
1068.0	930250.0	31638.7
1069.0	662487.0	32839.0
1070.0	695834.0	33857.6
1071.0	730152.0	34780.5
1073.0	802016.0	36862.2
1074.0	839592.0	38294.3
1075.0	878315.0	39153.3
1076.0	918206.0	40633.3
1077.0	959294.0	41544.4
1078.0	1001594.0	43063.0
1079.0	1045132.0	44017.6
1080.0	1089926.0	45574.9

FINISH

Seasonal Target Water-Demand File (tws.dat)

Seasonal water demands for the sample problem
(values are in acre-feet.

```
>
>
>
0      :unit code (0 -- acre-ft, 1 -- ft^3/s, 2 -- ft^3/d)
Date  PUMP_2      pump_1  PUMP_3
Jan.   343        619    2580
Feb.   310        599    2330
March  343        619    2580
April  332        599    2496
May    343        619    2580
June   332        599    2496
July   343        619    2580
```

Aug.	343	619	2580
Sept.	332	599	2496
Oct.	343	619	2580
Nov.	332	599	2496
Dec.	343	619	2580

Seasonal Water-Surface-Evaporation Coefficient File (pet.dat)

Sample seasonal pond water-surface evaporation coefficients
(Values are in inches per day)

```
>
>
1 :unit code(0 -- mm/d, 1 -- in/d, 2 -- ft/d)
Date      pond_2    pond_1
Jan.      0.1      0.01
Feb.      0.1      0.01
March     0.1      0.01
April     0.1      0.01
May       0.1      0.01
June      0.1      0.01
July      0.1      0.01
Aug.      0.1      0.01
Sept.     0.1      0.01
Oct.      0.1      0.01
Nov.      0.1      0.01
Dec.      0.1      0.01
```

Seasonal Rule-Curve File (rc.dat)

Seasonal rule-curve data for the sample problem

```
>
>
>
>
0 :Unit code(0 -- feet; 1 -- inches; 2 -- millimeters)
Date      Pond_2
Jan.      1350.5
Feb.      1355
March     1360
April     1355
May       1350
June      1350
July      1360
Aug.      1360
Sept.     1360
Oct.      1360
Nov.      1360
Dec.      1360
```

Local Net Inflow File (ifw.dat)

Local net inflows to three ponds for the sample problem

```
<
<
<
<
0 :Data unit code (0 -- acre-ft, 1 -- ft^3/s, 2 -- ft^3/d)
Date      POND_1    POND_2    POND_3
Oct.      1729.16   1494.33   36962.78
Nov.      1593.61   1274.00   32579.50
Dec.      13222.35  9886.98   66895.89
Jan.      19412.21  13013.84  171310.33
Feb.      6358.12   7925.43   203225.77
March     723.75    2390.50   118321.28
April     -407.64    64.59     67050.85
May       2926.71    122.61    31301.72
June      -840.34    -1525.57   22908.99
July      -435.46    -1355.91   8527.78
Aug.      -126.75    94.50      66.27
Sept.     77.64     314.43    7617.22
```

Precipitation File (rain.dat)

Total rainfall to ponds for the sample problem
These value are in inches.

```
<
<
<
1      :Unit code(0 -- feet; 1 -- inches; 2 -- millimeters)
Date   pond_2   Default
Oct.   2.0      0.1
```

File for Time-Dependent, Water-Surface-Evaporation Coefficients (pet2.dat)

Time-dependent pond water-surface
evaporation coefficients for the sample problem

```
<
<
<
1      :Unit code (0 -- mm/d, 1 -- in/d, 2 -- ft/d)
Date   pond_3     pond_1 Default
Oct.   0.15        0.1  0.01
Nov.   0.15        0.1  0.01
Dec.   0.15        0.1  0.01
Jan.   0.15        0.1  0.01
Feb.   0.15        0.1  0.01
March  0.15        0.1  0.01
April  0.15        0.1  0.01
May    0.15        0.1  0.01
June   0.15        0.1  0.01
July   0.15        0.1  0.01
Aug.   0.15        0.1  0.01
Sept.  0.15        0.1  0.01
```

File for Time-Dependent, Target Water Demands (tw2.dat)

Time-dependent water demands for the sample problem

```
<
<
<
<
0      :Unit code (0 -- acre-ft; 1 -- ft^3/s; 2 -- ft^3/d)
Date   pond_3     PUMP_3
Oct.   2580       2600
Nov.   2330       2330
Dec.   2580       2580
Jan.   2496       2496
Feb.   2580       2580
March  2496       2496
April  2580       2580
May    2580       2580
June   2496       2496
July   2580       2580
Aug.   2496       2496
Sept.  2580       2580
```

File for Time-Dependent, Pond Rule Curves (rc2.dat)

Time-dependent rule curves for the sample problem

```
<
<
<
<
0      :Unit code(0 -- feet; 1 -- inches; 2 -- millimeters)
Date   Pond_1
Oct.   1274.0
Nov.   1270.0
Dec.   1265.0
Jan.   1275.0
Feb.   1275.0
March  1275.0
April  1275.0
May    1275.0
June   1275.0
July   1275.0
Aug.   1275.0
Sept.  1275.0
```

File for Ground-Water Elevations (gwl.dat)

Ground-water elevations for the sample problem
Elevations are in feet above sea level.

```
<
<
<
0 :Unit code(0 -- feet; 1 -- inches; 2 -- millimeters)
Date POND_1 POND_2 POND_3 Pump_1 Pump_2 pump_3
Oct. 1276.0 1355.0 1045.0 1250.0 1250.0 1150.0
Nov. 1276.0 1355.0 1045.0 1250.0 1250.0 1150.0
Dec. 1276.0 1355.0 1045.0 1250.0 1250.0 1150.0
Jan. 1276.0 1355.0 1045.0 1250.0 1250.0 1150.0
Feb. 1276.0 1355.0 1045.0 1250.0 1250.0 1150.0
March 1276.0 1355.0 1045.0 1250.0 1250.0 1150.0
April 1276.0 1355.0 1045.0 1250.0 1250.0 1150.0
May 1276.0 1355.0 1045.0 1250.0 1250.0 1150.0
June 1276.0 1355.0 1045.0 1250.0 1250.0 1150.0
July 1276.0 1355.0 1045.0 1250.0 1250.0 1150.0
Aug. 1276.0 1355.0 1045.0 1250.0 1250.0 1150.0
Sept. 1276.0 1355.0 1045.0 1250.0 1250.0 1150.0
```

File for List of Nodes for Time-Series Water-Budget Output (snbl.dat)

This file is used to list nodes for which the time-series output of water budgets is needed. For each node specified, two output files are generated—one for nodal water budget (for example, bpond_1.out) and the other for water releases to downstream nodes (for example, rpond_1.out).

```
pond_1
pond_2
pond_3
pump_1
pump_2
pump_3
```

File for List of Canals for Water-Budget Output (sabl.dat)

This file is used to list canal reaches for which time-series output of water budgets is needed. In this sample problem, only three canals are listed for the time-series output of water budgets. The names of output files are automatically generated from the OPONDS program with the form arbud###.dat, where ### is the sequential number such as 001, 002, and 003 in the list file. For example, the output file name for canal reach from pond_1 to pump_1 is arbud001.dat.

```
pond_1 pump_1
pond_2 pump_2
pump_1 pond_3
```

Sample Problem Output

Four general output files are generated from the OPONDS program for the sample problem. The first one is for the input information and network configuration, the second one is for nodal water budgets, the third one is for canal water budgets, and the final one is for hydraulic-structure operations. The output file names are specified in the master data file. In addition to general outputs, there are two kinds of optional water-budget outputs in time-series format for specified nodes and canal reaches. It should not be expected that the outputs from running the same problem on different computers will match exactly. Small variations in output can be caused by differences in the way real numbers are stored and calculated. Storage and calculation of real numbers depend on the specific computer hardware, the FORTRAN compiler, and the math library that is loaded with the compiled program. Output variations among computers also depend on the size of the problem, the number of iterations required for solution, and the precision used when printing results.

Output File for Input Information and Network Configuration (nsim.out)

Operation of a river system.

Summary of simulation period:

```

=====
Length of a time step: 30.42 days
Number of time steps of a year: 12
Starting season: 10
Starting year: 1996
Number of simulation time steps: 12
Flow-convergence criterion: 0.100 cubic feet per second
Maximum iteration steps: 100
Number of decimal points: 2
  
```

Summary of pond elevation-volume-area relations:

```

=====
      No  Name      Number of  Minimum  Maximum
      ---  ---      records    (feet)   (feet)
-----  -
1  POND_1      71  1239.00  1310.00
2  POND_2      59  1308.00  1366.00
3  POND_3      71  1008.00  1080.00
  
```

Part 1: Pond-storage zoning information

```

=====
POND_1      1      :Pond name and corresponding node number
1239.00      0.00  1.00 :Bottom elevation, hydraulic conductivity, and thickness
48506.50      :Initial storage, in acre-feet
1274.00      :Rule-curve elevation, in feet
1  1240.00  100      :Zone number, elevation, and penalty coefficient
2  1280.00  1000     :Zone number, elevation, and penalty coefficient

POND_2      2      :Pond name and corresponding node number
1308.00      0.00  1.00 :Bottom elevation, hydraulic conductivity, and thickness
83856.91      :Initial storage, in acre-feet
1350.50      :Rule-curve elevation, in feet
1  1320.00  100      :Zone number, elevation, and penalty coefficient
2  1365.00  1000     :Zone number, elevation, and penalty coefficient

POND_3      3      :Pond name and corresponding node number
1010.00      0.00  1.00 :Bottom elevation, hydraulic conductivity, and thickness
71284.00      :Initial storage, in acre-feet
1039.00      :Rule-curve elevation, in feet
1  1020.00  100      :Zone number, elevation, and penalty coefficient
2  1050.00  1000     :Zone number, elevation, and penalty coefficient
3  1060.00  2000     :Zone number, elevation, and penalty coefficient
  
```

Part 2: Network flow configuration

```

=====
      Lower  Upper  Initial  Travel-  Weighting  Seepage  Evaporation
      boundary boundary storage time factor coefficient coefficient
      (cubic feet (cubic feet (acre- (day) per second) (inches
      per second) per second) feet) per day) per day)
-----  -
POND_1  PUMP_1      0.00  16000.00  10  0.00  1.00  0.20  0.01  0.00
POND_1  PUMP_2      0.00  16000.00  100  0.00  1.00  0.20  0.01  0.00
POND_2  PUMP_2      0.00  16000.00  10  0.00  1.00  0.20  0.01  0.00
POND_3  PUMP_3      0.00  16000.00  10  0.00  1.00  0.20  0.01  0.00
PUMP_1  POND_3     10.00  16000.00  10  0.00  1.00  0.20  -0.01  -1.00
PUMP_2  POND_3     10.00  16000.00  10  0.00  1.00  0.20  0.01  0.00
PUMP_3  SKSC       10.00  16000.00  10
  
```

PART 3: Canal geometry data

```

=====
      F-node  T-node  Roughness  Canal  Canal  Bottom  Side  Canal  Riverbed  Riverbed  Entry
      name    name    coefficient length slope width slope depth conductivity thickness elevation
      ---  ---  ---  ---  ---  ---  ---  ---  ---  ---  ---
      (feet) (feet) (feet) (feet) (feet) (feet) (feet) (feet per day) (feet) (feet)
-----  -
POND_1  PUMP_1      0.0250  1000.00  0.010000  50.00  0.000000  15.00  1.00  1.00  1240.00
PUMP_1  POND_3      0.0250  5000.00  0.001000  50.00  0.000000  10.00  1.00  1.00  1140.00
PUMP_2  POND_3      0.0250  500.00  0.010000  50.00  0.000000  30.00  1.00  1.00  1140.00
  
```


Part 4: Hydraulic-structure data

=====

Structure name	F-node name	T-node name	Structure type	Base elevation (feet)	Weir width or pipe diameter (feet)	Weir height or pipe length (feet)	Pipe-friction factor	Pipe-entry loss factor
Gate-1	Pond_1	Pump_1	Gate spillway	1240.00	2.00	0.00	1.00000	0.00000
Weir-2	Pond_2	Pump_2	Sharp-crested weir	1340.00	2.00	-20.00	0.00000	0.00000
Gate-3	Pond_3	Pump_3	Sluice gate	1020.00	10.00	0.00	0.00000	0.00000

Part 5: Surface-runoff data

=====

Initial criterion 5-day rainfall

Name	Antecedent 5-day rainfall (inches)	Dry condition (inches)	Wet condition (inches)	SCS curve number	Drainage area (acres)
PUMP_3	1.00	0.50	1.10	85.00	10000.00

Part 10: Seasonal target water demands, in acre-feet

=====

SEASON	PUMP_2	PUMP_1	PUMP_3
1	343.00	619.00	2580.00
2	310.00	599.00	2330.00
3	343.00	619.00	2580.00
4	332.00	599.00	2496.00
5	343.00	619.00	2580.00
6	332.00	599.00	2496.00
7	343.00	619.00	2580.00
8	343.00	619.00	2580.00
9	332.00	599.00	2496.00
10	343.00	619.00	2580.00
11	332.00	599.00	2496.00
12	343.00	619.00	2580.00

Part 11: Seasonal water-surface evaporation coefficients, in inches per day

=====

SEASON	POND_2	POND_1
1	0.10	0.01
2	0.10	0.01
3	0.10	0.01
4	0.10	0.01
5	0.10	0.01
6	0.10	0.01
7	0.10	0.01
8	0.10	0.01
9	0.10	0.01
10	0.10	0.01
11	0.10	0.01
12	0.10	0.01

Part 13: Seasonal rule-curve elevations, in feet

=====

SEASON	POND_2
1	1350.50
2	1355.00
3	1360.00
4	1355.00
5	1350.00
6	1350.00
7	1360.00
8	1360.00
9	1360.00
10	1360.00
11	1360.00
12	1360.00

Summary for local incremental inflow data file:
 =====

File name: ifw.dat
 Data unit: acre-feet
 Number of nodes: 3
 Number of records: 12
 List of nodal names: POND_1 POND_2 POND_3

Summary for precipitation data file:
 =====

File name: rain.dat
 Data unit: inches
 Number of nodes: 6
 Number of records: 1
 List of nodal name: POND_2 DEFAULT

Summary for water-surface evaporation coefficient data file:
 =====

File name: pet2.dat
 Data unit: inches per day
 Number of nodes: 6
 Number of records: 12
 List of nodal name: POND_3 POND_1 DEFAULT

Summary for target water-demand data file:
 =====

File name: tws2.dat
 Data unit: acre-feet
 Number of nodes: 2
 Number of records: 12
 List of nodal names: POND_3 PUMP_3

Summary for rule-curve elevation data file:
 =====

File name: rc2.dat
 Data unit: feet
 Number of nodes: 1
 Number of records: 12
 List of nodal names: POND_1

Summary for ground-water-level data file:
 =====

File name: gwl.dat
 Data unit: feet
 Number of nodes: 6
 Number of records: 12
 List of nodal names: POND_1 POND_2 POND_3 PUMP_1 PUMP_2
 PUMP_3

Node name and type of basic network
 =====

Node number	Node name	Node type
1	POND_1	POND
2	POND_2	POND
3	POND_3	POND
4	PUMP_1	GENERAL
5	PUMP_2	GENERAL
6	PUMP_3	GENERAL

Basic network
=====

		Flow boundary		Penalty coefficient	Type
Arc	From- node	To- node	Lower (cubic feet per second)	Upper (cubic feet per second)	
1	SKSC	POND_1	0.00	803.69	100
2	POND_1	SKSC	0.00	361.97	1000
3	SKSC	POND_2	0.00	1383.78	100
4	POND_2	SKSC	0.00	2116.07	1000
5	SKSC	POND_3	0.00	1173.06	100
6	POND_3	SKSC	0.00	2292.43	1000
7	POND_3	SKSC	0.00	3295.78	2000
8	POND_1	STRM001004	0.00	16000.00	10
9	STRM001004	PUMP_1	0.00	16000.00	10
10	POND_1	STRM001005	0.00	16000.00	100
11	STRM001005	PUMP_2	0.00	16000.00	100
12	POND_2	STRM002005	0.00	16000.00	10
13	STRM002005	PUMP_2	0.00	16000.00	10
14	POND_3	STRM003006	0.00	16000.00	10
15	STRM003006	PUMP_3	0.00	16000.00	10
16	PUMP_1	STRM004003	10.00	16000.00	10
17	STRM004003	POND_3	10.00	16000.00	10
18	PUMP_2	STRM005003	10.00	16000.00	10
19	STRM005003	POND_3	10.00	16000.00	10
20	PUMP_3	SKSC	10.00	16000.00	10

Pond-zoning penalty coefficients
=====

Name	Zone 1	Zone 2	Zone 3
POND_1	100	1000	
POND_2	100	1000	
POND_3	100	1000	2000

Flow-zoning penalty coefficients
=====

From- node	To node	Normal flow zone	Lower flow zone	Upper flow zone
POND_1	PUMP_1	10		
POND_1	PUMP_2	100		
POND_2	PUMP_2	10		
POND_3	PUMP_3	10		
PUMP_1	POND_3	10		
PUMP_2	POND_3	10		
PUMP_3	SKSC	10		

Nodal water budgets for whole simulation
=====

Node name	Node type	Initial storage (acre- feet)	Upstream inflow (acre- feet)	Local net inflow (acre- feet)	Evap- ration (acre- feet)	Rainfall (acre- feet)	Seepage (acre- feet)	Withdrawal (acre- feet)	Downstream release (acre- feet)	Final storage (acre- feet)
POND_1	1	48506.50	0.00	44233.36	9070.00	26.95	0.00	0.00	49592.00	34104.81
POND_2	1	83856.91	0.02	33699.75	2006.36	1033.83	-730.60	0.00	42006.13	75308.61
POND_3	1	71284.00	98219.99	766768.44	43382.32	77.43	-1911.17	30374.00	797618.75	66885.85
PUMP_1	2	0.00	49056.46	0.00	0.00	0.00	0.00	7328.00	41728.46	0.00
PUMP_2	2	0.00	41555.05	0.00	0.00	0.00	0.00	4039.00	37516.05	0.00
PUMP_3	2	0.00	789537.75	0.00	0.00	0.00	0.00	30394.00	759143.75	0.00

Canal water budgets for whole simulation
=====

No.	From	To	Inflow (acre- feet)	Initial storage (acre- feet)	Canal seepage (acre- feet)	Surface evapo- ration (acre- feet)	Final storage (acre- feet)	Outflow (acre- feet)
1	POND_1	PUMP_1	49592.00	1576.56	495.93	0.00	39.61	49056.46
2	POND_1	PUMP_2	0.00	0.00	0.00	0.00	0.00	0.00
3	POND_2	PUMP_2	42006.13	1338.02	420.07	0.00	31.01	41555.05
4	POND_3	PUMP_3	797618.75	25902.82	7976.17	0.00	104.85	789537.75
5	PUMP_1	POND_3	41728.46	1817.42	-19448.18	13.97	63.69	61098.98
6	PUMP_2	POND_3	37516.05	1203.05	375.17	0.00	19.88	37121.00

7 PUMP_3 SKSC 759143.75 0.00 0.00 0.00 0.00 759143.75

System water budgets

=====

Budget	Pond	Canal	Total
-----	-----	-----	-----
Initial storage:	203647.41	0.00	203647.41
Total net inflow:	844701.56	0.00	844701.56
Runoff:	1138.22	0.00	1138.22
Evaporation:	54458.68	13.97	54472.65
Ground-water seepage:	-2641.76	-10180.84	-12822.60
Water withdrawal:	72135.00	0.00	72135.00
Outflow:	0.00	759143.75	759143.75
Final storage:	176299.28	259.03	176558.31
-----	-----	-----	-----
In - Out =			0.13

Output File for Nodal Water Budgets (ndbt.out)

Water budgets for time step: 1

=====

			[--, not applicable]										
	Node		Initial	Upstream	Local Net	Evapo-			Downstream		Final		
No.	name	type	storage	inflow	inflow	ration	Runoff	Seepage	Withdrawal	release	storage	Final	Water
			(acre-	(acre-	(acre-	(acre-	(acre-	(acre-	(acre-	(acre-	(acre-	stage	depth
			feet)	feet)	feet)	feet)	feet)	feet)	feet)	feet)	feet)	(feet)	(feet)
1	POND_1	1	48506.50	0.00	1729.16	819.82	26.95	0.00	0.00	1275.66	48167.13	1273.89	34.89
2	POND_2	1	83856.91	0.00	1494.33	157.25	1033.83	-84.91	0.00	1015.08	85297.66	1350.74	42.74
3	POND_3	1	71284.00	2813.19	36962.78	3533.21	77.43	-169.59	2580.00	33909.79	71284.00	1039.00	29.00
4	PUMP_1	2	--	1222.37	0.00	--	0.00	--	619.00	603.37	--	--	--
5	PUMP_2	2	--	972.68	0.00	--	0.00	--	343.00	629.68	--	--	--
6	PUMP_3	2	--	32493.18	0.00	--	0.00	--	2600.00	29893.18	--	--	--

Water budgets for time step: 2

=====

			[--, not applicable]										
			Initial storage	Upstream inflow	Local Net inflow	Evapo-ration	Runoff	Seepage	Withdrawal	Downstream release	Final storage	Final stage	Water depth
No.	Node name	Node type	(acre-feet)	(acre-feet)	(acre-feet)	(acre-feet)	(acre-feet)	(acre-feet)	(acre-feet)	(acre-feet)	(acre-feet)	(feet)	(feet)
1	POND_1	1	48167.13	0.00	1593.61	817.88	0.00	0.00	0.00	12627.76	36315.10	1270.00	31.00
2	POND_2	1	85297.66	0.00	1274.00	160.02	0.00	-81.79	0.00	949.57	85543.86	1350.78	42.78
3	POND_3	1	71284.00	13259.98	32579.50	3533.21	0.00	-169.59	2330.00	40145.87	71284.00	1039.00	29.00
4	PUMP_1	2	--	12139.73	0.00	--	0.00	--	599.00	11540.73	--	--	--
5	PUMP_2	2	--	941.33	0.00	--	0.00	--	332.00	609.33	--	--	--
6	PUMP_3	2	--	39518.70	0.00	--	0.00	--	2330.00	37188.70	--	--	--

Water budgets for time step: 3

=====

		[-, not applicable]												
	Node		Initial	Upstream	Local	Evapo-			Downstream			Final		
No.	name	type	storage	inflow	inflow	ration	Runoff	Seepage	Withdrawal	release	storage	Final	Water	
			(acre-	(acre-	(acre-	(acre-	(acre-	(acre-	(acre-	(acre-	(acre-	stage	depth	
			feet)	feet)	feet)	feet)	feet)	feet)	feet)	feet)	feet)	(feet)	(feet)	
1	POND_1	1	36315.10	0.00	13222.35	717.41	0.00	0.00	0.00	25443.95	23376.10	1265.00	26.00	
2	POND_2	1	85543.86	0.01	9886.98	160.49	0.00	-81.24	0.00	962.47	94389.13	1352.15	44.15	
3	POND_3	1	71284.00	25655.43	66895.89	3533.21	0.00	-169.59	2580.00	86607.71	71284.00	1039.00	29.00	
4	PUMP_1	2	--	24773.01	0.00	--	0.00	--	619.00	24154.01	--	--	--	
5	PUMP_2	2	--	952.47	0.00	--	0.00	--	343.00	609.47	--	--	--	
6	PUMP_3	2	--	84259.55	0.00	--	0.00	--	2580.00	81679.55	--	--	--	

Water budgets for time step: 4

=====

		[-, not applicable]											
	Node		Initial	Upstream	Local	Evapo-				Downstream	Final		
No.	name	type	storage	inflow	inflow	ration	Runoff	Seepage	Withdrawal	release	storage	Final	Water
			(acre-	(acre-	(acre-	(acre-	(acre-	(acre-	(acre-	(acre-	(acre-	stage	depth
			feet)	feet)	feet)	feet)	feet)	feet)	feet)	feet)	feet)	(feet)	(feet)
1	POND_1	1	23376.10	0.00	19412.21	581.53	0.00	0.00	0.00	443.09	41763.69	1271.85	32.85
2	POND_2	1	94389.13	0.00	13013.84	168.12	0.00	-57.42	0.00	962.09	106330.19	1353.86	45.86
3	POND_3	1	71284.00	3598.11	171310.34	3533.21	0.00	-169.59	2496.00	154318.00	86014.83	1040.52	30.52
4	PUMP_1	2	--	1222.37	0.00	--	0.00	--	619.00	603.37	--	--	--
5	PUMP_2	2	--	952.47	0.00	--	0.00	--	343.00	609.47	--	--	--
6	PUMP_3	2	--	150585.31	0.00	--	0.00	--	2496.00	148089.31	--	--	--

Water budgets for time step: 5

[--, not applicable]

No.	Node name	Node type	Initial storage (acre-feet)	Upstream inflow (acre-feet)	Local Net inflow (acre-feet)	Evapo-ration (acre-feet)	Runoff (acre-feet)	Seepage (acre-feet)	Withdrawal (acre-feet)	Downstream release (acre-feet)	Final storage (acre-feet)	Final stage (feet)	Water depth (feet)
1	POND_1	1	41763.69	0.00	6358.12	766.47	0.00	0.00	0.00	1219.14	46136.20	1273.25	34.25
2	POND_2	1	106330.19	0.00	7925.43	179.65	0.00	-24.36	0.00	927.65	113172.68	1354.79	46.79
3	POND_3	1	86014.83	2892.46	203225.77	3821.42	0.00	-136.71	2580.00	173304.08	112564.27	1042.99	32.99
4	PUMP_1	2	--	1202.37	0.00	--	0.00	--	599.00	603.37	--	--	--
5	PUMP_2	2	--	919.47	0.00	--	0.00	--	310.00	609.47	--	--	--
6	PUMP_3	2	--	170911.70	0.00	--	0.00	--	2580.00	168331.70	--	--	--

Water budgets for time step: 6

[--, not applicable]

No.	Node name	Node type	Initial storage (acre-feet)	Upstream inflow (acre-feet)	Local Net inflow (acre-feet)	Evapo-ration (acre-feet)	Runoff (acre-feet)	Seepage (acre-feet)	Withdrawal (acre-feet)	Downstream release (acre-feet)	Final storage (acre-feet)	Final stage (feet)	Water depth (feet)
1	POND_1	1	46136.20	0.00	723.75	806.29	0.00	0.00	0.00	1235.36	44818.30	1272.84	33.84
2	POND_2	1	113172.68	0.00	2390.50	192.39	0.00	-4.76	0.00	963.20	114412.36	1354.96	46.96
3	POND_3	1	112564.27	2874.73	118321.29	4452.65	0.00	-71.44	2496.00	155599.08	71284.00	1039.00	29.00
4	PUMP_1	2	--	1222.37	0.00	--	0.00	--	619.00	603.37	--	--	--
5	PUMP_2	2	--	952.47	0.00	--	0.00	--	343.00	609.47	--	--	--
6	PUMP_3	2	--	154588.78	0.00	--	0.00	--	2496.00	152092.78	--	--	--

Water budgets for time step: 7

[--, not applicable]

No.	Node name	Node type	Initial storage (acre-feet)	Upstream inflow (acre-feet)	Local Net inflow (acre-feet)	Evapo-ration (acre-feet)	Runoff (acre-feet)	Seepage (acre-feet)	Withdrawal (acre-feet)	Downstream release (acre-feet)	Final storage (acre-feet)	Final stage (feet)	Water depth (feet)
1	POND_1	1	44818.30	0.00	-407.64	796.43	0.00	0.00	0.00	1213.84	42400.39	1272.06	33.06
2	POND_2	1	114412.36	0.00	64.59	195.02	0.00	-0.94	0.00	950.60	113332.27	1354.81	46.81
3	POND_3	1	71284.00	2874.27	67050.84	3533.21	0.00	-169.59	2580.00	63981.50	71284.00	1039.00	29.00
4	PUMP_1	2	--	1202.37	0.00	--	0.00	--	599.00	603.37	--	--	--
5	PUMP_2	2	--	941.47	0.00	--	0.00	--	332.00	609.47	--	--	--
6	PUMP_3	2	--	66266.83	0.00	--	0.00	--	2580.00	63686.83	--	--	--

Water budgets for time step: 8

[--, not applicable]

No.	Node name	Node type	Initial storage (acre-feet)	Upstream inflow (acre-feet)	Local Net inflow (acre-feet)	Evapo-ration (acre-feet)	Runoff (acre-feet)	Seepage (acre-feet)	Withdrawal (acre-feet)	Downstream release (acre-feet)	Final storage (acre-feet)	Final stage (feet)	Water depth (feet)
1	POND_1	1	42400.39	0.00	2926.71	771.11	0.00	0.00	0.00	1235.39	43320.60	1272.35	33.35
2	POND_2	1	113332.27	0.00	122.61	192.73	0.00	-4.28	0.00	32402.33	80864.10	1350.00	42.00
3	POND_3	1	71284.00	31742.23	31301.72	3533.21	0.00	-169.59	2580.00	57100.34	71284.00	1039.00	29.00
4	PUMP_1	2	--	1222.37	0.00	--	0.00	--	619.00	603.37	--	--	--
5	PUMP_2	2	--	31078.96	0.00	--	0.00	--	343.00	30735.96	--	--	--
6	PUMP_3	2	--	56822.88	0.00	--	0.00	--	2580.00	54242.88	--	--	--

Water budgets for time step: 9

[--, not applicable]

No.	Node name	Node type	Initial storage (acre-feet)	Upstream inflow (acre-feet)	Local Net inflow (acre-feet)	Evapo-ration (acre-feet)	Runoff (acre-feet)	Seepage (acre-feet)	Withdrawal (acre-feet)	Downstream release (acre-feet)	Final storage (acre-feet)	Final stage (feet)	Water depth (feet)
1	POND_1	1	43320.60	0.00	-840.34	780.74	0.00	0.00	0.00	1213.84	40485.67	1271.42	32.42
2	POND_2	1	80864.10	0.00	-1525.57	151.49	0.00	-90.89	0.00	0.00	79277.94	1349.74	41.74
3	POND_3	1	71284.00	3866.78	22908.99	3533.21	0.00	-169.59	2496.00	20916.16	71284.00	1039.00	29.00
4	PUMP_1	2	--	1202.37	0.00	--	0.00	--	599.00	603.37	--	--	--
5	PUMP_2	2	--	1003.94	0.00	--	0.00	--	332.00	671.94	--	--	--
6	PUMP_3	2	--	21864.21	0.00	--	0.00	--	2496.00	19368.21	--	--	--

Water budgets for time step: 10

[--, not applicable]

No.	Node name	Node type	Initial storage (acre-feet)	Upstream inflow (acre-feet)	Local Net inflow (acre-feet)	Evapo-ration (acre-feet)	Runoff (acre-feet)	Seepage (acre-feet)	Withdrawal (acre-feet)	Downstream release (acre-feet)	Final storage (acre-feet)	Final stage (feet)	Water depth (feet)
1	POND_1	1	40485.67	0.00	-435.46	758.91	0.00	0.00	0.00	1235.39	38055.91	1270.60	31.60
2	POND_2	1	79277.94	0.01	-1355.91	150.69	0.00	-95.09	0.00	960.76	76905.67	1349.35	41.35
3	POND_3	1	71284.00	2894.30	8527.78	3533.21	0.00	-169.59	2580.00	5478.47	71284.00	1039.00	29.00
4	PUMP_1	2	--	1222.37	0.00	--	0.00	--	619.00	603.37	--	--	--
5	PUMP_2	2	--	946.37	0.00	--	0.00	--	343.00	603.37	--	--	--
6	PUMP_3	2	--	5943.89	0.00	--	0.00	--	2580.00	3363.89	--	--	--

Water budgets for time step: 11													
[--, not applicable]													
			Initial storage	Upstream inflow	Local Net inflow	Evapo-ration	Runoff	Seepage	Withdrawal	Downstream release	Final storage	Final stage	Water depth
No.	Node name	Node type	(acre-feet)	(acre-feet)	(acre-feet)	(acre-feet)	(acre-feet)	(acre-feet)	(acre-feet)	(acre-feet)	(acre-feet)	(feet)	(feet)
1	POND_1	1	38055.91	0.00	-126.75	737.82	0.00	0.00	0.00	1234.71	35956.62	1269.87	30.87
2	POND_2	1	76905.67	-0.01	94.50	149.49	0.00	-101.28	0.00	961.74	75990.21	1349.20	41.20
3	POND_3	1	71284.00	2874.25	66.27	3533.21	0.00	-169.59	2496.00	3038.96	65325.95	1038.32	28.32
4	PUMP_1	2	--	1222.37	0.00	--	0.00	--	619.00	603.37	--	--	--
5	PUMP_2	2	--	951.97	0.00	--	0.00	--	343.00	608.97	--	--	--
6	PUMP_3	2	--	3099.37	0.00	--	0.00	--	2496.00	603.37	--	--	--

Water budgets for time step: 12													
[--, not applicable]													
	Node	Node	Initial	Upstream	Local Net	Evapo-	Runoff	Seepage	Withdrawal	Downstream	Final	Final	Water
No.	name	type	storage	inflow	inflow	ration				release	storage	stage	depth
			(acre-	(acre-	(acre-	(acre-	(acre-	(acre-	(acre-	(acre-	(acre-	(feet)	(feet)
			feet)	feet)	feet)	feet)	feet)	feet)	feet)	feet)	feet)		
1	POND_1	1	35956.62	0.00	77.64	715.59	0.00	0.00	0.00	1213.86	34104.81	1269.21	30.21
2	POND_2	1	75990.21	0.00	314.43	149.03	0.00	-103.64	0.00	950.63	75308.61	1349.09	41.09
3	POND_3	1	65325.95	2874.26	7617.22	3309.39	0.00	-176.67	2580.00	3218.86	66885.85	1038.50	28.50
4	PUMP_1	2	--	1202.37	0.00	--	0.00	--	599.00	603.37	--	--	--
5	PUMP_2	2	--	941.47	0.00	--	0.00	--	332.00	609.47	--	--	--
6	PUMP_3	2	--	3183.37	0.00	--	0.00	--	2580.00	603.37	--	--	--

Output File for Canal Water Budgets (arbt.out)

Canal water budgets for time step: 1									
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No.	From	To	Inflow (acre- feet)	Initial storage (acre- feet)	Canal seepage (acre- feet)	Surface evapo- ration (acre- feet)	Final storage (acre- feet)	Outflow (acre- feet)	Outflow (cubic feet per second)
1	POND_1	PUMP_1	1275.66	0.00	12.76	0.00	40.53	1222.37	20.26
2	POND_1	PUMP_2	0.00	0.00	0.00	0.00	0.00	0.00	0.00
3	POND_2	PUMP_2	1015.08	0.00	10.15	0.00	32.25	972.68	16.12
4	POND_3	PUMP_3	33909.79	0.00	339.10	0.00	1077.51	32493.18	538.53
5	PUMP_1	POND_3	603.37	0.00	-1669.70	1.16	62.09	2209.82	36.62
6	PUMP_2	POND_3	629.68	0.00	6.30	0.00	20.01	603.37	10.00
7	PUMP_3	SKSC	29893.18	0.00	0.00	0.00	0.00	29893.18	495.44

Canal water budgets for time step: 2									
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				Initial	Canal	Surface	Final	Outflow	Outflow
No.	From	To	Inflow (acre- feet)	storage (acre- feet)	seepage (acre- feet)	evapo- ration (acre- feet)	storage (acre- feet)	(acre- feet)	(cubic feet per second)

1	POND_1	PUMP_1	12627.76	40.53	126.28	0.00	402.28	12139.73	201.20
2	POND_1	PUMP_2	0.00	0.00	0.00	0.00	0.00	0.00	0.00
3	POND_2	PUMP_2	949.57	32.25	9.50	0.00	31.00	941.33	15.60
4	POND_3	PUMP_3	40145.87	1077.51	401.46	0.00	1303.22	39518.70	654.96
5	PUMP_1	POND_3	11540.73	62.09	-1463.68	1.16	408.72	12656.61	209.76
6	PUMP_2	POND_3	609.33	20.01	6.09	0.00	19.87	603.37	10.00
7	PUMP_3	SKSC	37188.70	0.00	0.00	0.00	0.00	37188.70	616.35

Canal water budgets for time step: 3									
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No.	From	To	Inflow (acre- feet)	Initial storage (acre- feet)	Canal seepage (acre- feet)	Surface evapo- ration (acre- feet)	Final storage (acre- feet)	Outflow (acre- feet)	Outflow (cubic feet per second)
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1	POND_1	PUMP_1	25443.95	402.28	254.44	0.00	818.78	24773.01	410.58
2	POND_1	PUMP_2	0.00	0.00	0.00	0.00	0.00	0.00	0.00
3	POND_2	PUMP_2	962.47	31.00	9.63	0.00	31.38	952.47	15.79
4	POND_3	PUMP_3	86607.71	1303.22	866.08	0.00	2785.30	84259.55	1396.48
5	PUMP_1	POND_3	24154.01	408.72	-1308.12	1.16	817.63	25052.06	415.20
6	PUMP_2	POND_3	609.47	19.87	6.09	0.00	19.88	603.37	10.00
7	PUMP_3	SKSC	81679.55	0.00	0.00	0.00	0.00	81679.55	1353.72

Canal water budgets for time step: 4

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No.	From	To	Inflow (acre- feet)	Initial storage (acre- feet)	Canal seepage (acre- feet)	Surface evapo- ration (acre- feet)	Final storage (acre- feet)	Outflow (acre- feet)	Outflow (cubic feet per second)
1	POND_1	PUMP_1	443.09	818.78	4.43	0.00	35.06	1222.37	20.26
2	POND_1	PUMP_2	0.00	0.00	0.00	0.00	0.00	0.00	0.00
3	POND_2	PUMP_2	962.09	31.38	9.62	0.00	31.37	952.47	15.79
4	POND_3	PUMP_3	154318.00	2785.30	1543.18	0.00	4974.80	150585.31	2495.73
5	PUMP_1	POND_3	603.37	817.63	-1657.57	1.16	82.67	2994.74	49.63
6	PUMP_2	POND_3	609.47	19.88	6.09	0.00	19.88	603.37	10.00
7	PUMP_3	SKSC	148089.31	0.00	0.00	0.00	0.00	148089.31	2454.36

Canal water budgets for time step: 5

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No.	From	To	Inflow (acre- feet)	Initial storage (acre- feet)	Canal seepage (acre- feet)	Surface evapo- ration (acre- feet)	Final storage (acre- feet)	Outflow (acre- feet)	Outflow (cubic feet per second)
1	POND_1	PUMP_1	1219.14	35.06	12.19	0.00	39.64	1202.37	19.93
2	POND_1	PUMP_2	0.00	0.00	0.00	0.00	0.00	0.00	0.00
3	POND_2	PUMP_2	927.65	31.37	9.28	0.00	30.28	919.47	15.24
4	POND_3	PUMP_3	173304.08	4974.80	1733.04	0.00	5634.14	170911.70	2832.61
5	PUMP_1	POND_3	603.37	82.67	-1668.38	1.16	64.17	2289.09	37.94
6	PUMP_2	POND_3	609.47	19.88	6.09	0.00	19.88	603.37	10.00
7	PUMP_3	SKSC	168331.70	0.00	0.00	0.00	0.00	168331.70	2789.85

Canal water budgets for time step: 6

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No.	From	To	Inflow (acre- feet)	Initial storage (acre- feet)	Canal seepage (acre- feet)	Surface evapo- ration (acre- feet)	Final storage (acre- feet)	Outflow (acre- feet)	Outflow (cubic feet per second)
1	POND_1	PUMP_1	1235.36	39.64	12.35	0.00	40.27	1222.37	20.26
2	POND_1	PUMP_2	0.00	0.00	0.00	0.00	0.00	0.00	0.00
3	POND_2	PUMP_2	963.20	30.28	9.63	0.00	31.38	952.47	15.79
4	POND_3	PUMP_3	155599.08	5634.14	1555.99	0.00	5088.44	154588.78	2562.08
5	PUMP_1	POND_3	603.37	64.17	-1668.68	1.16	63.70	2271.35	37.64
6	PUMP_2	POND_3	609.47	19.88	6.09	0.00	19.88	603.37	10.00
7	PUMP_3	SKSC	152092.78	0.00	0.00	0.00	0.00	152092.78	2520.71

Canal water budgets for time step: 7

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No.	From	To	Inflow (acre- feet)	Initial storage (acre- feet)	Canal seepage (acre- feet)	Surface evapo- ration (acre- feet)	Final storage (acre- feet)	Outflow (acre- feet)	Outflow (cubic feet per second)
1	POND_1	PUMP_1	1213.84	40.27	12.14	0.00	39.60	1202.37	19.93
2	POND_1	PUMP_2	0.00	0.00	0.00	0.00	0.00	0.00	0.00
3	POND_2	PUMP_2	950.60	31.38	9.51	0.00	31.01	941.47	15.60
4	POND_3	PUMP_3	63981.50	5088.44	639.82	0.00	2163.30	66266.83	1098.28
5	PUMP_1	POND_3	603.37	63.70	-1668.68	1.16	63.69	2270.90	37.64
6	PUMP_2	POND_3	609.47	19.88	6.09	0.00	19.88	603.37	10.00
7	PUMP_3	SKSC	63686.83	0.00	0.00	0.00	0.00	63686.83	1055.52

Canal water budgets for time step: 8

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No.	From	To	Inflow (acre- feet)	Initial storage (acre- feet)	Canal seepage (acre- feet)	Surface evapo- ration (acre- feet)	Final storage (acre- feet)	Outflow (acre- feet)	Outflow (cubic feet per second)
1	POND_1	PUMP_1	1235.39	39.60	12.35	0.00	40.27	1222.37	20.26
2	POND_1	PUMP_2	0.00	0.00	0.00	0.00	0.00	0.00	0.00
3	POND_2	PUMP_2	32402.33	31.01	324.02	0.00	1030.36	31078.96	515.09
4	POND_3	PUMP_3	57100.34	2163.30	571.00	0.00	1869.75	56822.88	941.76
5	PUMP_1	POND_3	603.37	63.69	-1668.68	1.16	63.69	2270.88	37.64
6	PUMP_2	POND_3	30735.96	19.88	307.36	0.00	977.13	29471.35	488.44
7	PUMP_3	SKSC	54242.88	0.00	0.00	0.00	0.00	54242.88	899.00

Canal water budgets for time step: 9

No.	From	To	Inflow (acre- feet)	Initial storage (acre- feet)	Canal seepage (acre- feet)	Surface evapo- ration (acre- feet)	Final storage (acre- feet)	Outflow (acre- feet)	Outflow (cubic feet per second)
1	POND_1	PUMP_1	1213.84	40.27	12.14	0.00	39.60	1202.37	19.93
2	POND_1	PUMP_2	0.00	0.00	0.00	0.00	0.00	0.00	0.00
3	POND_2	PUMP_2	0.00	1030.36	0.00	0.00	26.42	1003.94	16.64
4	POND_3	PUMP_3	20916.16	1869.75	209.18	0.00	712.52	21864.21	362.37
5	PUMP_1	POND_3	603.37	63.69	-1668.68	1.16	63.69	2270.88	37.64
6	PUMP_2	POND_3	671.94	977.13	6.73	0.00	46.44	1595.90	26.45
7	PUMP_3	SKSC	19368.21	0.00	0.00	0.00	0.00	19368.21	321.00

Canal water budgets for time step: 10

No.	From	To	Inflow (acre- feet)	Initial storage (acre- feet)	Canal seepage (acre- feet)	Surface evapo- ration (acre- feet)	Final storage (acre- feet)	Outflow (acre- feet)	Outflow (cubic feet per second)
1	POND_1	PUMP_1	1235.39	39.60	12.35	0.00	40.27	1222.37	20.26
2	POND_1	PUMP_2	0.00	0.00	0.00	0.00	0.00	0.00	0.00
3	POND_2	PUMP_2	960.76	26.42	9.61	0.00	31.20	946.37	15.68
4	POND_3	PUMP_3	5478.47	712.52	54.78	0.00	192.32	5943.89	98.51
5	PUMP_1	POND_3	603.37	63.69	-1668.68	1.16	63.69	2270.88	37.64
6	PUMP_2	POND_3	603.37	46.44	6.03	0.00	20.36	623.41	10.33
7	PUMP_3	SKSC	3363.89	0.00	0.00	0.00	0.00	3363.89	55.75

Canal water budgets for time step: 11

No.	From	To	Inflow (acre- feet)	Initial storage (acre- feet)	Canal seepage (acre- feet)	Surface evapo- ration (acre- feet)	Final storage (acre- feet)	Outflow (acre- feet)	Outflow (cubic feet per second)
1	POND_1	PUMP_1	1234.71	40.27	12.35	0.00	40.26	1222.37	20.26
2	POND_1	PUMP_2	0.00	0.00	0.00	0.00	0.00	0.00	0.00
3	POND_2	PUMP_2	961.74	31.20	9.62	0.00	31.36	951.97	15.78
4	POND_3	PUMP_3	3038.96	192.32	30.41	0.00	101.50	3099.37	51.37
5	PUMP_1	POND_3	603.37	63.69	-1668.68	1.16	63.69	2270.88	37.64
6	PUMP_2	POND_3	608.97	20.36	6.09	0.00	19.87	603.37	10.00
7	PUMP_3	SKSC	603.37	0.00	0.00	0.00	0.00	603.37	10.00

Canal water budgets for time step: 12

No.	From	To	Inflow (acre- feet)	Initial storage (acre- feet)	Canal seepage (acre- feet)	Surface evapo- ration (acre- feet)	Final storage (acre- feet)	Outflow (acre- feet)	Outflow (cubic feet per second)
1	POND_1	PUMP_1	1213.86	40.26	12.15	0.00	39.61	1202.37	19.93
2	POND_1	PUMP_2	0.00	0.00	0.00	0.00	0.00	0.00	0.00
3	POND_2	PUMP_2	950.63	31.36	9.51	0.00	31.01	941.47	15.60
4	POND_3	PUMP_3	3218.86	101.50	32.14	0.00	104.85	3183.37	52.76
5	PUMP_1	POND_3	603.37	63.69	-1668.68	1.16	63.69	2270.88	37.64
6	PUMP_2	POND_3	609.47	19.87	6.09	0.00	19.88	603.37	10.00
7	PUMP_3	SKSC	603.37	0.00	0.00	0.00	0.00	603.37	10.00

Output File for Operation of Hydraulic Structures (hydr.out)

Parameters for hydraulic structures: 1

[-999.99, not flow under gate]

Structure name	Upstream node name	Downstream node name	Structure type	Base elevation (feet)	Weir length (feet)	Discharge (cubic feet per second)	Upstream Water elevation (feet)	Gate-opening or weir height (feet)
Gate-1	Pond_1	Pump_1	Gate spillway	1240.00	2.00	21.14	1274.00	0.35
Weir-2	Pond_2	Pump_2	Sharp-crested weir	1340.00	2.00	16.82	1350.50	8.65
Gate-3	Pond_3	Pump_3	Sluice gate	1020.00	10.00	562.00	1039.00	2.90

Parameters for hydraulic structures: 2

[-999.99, not flow under gate]

Structure name	Upstream node name	Downstream node name	Structure type	Base elevation (feet)	Weir length (feet)	Discharge (cubic feet per second)	Upstream Water elevation (feet)	Gate-opening or weir height (feet)
Gate-1	Pond_1	Pump_1	Gate spillway	1240.00	2.00	209.29	1273.89	3.55
Weir-2	Pond_2	Pump_2	Sharp-crested weir	1340.00	2.00	15.74	1350.74	8.97
Gate-3	Pond_3	Pump_3	Sluice gate	1020.00	10.00	665.36	1039.00	3.48

Parameters for hydraulic structures: 3

[-999.99, not flow under gate]

Structure name	Upstream node name	Downstream node name	Structure type	Base elevation (feet)	Weir length (feet)	Discharge (cubic feet per second)	Upstream Water elevation (feet)	Gate-opening or weir height (feet)
Gate-1	Pond_1	Pump_1	Gate spillway	1240.00	2.00	421.70	1270.00	7.99
Weir-2	Pond_2	Pump_2	Sharp-crested weir	1340.00	2.00	15.95	1350.78	9.00
Gate-3	Pond_3	Pump_3	Sluice gate	1020.00	10.00	1435.40	1039.00	8.07

Parameters for hydraulic structures: 4

[-999.99, not flow under gate]

Structure name	Upstream node name	Downstream node name	Structure type	Base elevation (feet)	Weir length (feet)	Discharge (cubic feet per second)	Upstream Water elevation (feet)	Gate-opening or weir height (feet)
Gate-1	Pond_1	Pump_1	Gate spillway	1240.00	2.00	7.34	1265.00	0.14
Weir-2	Pond_2	Pump_2	Sharp-crested weir	1340.00	2.00	15.95	1352.15	10.37
Gate-3	Pond_3	Pump_3	Sluice gate	1020.00	10.00	2557.59	1039.00	-999.99

Parameters for hydraulic structures: 5

[-999.99, not flow under gate]

Structure name	Upstream node name	Downstream node name	Structure type	Base elevation (feet)	Weir length (feet)	Discharge (cubic feet per second)	Upstream Water elevation (feet)	Gate-opening or weir height (feet)
Gate-1	Pond_1	Pump_1	Gate spillway	1240.00	2.00	20.21	1271.85	0.34
Weir-2	Pond_2	Pump_2	Sharp-crested weir	1340.00	2.00	15.37	1353.87	12.12
Gate-3	Pond_3	Pump_3	Sluice gate	1020.00	10.00	2872.26	1040.53	-999.99

Parameters for hydraulic structures: 6

[-999.99, not flow under gate]

Structure name	Upstream node name	Downstream node name	Structure type	Base elevation (feet)	Weir length (feet)	Discharge (cubic feet per second)	Upstream Water elevation (feet)	Gate-opening or weir height (feet)
Gate-1	Pond_1	Pump_1	Gate spillway	1240.00	2.00	20.47	1273.26	0.34
Weir-2	Pond_2	Pump_2	Sharp-crested weir	1340.00	2.00	15.96	1354.79	13.00
Gate-3	Pond_3	Pump_3	Sluice gate	1020.00	10.00	2578.83	1042.99	13.84

Parameters for hydraulic structures: 7

[-999.99, not flow under gate]

Structure name	Upstream node name	Downstream node name	Structure type	Base elevation (feet)	Weir length (feet)	Discharge (cubic feet per second)	Upstream Water elevation (feet)	Gate-opening or weir height (feet)
Gate-1	Pond_1	Pump_1	Gate spillway	1240.00	2.00	20.12	1272.84	0.34
Weir-2	Pond_2	Pump_2	Sharp-crested weir	1340.00	2.00	15.75	1354.96	13.18
Gate-3	Pond_3	Pump_3	Sluice gate	1020.00	10.00	1060.40	1039.00	5.76

Parameters for hydraulic structures: 8

[-999.99, not flow under gate]

Structure name	Upstream node name	Downstream node name	Structure type	Base elevation (feet)	Weir length (feet)	Discharge (cubic feet per second)	Upstream Water elevation (feet)	Gate-opening or weir height (feet)
Gate-1	Pond_1	Pump_1	Gate spillway	1240.00	2.00	20.47	1272.06	0.35
Weir-2	Pond_2	Pump_2	Sharp-crested weir	1340.00	2.00	537.02	1354.82	1.95
Gate-3	Pond_3	Pump_3	Sluice gate	1020.00	10.00	946.35	1039.00	5.09

Parameters for hydraulic structures: 9

[-999.99, not flow under gate]

Structure name	Upstream node name	Downstream node name	Structure type	Base elevation (feet)	Weir length (feet)	Discharge (cubic feet per second)	Upstream Water elevation (feet)	Gate-opening or weir height (feet)
Gate-1	Pond_1	Pump_1	Gate spillway	1240.00	2.00	20.12	1272.36	0.34
Weir-2	Pond_2	Pump_2	Sharp-crested weir	1340.00	2.00	0.00	1350.00	1.95
Gate-3	Pond_3	Pump_3	Sluice gate	1020.00	10.00	346.65	1039.00	1.74

Parameters for hydraulic structures: 10

[-999.99, not flow under gate]

Structure name	Upstream node name	Downstream node name	Structure type	Base elevation (feet)	Weir length (feet)	Discharge (cubic feet per second)	Upstream Water elevation (feet)	Gate-opening or weir height (feet)
Gate-1	Pond_1	Pump_1	Gate spillway	1240.00	2.00	20.47	1271.42	0.35
Weir-2	Pond_2	Pump_2	Sharp-crested weir	1340.00	2.00	15.94	1349.74	7.96
Gate-3	Pond_3	Pump_3	Sluice gate	1020.00	10.00	90.80	1039.00	0.43

Parameters for hydraulic structures: 11

[-999.99, not flow under gate]

Structure name	Upstream node name	Downstream node name	Structure type	Base elevation (feet)	Weir length (feet)	Discharge (cubic feet per second)	Upstream Water elevation (feet)	Gate-opening or weir height (feet)
Gate-1	Pond_1	Pump_1	Gate spillway	1240.00	2.00	20.46	1270.60	0.36
Weir-2	Pond_2	Pump_2	Sharp-crested weir	1340.00	2.00	15.94	1349.35	7.58
Gate-3	Pond_3	Pump_3	Sluice gate	1020.00	10.00	50.37	1039.00	0.24

Parameters for hydraulic structures: 12

[-999.99, not flow under gate]

Structure name	Upstream node name	Downstream node name	Structure type	Base elevation (feet)	Weir length (feet)	Discharge (cubic feet per second)	Upstream Water elevation (feet)	Gate-opening or weir height (feet)
Gate-1	Pond_1	Pump_1	Gate spillway	1240.00	2.00	20.12	1269.87	0.35
Weir-2	Pond_2	Pump_2	Sharp-crested weir	1340.00	2.00	15.76	1349.21	7.44
Gate-3	Pond_3	Pump_3	Sluice gate	1020.00	10.00	53.35	1038.33	0.26

Output File for Time-Series Output of Water Budget for Node Pond_1 (bpond_1.out)

Water Budgets for POND_1

No.	Initial storage (acre-feet)	Upstream inflow (acre-feet)	Local net inflow (acre-feet)	Evapo-ration (acre-feet)	Runoff (acre-feet)	Seepage (acre-feet)	Withdrawal (acre-feet)	Downstream release (acre-feet)	Final storage (acre-feet)	Final stage (feet)	Final depth (feet)
1	48506.50	0.00	1729.16	819.82	26.95	0.00	0.00	1275.66	48167.13	1273.89	34.89
2	48167.13	0.00	1593.61	817.88	0.00	0.00	0.00	12627.76	36315.10	1270.00	31.00
3	36315.10	0.00	13222.35	717.41	0.00	0.00	0.00	25443.95	23376.10	1265.00	26.00
4	23376.10	0.00	19412.21	581.53	0.00	0.00	0.00	443.09	41763.69	1271.85	32.85
5	41763.69	0.00	6358.12	766.47	0.00	0.00	0.00	1219.14	46136.20	1273.25	34.25
6	46136.20	0.00	723.75	806.29	0.00	0.00	0.00	1235.36	44818.30	1272.84	33.84
7	44818.30	0.00	-407.64	796.43	0.00	0.00	0.00	1213.84	42400.39	1272.06	33.06
8	42400.39	0.00	2926.71	771.11	0.00	0.00	0.00	1235.39	43320.60	1272.35	33.35
9	43320.60	0.00	-840.34	780.74	0.00	0.00	0.00	1213.84	40485.67	1271.42	32.42
10	40485.67	0.00	-435.46	758.91	0.00	0.00	0.00	1235.39	38055.91	1270.60	31.60
11	38055.91	0.00	-126.75	737.82	0.00	0.00	0.00	1234.71	35956.62	1269.87	30.87
12	35956.62	0.00	77.64	715.59	0.00	0.00	0.00	1213.86	34104.81	1269.21	30.21

Output File for Time-Series Output of Release from Node Pond_1 (rpond_1.dat)

Outflows from POND_1
=====

No.	PUMP_1		PUMP_2	
	(acre- feet)	(cubic feet per second)	(acre- feet)	(cubic feet per second)
1	1275.66	21.14	0.00	0.00
2	12627.76	209.29	0.00	0.00
3	25443.95	421.70	0.00	0.00
4	443.09	7.34	0.00	0.00
5	1219.14	20.21	0.00	0.00
6	1235.36	20.47	0.00	0.00
7	1213.84	20.12	0.00	0.00
8	1235.39	20.47	0.00	0.00
9	1213.84	20.12	0.00	0.00
10	1235.39	20.47	0.00	0.00
11	1234.71	20.46	0.00	0.00
12	1213.86	20.12	0.00	0.00

Output File for Time-Series Output of Arc Water Budget (arbud001.out)

Canal water budgets from POND_1 to PUMP_1
=====

No.	Inflow (acre- feet)	Initial storage (acre- feet)	Canal seepage (acre- feet)	Water- surface evapo- ration (acre- feet)	Final storage (acre- feet)	Outflow (acre- feet)	Outflow (cubic feet per second)
1	1275.66	0.00	12.76	0.00	40.53	1222.37	20.26
2	12627.76	40.53	126.28	0.00	402.28	12139.73	201.20
3	25443.95	402.28	254.44	0.00	818.78	24773.01	410.58
4	443.09	818.78	4.43	0.00	35.06	1222.37	20.26
5	1219.14	35.06	12.19	0.00	39.64	1202.37	19.93
6	1235.36	39.64	12.35	0.00	40.27	1222.37	20.26
7	1213.84	40.27	12.14	0.00	39.60	1202.37	19.93
8	1235.39	39.60	12.35	0.00	40.27	1222.37	20.26
9	1213.84	40.27	12.14	0.00	39.60	1202.37	19.93
10	1235.39	39.60	12.35	0.00	40.27	1222.37	20.26
11	1234.71	40.27	12.35	0.00	40.26	1222.37	20.26
12	1213.86	40.26	12.15	0.00	39.61	1202.37	19.93

APPENDIX E. COMPUTER PROGRAM LISTING

```

=====
* Name:      OPONDS
* Purpose:   Optimal operation of a system of ponds using linear
*            network flow model.
* Platform:  Unix/DG Avion
* Version:   1.0
* Author:    Xiaodong Jian
*            U.S. Geological Survey
*            4821 Quail Crest Place
*            Lawrence, KS 66049
* Date:      July, 1997
=====
PROGRAM      OPONDS
IMPLICIT     NONE
INTEGER      LDRES, LDND, LDREAR, LDARC, LDRETB, LDCTAR, LDP, LDFIL,
&            LDFXAR, NTL5
PARAMETER    (LDRES = 100, LDND = 300, LDREAR = 200, LDARC = 1000,
&            LDRETB = 5000, LDCTAR = 10, LDP = 365, LDFIL = 35,
&            LDFXAR = 10, NTL5 = 5)
INTEGER      II(LDARC), JJ(LDARC), HI(LDARC), LO(LDARC), COST(LDARC),
&            FLOW(LDARC), ARTYP(LDARC), ARCBUD(LDARC, 0:6)
INTEGER      OHI(LDARC)
INTEGER      REAR(LDREAR), PTRE(LDRES), PTDWAR(LDND, 2),
&            NDDWAR(LDARC)
REAL         REZN(LDREAR)
CHARACTER    NDNAM(LDND)*12
INTEGER      NDTYP(LDND), NDSEQ(LDND), NDXAR(LDND,6),
&            NOBUD(LDND,0:10)
INTEGER      NNODS, NDWAR, SKSC, NARCS
REAL         APRX, CONST, PERD
LOGICAL      ZEROFG
INTEGER      LDRC, LDRCTB
PARAMETER    (LDRC = LDRES, LDRCTB = LDP)
REAL         INST(LDRES), RC(LDRES)
INTEGER      NRCND, RCND(LDRC, 3), RCUNIT
REAL         RCTB(0:LDRCTB, LDRC)
LOGICAL      RCFLAG, RCFIL
CHARACTER    SYSNAM*40, MNTH*5
INTEGER      PN
REAL         OINST(LDRES)
INTEGER      NPER
INTEGER      NOP, NSPS, STMO, YR, I, NRES
INTEGER      ARCS
INTEGER      ARC, YEAR, MTH, KARC
INTEGER      J, MXCST
INTEGER      FCRIT, OFLOW(LDARC), NITR, LDITR, OARCS
INTEGER      ISGN
LOGICAL      FLWARC, NOTCOV
INTEGER      IN, IN_IFW, IN_RN, IN_EV, IN_WS, IN_RC, IN_GW, IN_FX
INTEGER      IN_FB
INTEGER      OU_NT, OU_ND, OU_AR, OU_HY, OU_SN, OU_SA
LOGICAL      NDBFLG, ARBFLG, HYBFLG
INTEGER      CTARFW(LDCTAR, 3), NCTAR
CHARACTER    FILNAM(0:LDFIL)*30
INTEGER      LDSTRM
PARAMETER    (LDSTRM = LDARC)
INTEGER      STRMAR(LDSTRM, 0:6), NSTRM, NARCND
REAL         STRMCF(LDSTRM, 0:4) ! OCF(LDSTRM)
INTEGER      LDSTR
PARAMETER    (LDSTR = LDSTRM)
INTEGER      NSTR, STRDIR(LDSTR, 3)
REAL         STRDAT(LDSTR, 9)
INTEGER      LDHY, LDHYTP
PARAMETER    (LDHY = LDND, LDHYTP = 6)
CHARACTER    HYDIR(LDHY, 0:2)*12
INTEGER      NHY, HYTPCD(LDHY, 0:1), NHYTP
REAL         HYDAT(LDHY, 5), HYOUT(LDHY, 3)
CHARACTER    HYTP(0:LDHYTP)*20
INTEGER      PTRES(LDRES)
REAL         RESDAT(LDRES, 0:2)
INTEGER      XP
REAL         XF
INTEGER      LDIFW
PARAMETER    (LDIFW = LDND)
INTEGER      NIFW, IFWND(LDIFW, 3), IFWCD
INTEGER      NFXAR, FXAR(0:2, LDFXAR), FXUNIT
LOGICAL      FWFLAG, FXFLAG
INTEGER      LDWS
PARAMETER    (LDWS = LDND)
INTEGER      NWSND, WSND(LDWS, 3), WSUNIT
REAL         WSTB(0:LDP, LDWS)
LOGICAL      WSFLAG, WSFIL
INTEGER      LDEV
PARAMETER    (LDEV = LDRES)
INTEGER      NEV, EVND(LDEV, 3), EVUNIT
REAL         EVTB(0:LDP, LDEV)
LOGICAL      EVFLAG, EVFIL
INTEGER      LDGWND

```

```

PARAMETER (LDGWND = LDND)
INTEGER NGWND, GWND(LDGWND, 3), GWTYPE
REAL GWLVL(LDGWND)
LOGICAL GWFLAG
CHARACTER GWUNIT(0:2, 2)*21
INTEGER LDRAIN, LDRNOF, LDRFTB
PARAMETER (LDRAIN = LDND, LDRNOF = LDND, LDRFTB = 4)
INTEGER NRAIN, RAINND(LDRAIN, 3), RAINTY
LOGICAL RNFLAG
INTEGER NRNOF, RNOFND(LDRNOF)
REAL RNOFTB(LDRNOF, 0:LDRFTB), ASDR(LDRNOF, 5)
CHARACTER RNUNIT(0:2, 2)*21
INTEGER LDFBAR, LDFBTB
PARAMETER (LDFBAR = LDARC, LDFBTB = LDP)
INTEGER NFBAR, FBAR(0:7, LDFBAR), FBUNIT
REAL FBTB(0:LDFBTB, LDFBAR)
LOGICAL FBFLAG, FBFIL
INTEGER LDSNBL, LDSABL
PARAMETER (LDSNBL = LDRES, LDSABL = 100)
INTEGER NSNBL, SNBLND(LDSNBL, 3), NSABL, SABLND(LDSABL, 3)
LOGICAL SNBLFG, SABLFG
INTEGER LDUNIT
PARAMETER (LDUNIT = 2)
CHARACTER UNITNM_1(0:2)*21, UNITNM_2(0:2)*21, UNITNM_3(0:2)*21
REAL CNDBT(0:10), CARBT(6), CSNDBT(LDND, 0:10),
& CSARBT(LDARC, 0:6)
INTEGER SAVOPT
INTEGER LDPL, LDCOL
PARAMETER (LDPL = LDND, LDCOL = 50)
CHARACTER CTERM*500, COLSTR(LDCOL)*30
REAL RTERM, RPOOL(LDPL)
INTEGER IFAULT
LOGICAL FLAG
LOGICAL ERR, LAST, DEBUG, CHECK
COMMON /NDNAME/ NDNAM
COMMON /SAVOPT/ SAVOPT
COMMON /CHECK/ CHECK
*
* DATA UNITNM_1/ 'acre-feet', 'cubic feet per second',
& DATA UNITNM_2/ 'cubic feet per day'/
DATA UNITNM_3/ 'feet', 'inches', 'millimeters'/
& DATA UNITNM_3/ 'millimeters per day', 'inches per day',
& DATA UNITNM_3/ 'feet per day'/
& DATA GWUNIT / 'feet', 'inches', 'meter',
& DATA GWUNIT / 'acre-feet per day', 'cubic feet per second',
& DATA GWUNIT / 'cubic feet per day'/
& DATA RNUNIT / 'feet', 'inches', 'millimeters',
& DATA RNUNIT / 'acre feet per day', 'cubic feet per second',
& DATA RNUNIT / 'cubic feet per day'/
*
* ZEROFG = .TRUE.
*
* Open data files and assign associated fortran file units
*
CALL OPMDF(FILNAM, LDFIL, CTERM, COLSTR, LDCOL)
PRINT *, 'Please wait! Processing data files'
IN = 8
CALL IO_OPFIL(IN, 1, FILNAM(0), 'ENTER NETWORK FILE: ')
*
* Open output files
*
OU_NT = 26 ! GENERAL OUPUT.
OU_ND = 27 ! NODAL BUDGET
OU_AR = 28 ! ARC BUDGET
OU_HY = 29 ! FLOW IN STRUCTURE.
OU_SN = 30 ! SINGLE NODAL BUDGET LIST.
OU_SA = 31 ! SINGLE ARC BUDGET LIST.
CALL IO_OPFIL(OU_NT, 3, FILNAM(OU_NT), 'ENTER GENERAL OUTPUT: ')
IF (FILNAM(OU_NT) .EQ. ' ') THEN
NDBFLG = .FALSE.
ELSE
NDBFLG = .TRUE.
CALL IO_OPFIL(OU_ND, 3, FILNAM(OU_ND),
& 'ENTER NODAL BUDGET OUTPUT: ')
& ENDIF
IF (FILNAM(OU_AR) .EQ. ' ') THEN
ARBFLG = .FALSE.
ELSE
ARBFLG = .TRUE.
CALL IO_OPFIL(OU_AR, 3, FILNAM(OU_AR),
& 'ENTER ARC BUDGET OUTPUT: ')
& ENDIF
IF (FILNAM(OU_HY) .EQ. ' ') THEN
HYBFLG = .FALSE.
ELSE
HYBFLG = .TRUE.
CALL IO_OPFIL(OU_HY, 3, FILNAM(OU_HY),
& 'ENTER STRUCTURE OUTPUT: ')
& ENDIF
*
*-----Assign fortran file units for input data files.
*
IN_IFW = 16
IN_RN = 17

```

```

IN_EV = 18
IN_WS = 19
IN_RC = 20
IN_FB = 21
IN_GW = 22
IN_FX = 23
*
*      Initialize some arrays and assign constants.
*
CONST = 86400.0 / 43560.0
APRX = .5
*
*-----Initialization
*
NOP = 0
NARCS = 0
NCTAR = 0
LAST = .FALSE.
ERR = .FALSE.
FLWARC = .FALSE.
DO I = 1, LDARC
    FLOW(I) = 0
    OFLOW(I) = 0
    COST(I) = 0
    REAR(I) = 0
    NDDWAR(I) = 0
ENDDO
DO I = 1, LDND
    NDNAM(I) = ' '
    PTDWAR(I, 1) = 0
    PTDWAR(I, 2) = 0
ENDDO
DO 13 I = 1, LDRES
    PTRE(I) = 0
13 CONTINUE
SKSC = LDND
NDNAM(SKSC) = 'SKSC'
NDWAR = 0
NSTRM = 0
NITR = 0
*
*      Set default value
*
PERD = 30.42
NPER = 12
XP = 0
XF = 10**XP
FCRIT = INT(0.1 * PERD * CONST * XF)
LDITR = 100
NIFW = 0
NWSND = 0
NEV = 0
NRCND = 0
NRAIN = 0
NGWND = 0
NFBAR = 0
NFXAR = 0
*
*      Read sytem name, simulation period, time period length, and
*      output accuracy in no. of decimal points in ac-ft.
*
READ (IN, '(A)') SYSNAM
READ (IN, '(A)')
READ (IN, *) PERD, NPER
READ (IN, *) STMO, YR
READ (IN, *) NSPS
READ (IN, *) RTERM, LDITR
READ (IN, *) XP
*
SAVOPT = 0
CALL GETINT(SAVOPT, IN, IFAULT)
*
WRITE (OU_NT, 802) SYSNAM, PERD, NPER, STMO, YR,
& NSPS, RTERM, LDITR, XP
802 FORMAT(A,
&   ///, 'Summary of simulation period: ',
&   ///, '=====',
&   ///, 'Length of a time step: ', F7.2, ' days',
&   ///, 'Number of time steps of a year: ', I4,
&   ///, 'Starting season: ', I4,
&   ///, 'Starting year: ', I4,
&   ///, 'Number of simulation time steps: ', I4,
&   ///, 'Flow-convergence criterion: ', F8.3,
&   ///, ' cubic feet per second',
&   ///, 'Maximum iteration steps: ', I4,
&   ///, 'Number of decimal points: ', I4)
*
XP = XP + 1
XF = 10**XP
IF (RTERM .NE. 0) THEN
    FCRIT = INT(RTERM * PERD * CONST * XF)
ENDIF
*
*-----READ RESERVOIR CAPACITY TABLES

```

```

*
*      CALL SETZVA(NRES, NNODS, NDNAM, NDTYP, LDND,
*      &          FILNAM(9), NTLS, OU_NT)
*
*      Read reservoir zoning, network configuration,
*      other physical parameters, and seasonal-dependent
*      data.
*
*-----1. READ RESERVOIR ZONES (BOUNDS) AND CREATE THE BASIC
*      storage arcs of reservoirs.
*
*      PN = 1
*      CALL NETWK_1(NDNAM, NDSEQ, LDND,
*      &          NARCS, II, JJ, HI, LO, COST, ARTYP, LDARC,
*      &          NRES, PTRES, RC, INST, RESDAT, LDRES,
*      &          PTRE, LDRES, REAR, REZN, LDREAR,
*      &          SKSC, XF, FILNAM(PN), NTLS,
*      &          IN, OU_NT, PN)
*
*-----2. Channel flow bounds and stream routing
*      coefficients
*
*      PN = 2
*      CALL NETWK_2(NNODS, NDNAM, NDTYP, LDND,
*      &          NARCS, II, JJ, HI, LO, COST, ARTYP, LDARC,
*      &          NARCND, NSTRM, STRMCF, STRMAR, LDSTRM,
*      &          PTDWAR, LDND, NDWAR, NDDWAR, LDARC,
*      &          NCTAR, CTARFW, LDCTAR,
*      &          PERD, CONST, XF, FILNAM(PN), NTLS,
*      &          IN, OU_NT, PN)
*      IF (NSTRM .GT. 0) FLWARC = .TRUE.
*      ARCS = NARCS
*      MXCST = -9999
*      DO I = 1, NARCS
*
*          IF (COST(I) .GT. MXCST) MXCST = COST(I)
*
*      ENDDO
*      DO 16 I = 1, NRES
*
*          OINST(I) = INST(I)
*
*      16 CONTINUE
*
*-----3. Read Channel geometry data such as length, roughness
*      slope, riverbed hydraulic conductivity.
*
*      PN = 3
*      CALL STRM_DAT(NDNAM, LDND, II, JJ, LDARC,
*      &          NSTRM, STRMAR, LDSTRM,
*      &          NSTR, STRDIR, STRDAT, LDSTR,
*      &          FILNAM(PN), NTLS, IN, OU_NT, PN)
*      CALL STRM_KX(NSTRM, STRMAR, STRMCF, LDSTRM,
*      &          NSTR, STRDIR, STRDAT, LDSTR)
*
*-----4. Read parameters for hydraulic structures
*
*      PN = 4
*      CALL HYDR_DAT(NNODS, NDNAM, LDND, JJ, LDARC,
*      &          PTDWAR, LDND, NDDWAR, LDARC,
*      &          NSTRM, STRMAR, LDSTRM,
*      &          NHYTP, HYTP, LDHYTP,
*      &          NHY, HYDIR, HYTPCD, HYDAT, LDHY,
*      &          FILNAM(PN), NTLS, IN, OU_NT, PN,
*      &          CTERM, COLSTR, LDCOL)
*
*-----5. Surface runoff parameters
*
*      PN = 5
*      CALL RNOF_DAT(NDNAM, LDND,
*      &          NRNOF, RNOFND, LDRNOF, RNOFTB, LDRFTB, ASDR,
*      &          FILNAM(PN), NTLS, IN, OU_NT, PN,
*      &          CTERM, COLSTR, LDCOL)
*
*-----10. Seasonal target water demands
*
*      PN = 10
*      WSFLAG = .FALSE.
*      CALL TWS_DAT(NNODS, NDNAM, LDND, NWSND, WSND, LDWS, NPER, WSTB,
*      &          LDP, WSUNIT, WSFLAG, PN, FILNAM(PN), NTLS, IN, OU_NT,
*      &          UNITNM_1, LDUNIT, CTERM, COLSTR, LDCOL)
*
*-----11. Seasonal surface water evaporation coefficients
*
*      PN = 11
*      CALL RESEV_DAT (NNODS, NDNAM, LDND, NEV, EVND, LDEV,
*      &          NPER, EVTB, LDP, EVUNIT, EVFLAG,
*      &          FILNAM(PN), NTLS, IN, OU_NT, PN,
*      &          UNITNM_3, LDUNIT, CTERM, COLSTR, LDCOL)
*
*-----12. Seasonal flow bounds
*
*      PN = 12
*      CALL FB_DAT(II, JJ, ARTYP, LDARC, NDNAM, LDND,
*      &          NFBAR, FBAR, LDFBAR, FBTB, LDFBTB,
*      &          FBUNIT, FBFLAG, FILNAM(PN), NTLS, IN, PN, OU_NT,
*      &          PTDWAR, LDND, NDDWAR, LDARC,
*      &          UNITNM_1, LDUNIT, CTERM, COLSTR, LDCOL)
*
*-----13. Seasonal rule curve

```



```

*
  PN = 13
  CALL RC_DAT(NNODS, NDNAM, LDND,
&             NRCND, RCND, LDRC, NPER, RCTB, LDRCTB, RCUNIT, RCFLAG,
&             FILNAM(PN), NTLN, IN, OU_NT, PN,
&             UNITNM_2, LDUNIT, CTERM, COLSTR, LDCOL)
*
*-----Open time-dependent data file
*
*
*-----1. Open local incremental inflow file
*
  CALL FIL_HEAD(NNODS, NDNAM, LDND, NIFW, IFWND, LDIFW, IFWCD,
&              UNITNM_1, LDUNIT, FWFLAG, FLAG, FILNAM(IN_IFW),
&              NTLN, IN_IFW, OU_NT, 'local incremental inflow',
&              CTERM, COLSTR, LDCOL)
*
*-----2. Open a rainfall data file
*
  CALL FIL_HEAD(NNODS, NDNAM, LDND, NRAIN, RAINND, LDRAIN, RAINTY,
&              RNUNIT, LDUNIT, RNFLAG, FLAG, FILNAM(IN_RN),
&              NTLN, IN_RN, OU_NT, 'precipitation',
&              CTERM, COLSTR, LDCOL)
*
*-----Open water surface coefficient file
*
  CALL FIL_HEAD(NNODS, NDNAM, LDND, NEV, EVND, LDEV, EVUNIT,
&              UNITNM_3, LDUNIT, EVFLAG, EVFIL, FILNAM(IN_EV),
&              NTLN, IN_EV, OU_NT,
&              'water-surface evaporation coefficient',
&              CTERM, COLSTR, LDCOL)
*
*-----Open target water demand file
*
  CALL FIL_HEAD(NNODS, NDNAM, LDND, NWSND, WSND, LDWS, WSUNIT,
&              UNITNM_1, LDUNIT, WSFLAG, WSFIL, FILNAM(IN_WS),
&              NTLN, IN_WS, OU_NT, 'target water-demand',
&              CTERM, COLSTR, LDCOL)
*
*-----Open rule curve data file
*
  CALL FIL_HEAD(NNODS, NDNAM, LDND, NRCND, RCND, LDRC, RCUNIT,
&              UNITNM_2, LDUNIT, RCFLAG, RCFIL, FILNAM(IN_RC),
&              NTLN, IN_RC, OU_NT, 'rule-curve elevation',
&              CTERM, COLSTR, LDCOL)
*
*-----Open flow bound data file
*
  CALL FB_FIL(NDNAM, LDND, II, JJ, ARTYP, LDARC,
&            PTDWAR, LDND, NDDWAR, LDARC,
&            NFXAR, FXAR, LDFXAR, FBUNIT,
&            UNITNM_1, LDUNIT, FBFLAG, FBFIL, FILNAM(IN_FB),
&            NTLN, IN_FB, OU_NT, CTERM, COLSTR, LDCOL)
*
*-----Open a groundwater level data file
*
  CALL FIL_HEAD(NNODS, NDNAM, LDND, NGWND, GWND, LDGWND, GWTYPE,
&              GWUNIT, LDUNIT, GWFLAG, FLAG, FILNAM(IN_GW),
&              NTLN, IN_GW, OU_NT, 'ground-water-level',
&              CTERM, COLSTR, LDCOL)
*
*-----Open a fixed flow file
*
  CALL FX_FIL(NDNAM, LDND, II, JJ, ARTYP, LDARC,
&            PTDWAR, LDND, NDDWAR, LDARC,
&            NFXAR, FXAR, LDFXAR,
&            FXUNIT, UNITNM_1, LDUNIT, FXFLAG,
&            FILNAM(IN_FX), NTLN, IN_FX, OU_NT,
&            CTERM, COLSTR, LDCOL)
*
*-----WRITE BASIC NETWORK INFORMATION
*
  IF (SAVOPT .EQ. 0 .OR. SAVOPT .EQ. 1) THEN
    CALL PRNTNF (NDNAM, NDTP, NDSEQ, NRES, 0, LDND,
&              LDARC, LDRES,
&              II, JJ, HI, LO, COST, ARTYP, PTRE, REAR,
&              NNODS, PTDWAR, NDDWAR,
&              PERD, NARCS, CONST, XF, OU_NT)
  ENDIF
*
*-----Open single nodal/arc budget list files
*
  CALL SNBL(NNODS, NDNAM, LDND, II, JJ, LDARC,
&          PTDWAR, LDND, NDDWAR, LDARC,
&          NSTRM, STRMAR, LDSTRM,
&          NSNBL, SNBLND, LDSNBL, SNBLFG, FILNAM(OU_SN))
  CALL SABL(NDNAM, LDND, PTDWAR, II, JJ, NDDWAR, LDARC,
&          NSTRM, STRMAR, LDSTRM,
&          NSABL, SABLND, LDSABL, SABLFG,
&          FILNAM(OU_SA))
*
  DO I = 1, NARCS
    OHI(I) = HI(I)
  ENDDO

```

```

*
*-----BEGIN TO CALCULATION
*
30 PRINT *, 'Begin simulation'
CONTINUE
NOTCOV = .TRUE.
NOP = NOP + 1
IF (NOP .GT. 1) THEN
    PRINT '(A, I4)', ' Finish simulation time period:', NOP-1
ENDIF
IF (NOP .GT. NSPS) GOTO 80
MTH = NOP + STMO - 1
MTH = MOD (MTH, NPER)
IF (MTH .EQ. 0) MTH = NPER

*
*      PREPARE THE TIME FOR OUTPUT
*
IF (STMO .EQ. 1 .AND. NOP .EQ. 1) THEN
    YEAR = YEAR - 1
END IF
IF (MTH .EQ. 1) THEN
    YEAR = YEAR + 1
END IF
IF (NPER .EQ. 12) THEN
    CALL MONTH(2, MTH, MNTH)
ELSE
    WRITE(MNTH, '(I4)') MTH
ENDIF
DO I = 1, LDND
    DO J = 1, 6
        NDXAR(I, J) = 0
    ENDDO
ENDDO

*
*      CALL SUBROUTINE NVAR TO GET THE NET INFLOW TO NODES
*      AND CREATE THE CORESPONGING NET INFLOW ARCS.
*
CALL NVAR (NNODS, NDNAM, NDTYP, NDSEQ, LDND,
&          NARCS, II, JJ, HI, LO, COST, ARTYP, LDARC,
&          MTH, IN_IFW, LAST, LDRES,
&          INST, RC, SKSC,
&          NDXAR, PERD, XF,
&          NIFW, IFWND, LDIFW,
&          PTRE, LDRES, REAR, REZN, LDREAR,
&          NRCND, RCND, LDRC, RCTB, LDRCTB, RCFLAG, RCFIL,
&          IN_RC,
&          APRX, OU_NT,
&          RPOOL, LDPL, CTERM, COLSTR, LDCOL)
IF (ERR) GO TO 99
IF (FBFLAG) THEN
    CALL FB_ARC(LO, HI, ARTYP, LDARC,
&              NFBAR, FBAR, LDFBAR, FBTB, LDFBTB, FBFIL,
&              MTH, PERD, XF, IN_FB,
&              CTERM, COLSTR, LDCOL)
ENDIF

*
*-----Define rainfall arcs
*
IF (RNFLAG) THEN
    CALL RNOF_ARC(NDTYP, NDSEQ, LDND,
&                NARCS, II, JJ, LO, HI, COST, ARTYP, LDARC,
&                INST, LDRES,
&                NDXAR, LDND, NRAIN, RAINND, LDRAIN, RAINTY,
&                NRNOF, RNOFND, LDRNOF, RNOFTB, LDRFTB,
&                A5DR, PERD,
&                SKSC, XF, IN_RN,
&                CTERM, COLSTR, LDCOL)
ENDIF

*
*-----Define reservoir water surface EV arc
*
IF (EVFLAG) THEN
    CALL RESEV_ARC(NDTYP, NDSEQ, LDND,
&                 NARCS, II, JJ, LO, HI, COST, ARTYP, LDARC,
&                 INST, LDRES,
&                 NDXAR, LDND, NEV, EVND, LDEV, EVTB, LDP,
&                 MTH, SKSC, PERD, EVFIL, XF, IN_EV,
&                 CTERM, COLSTR, LDCOL)
ENDIF

*
*-----Target water demand arcs
*
IF (WSFLAG) THEN
    CALL TWS_ARC(NARCS, II, JJ, LO, HI, COST, ARTYP, LDARC,
&               NWSND, WSND, LDWS, WSTB, LDP, WSFIL,
&               NDXAR, LDND, MTH, SKSC, PERD, MXCST, XF, IN_WS,
&               CTERM, COLSTR, LDCOL)
ENDIF

*
*-----Define Reservoir seepage arcs
*
IF (GWFLAG) THEN
    CALL GW_DAT(NGWND, GWND, GWLVL, LDGWND, GWTYPE, IN_GW,
&              CTERM, COLSTR, LDCOL)

```

```

ENDIF
CALL RESLOS(
&      NARCS, II, JJ, LO, HI, COST, ARTYP, LDARC,
&      NRES, PTRES, INST, RESDAT, LDRES,
&      NGWND, GWND, GWLVL, LDGWND, GWTYPE,
&      NDXAR, LDND, SKSC, PERD, XF)
*
*-----Calculate the overflow over weir
*
CALL HYDR_OFW(NDSEQ, LDND,
&      II, HI, LO, OHI, LDARC,
&      NHY, HYTPCD, HYDAT, HYOUT, LDHY,
&      OINST, LDRES,
&      PERD, CONST, XF, OU_NT)
*
*-----Assign flows for fixed flow arcs
*
IF (FXFLAG) THEN
CALL FX_ARC(HI, LO, COST, LDARC, NFXAR, FXAR, LDFXAR, FXUNIT,
&      XF, PERD, IN_FX,
&      RPOOL, LDPL, CTERM, COLSTR, LDCOL)
ENDIF
*
*      Flow dependent arcs such as Channel seepage arcs
*
IF (FLWARC) THEN
      NITR = 0
      OARCS = NARCS
      DO I = 1, LDARC
        OFLOW(I) = 0
      ENDDO
40  ENDIF
CONTINUE
IF (FLWARC) THEN
      NARCS = OARCS
      IF (EVFIL) THEN
        I = 0
      ELSE
        I = MTH
      ENDIF
*
*
CALL STRM_ROUT(
&      NARCS, II, JJ, HI, LO, COST, FLOW, ARTYP, LDARC,
&      NSTRM, STRMCF, STRMAR, LDSTRM,
&      NSTR, STRDIR, STRDAT, LDSTR,
&      NGWND, GWND, GWLVL, LDGWND,
&      NEV, EVND, LDEV, I, EVTB, LDP,
&      SKSC, PERD, XF)
ENDIF
*
*
SOLVE THE NETWORK BY USING THE OUT OF KILTER TECHNIQUE
CALL KLTR(II, JJ, HI, LO, COST, FLOW, LDND, NARCS, DEBUG, KARC,
&      IFAULT)
IF (IFAULT .NE. 0) THEN
      STOP
ENDIF
IF (DEBUG) THEN
      IF (FLWARC .AND. NITR .LT. LDITR) THEN
        NITR = NITR + 1
        GOTO 40
      ELSE
        CALL OUTPUT2(NDNAM, LDND, II, JJ, HI, LO, COST, FLOW,
&      ARTYP, NARCS)
        PRINT *, '*****SOLUTION IS INFEASIBLE*****'
        PRINT *, 'CHECK THE ARC FROM ', NDNAM(II(IABS(KARC))),
&      ' TO ', NDNAM(JJ(IABS(KARC))), 'WITH ARC NUMBER:',
&      ABS(KARC)
        PRINT *, 'THE CURRENT TIME PERIOD IS ', NOP
        PRINT *, 'THE LOCAL ITERATION IS ', NITR
        STOP
      ENDIF
      STOP !CALL EXIT
ENDIF
END IF
IF (FLWARC) THEN
      CALL FLWCK(NNODS, LDND,
&      PTDWAR, NDDWAR, LDARC,
&      FLOW, OFLOW, FCRT, NOTCOV)
*
      IF (NOTCOV) THEN
        NITR = NITR + 1
        IF (NITR .LE. LDITR) THEN
          GOTO 40
        ELSE
          GOTO 43
        ENDIF
      ENDIF
43  ENDIF
CONTINUE
*
*
CALCULATE STAGES OF PONDS

```

```

*
*      CALL RESSTG(NNODS, NDTYP, NDSEQ, LDND,
&      RC, INST, PTRE, LDRES, REAR, LDREAR,
&      FLOW, LDARC, XF)
*
*      Calculate nodal budgets and save into a file
*
*      CALL NDBUD(FLOW, LDARC, RC, OINST, INST, RESDAT, LDRES,
&      NNODS, NDTYP, NDSEQ, PTDWAR, LDND, NDDWAR, LDARC,
&      NDWAR, NODBUD, XF, OU_NT)
*      CALL PRTND(NNODS, NDNAM, NDTYP, NODBUD, LDND, NOP, XF, XP,
&      NSNBL, SNBLND, LDSNBL, SNBLFG, NDBFLG, ZEROFG, OU_ND)
*
*      Calculate arc budgets
*
*      CALL ARBUD(NNODS, PTDWAR, LDND, NDDWAR, LDARC,
&      II, JJ, FLOW, ARCBUD, LDARC,
&      NSTRM, STRMAR, LDSTRM)
*      CALL PTARBD(II, JJ, ARCBUD, NDDWAR, LDARC,
&      NNODS, NDNAM, PTDWAR, LDND,
&      NSABL, SABLND, LDSABL,
&      NSNBL, SNBLND, LDSNBL, SNBLFG,
&      NOP, XF, XP, ARBFLG, PERD, CONST, OU_AR)
*
*      Flow in the hydraulic structures
*
*      IF (NHY .GT. 0) THEN
&      CALL HYDR_HITE(
&      ARCBUD, LDARC,
&      NHY, HYTPCD, HYDAT, HYOUT, LDHY,
&      PERD, CONST, XF)
&      CALL HYDR_PRN(HYTP, LDHYTP,
&      NHY, HYDIR, HYTPCD, HYDAT, HYOUT, LDHY, NOP,
&      HYBFLG, OU_HY)
&      ENDIF
*
*-----System water budget
*
*      CALL SAVBUD(NNODS, NDNAM, NDTYP, NODBUD, PTDWAR, LDND,
&      II, JJ, ARCBUD, NDDWAR, LDARC,
&      NOP, NSPS, XF, XP, OU_NT,
&      CNDBT, CARBT, CSNDBT, LDND, CSARBT, LDARC)
*
300  CONTINUE
    DO I = 1, NRES
&      OINST(I) = INST(I)
&
&      ENDDO
&      DO I = 1, NSTRM
&      ARC = STRMAR(I, 5)
&      STRMCF(I, 0) = ISGN(ARC)*FLOW(IABS(ARC))/XF
&
&      ENDDO
&      IF (NARCS .GT. ARCS) THEN
&      DO I = ARCS + 1, NARCS
&      II(I) = 0
&      JJ(I) = 0
&      HI(I) = 0
&      LO(I) = 0
&      FLOW(I) = 0
&      COST(I) = 0
&      ENDDO
&
&      ENDIF
&      NARCS = ARCS
&      GO TO 30
80    CONTINUE
    CALL SVINFO(FILNAM, LDFIL, NHY, NSNBL, NSABL,
&      NDBFLG, ARBFLG, HYBFLG)
99    STOP
    END
*=====
SUBROUTINE GETINT(IVAR, IN, IFAULT)
IMPLICIT NONE
INTEGER IVAR, IN, IFAULT
*
*      CHARACTER CTERM*50
*
*      CTERM = ' '
*      IFAULT = 1
5    READ (IN, '(a)', END = 5) CTERM
    CONTINUE
    IF (CTERM .NE. ' ') THEN
&      BACKSPACE(IN)
&      READ (IN, *) IVAR
&      IFAULT = 0
&
&      ENDIF
&      RETURN
&      END
*=====
* Name:      arbud
* Purpose:   Calculate the water budget for arcs.
* Author:    Xiaodong Jian
* Date:      1/16/95
*=====
SUBROUTINE ARBUD(NND, PTDWAR, LDND, DWAR, LDDWAR,

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```

&      II, JJ, FLOW, ARCBUD, LDARC,
&      NSTRM, STRMAR, LDSTRM)
IMPLICIT NONE
INTEGER NND, LDND, PTDWAR(LDND, 2), LDDWAR, DWAR(LDDWAR)
INTEGER LDARC, II(LDARC), JJ(LDARC), FLOW(LDARC),
& ARCBUD(LDARC, 0:6)
& INTEGER NSTRM, LDSTRM, STRMAR(LDSTRM, 0:6)
*
INTEGER I, J, N, STRM, DWND, ARC, LIM1, LIM2, ARC2
INTEGER ISGN
LOGICAL UNSTRM
*
DO 100 N = 1, NND
      LIM1 = PTDWAR(N, 1)
      LIM2 = PTDWAR(N, 2)
      DO 50 J = LIM1, LIM2
        ARC = DWAR(J)
*
*      Check if the current arc is a stream arc.
*
      UNSTRM = .TRUE.
      I = 1
      DO WHILE (I .LE. NSTRM .AND. UNSTRM)
        IF (ARC .EQ. STRMAR(I, 1)) THEN
          UNSTRM = .FALSE.
          STRM = I
          ENDIF
          I = I + 1
        ENDDO
*
*-----Calculate the arc water budget.
*
      IF (UNSTRM) THEN
        IF (ARC .GT. 0) THEN
          DWND = JJ(ARC)
          ELSE
            ARC = -ARC
            DWND = II(-ARC)
          ENDIF
          ARCBUD(ARC, 0) = DWND
          ARCBUD(ARC, 1) = FLOW(ARC)
          ARCBUD(ARC, 2) = 0
          ARCBUD(ARC, 3) = 0
          ARCBUD(ARC, 4) = 0
          ARCBUD(ARC, 5) = 0
          ARCBUD(ARC, 6) = FLOW(ARC)
          ELSE
            ARC2 = STRMAR(STRM, 6)
            IF (ARC2 .GT. 0) THEN
              DWND = JJ(ARC2)
            ELSE IF (ARC2 .LT. 0) THEN
              DWND = II(-ARC2)
            ENDIF
            ARC = IABS(ARC)
            ARCBUD(ARC, 0) = DWND
            DO I = 1, 6
              ARC2 = STRMAR(STRM, I)
              IF (ARC2 .NE. 0) THEN
                ARCBUD(ARC, I) = ISGN(ARC2) * FLOW(IABS(ARC2))
              ELSE
                ARCBUD(ARC, I) = 0
              ENDIF
            ENDDO
          ENDIF
        ENDIF
50      CONTINUE
100     CONTINUE
      RETURN
      END
*-----
* Name:      ptarbd
* Purpose:   print arc water budget into a file.
* Author:    Xiaodong Jian
* Date:      1/16/96
*-----
      SUBROUTINE PTARBD(II, JJ, ARCBUD, NDDWAR, MXARC,
&      NNODS, NDNAM, PTDWAR, MXND,
&      NSABL, SABLND, LDSABL,
&      NSNBL, SNBLND, LDSNBL, SNBLFG,
&      NOP, XF, XP, ARBFLG, PERD, CONST, IOUT)
IMPLICIT NONE
INTEGER MXARC, MXND, NNODS, NOP, XP, IOUT
INTEGER II(MXARC), JJ(MXARC), ARCBUD(MXARC, 0:6)
INTEGER PTDWAR(MXND, 2), NDDWAR(MXARC)
REAL XF, PERD, CONST
CHARACTER NDNAM(MXND)*(*)
*
INTEGER NSABL, LDSABL, SABLND(LDSABL, 3)
INTEGER NSNBL, LDSNBL, SNBLND(LDSNBL, 3)
LOGICAL SNBLFG, ARBFLG
*
INTEGER I, J, K, L, N, OJ, ARC, LIM1, LIM2, BUD(6)
INTEGER OU, NOFW, OFWID, OFW(20), NARC
*
CHARACTER FMT*40, FMT2*30, OFWFMT*30

```



```

*
      IF (CN2INT(I,OJ,NSABL,SABLND(1,1),SABLND(1,2),L)) THEN
        OU = SABLND(L, 3)
        IF (XP .GT. 0) THEN
          WRITE(OU, FMT2) NOP,
&          (BUD(L)/XF, L=1,6), BUD(6)/XF/PERD/CONST
        ELSE
&          WRITE(OU, FMT2) NOP,
          (BUD(L), L=1,6), BUD(6)/PERD/CONST
        ENDIF
      ENDIF

*-----Save downstream release.
*
      IF (OFWFLG) THEN
        NOFW = NOFW + 1
        OFW(NOFW) = BUD(1)
      ENDIF
50      CONTINUE

      IF (OFWFLG) THEN
        OU = SNBLND(OFWID, 3)
        IF (XP .GT. 0) THEN
&          WRITE(OU, OFWFMT) NOP,
          (OFW(L)/XF, OFW(L)/XF/PERD/CONST, L = 1, NOFW)
        ELSE
&          WRITE(OU, OFWFMT) NOP,
          (OFW(L), OFW(L)/PERD/CONST, L = 1, NOFW)
        ENDIF
      ENDIF

100     CONTINUE
      RETURN
901     FORMAT (I4, 1X, 2A12, T30, 6F10.2)
900     FORMAT(/,T20, 'Canal water budgets for time step:', I3,
&           /,T20, '=====')
&           /,T30, 'Initial      Canal      Surface',
&           /,T30, 'Outflow',
&           /,T30, 'Inflow storage seepage ration storage',
&           /,T30, 'Outflow (cubic',
&           /,T30, '(acre- (acre- (acre- (acre- (acre-',
&           /,T30, '(acre- feet_per',
&           /, 'No. From', T18, 'To',
&           /,T30, 'feet) feet) feet) feet) feet)',
&           /,T30, 'feet) second)',
&           /, '-----', T18, '-----',
&           /,T30, '-----')
&           /,T30, '-----')
      END

*=====
* Name:      setzva
* Purpose:   Get pond characteristic data of elevation, volume,,
*            and surface area from a file.
* Author:    Xiaodong Jian
* Date:      04/07/97
*=====
      SUBROUTINE SETZVA(NRES, NNODS, NDNAM, NDTYP, LDND,
&          FILNAM, NTLS, OU)
      IMPLICIT NONE
      INTEGER NRES, NNODS, LDND, NDTYP(LDND), NTLS, OU
      CHARACTER NDNAM(LDND)*(*), FILNAM*(*)

      INTEGER IU, I
      INTEGER ZVAMTH, SAVOPT
      COMMON /ZVAMTH/ ZVAMTH
      COMMON /SAVOPT/ SAVOPT

*-----Open a data file
*
      IU = 9
      CALL IO_OPFIL(IU, 1, FILNAM,
&          'Enter pond characteristics file: ')

*-----Skip title lines
*
      DO I = 1, NTLS
        READ (IU, *)
      ENDDO

*-----Read data index
*
      READ (IU, *, ERR = 99) ZVAMTH

*-----Read corresponding data according to data index
*
      IF (ZVAMTH .EQ. 0) THEN
&          CALL SETTAB(NRES, NNODS, NDNAM, NDTYP, LDND,
          IU, OU, SAVOPT)
      ELSE IF (ZVAMTH .EQ. 1) THEN
&          CALL SETEQS(NRES, NNODS, NDNAM, NDTYP, LDND,
          IU, OU, SAVOPT)
      ENDIF
99     RETURN

```



```

      IF (CTERM .NE. ' ') THEN
          NCOL = NCOL + 1
          K = J + NCOL
          BACKSPACE(IU)
          READ(IU, *) ELE, CAP, AREA
          RETB(1,K) = ELE
          RETB(2,K) = CAP
          RETB(3,K) = AREA
          IF (ELE .LT. MINV) MINV = ELE
          IF (ELE .GT. MAXV) MAXV = ELE
          GO TO 15
      END IF
      PTRETB(2, NRES+1) = PTRETB(2, NRES) + NCOL
      J = J + NCOL
*
      IF (SAVOPT .EQ. 0 .OR. SAVOPT .EQ. 1) THEN
          WRITE (OU, 801) NRES, NAME, NCOL, MINV, MAXV
801      FORMAT(I5, 2X, A12, 1X, I10, 2F10.2)
      ENDIF
      GO TO 5
90      CONTINUE
      CLOSE(IU)
      RETURN
      END
=====
* Name:      gettab
* Purpose:   Get the Z-V-A table to interpret the Z-V-A for a given
*            node.
* Author:    Xiaodong Jian
* Date:      4/9/97
=====
      SUBROUTINE GETTAB(GETZVA, RESND, ZVAIDX, ZVA, OU)
      IMPLICIT NONE
      INTEGER RESND, ZVAIDX, OU
      REAL ZVA(3)
      LOGICAL GETZVA
*
*-----Local variables
*
      REAL X, X1, Y1, X2, Y2, VAL, VAL1, DIF
      INTEGER I, N, K, LIM
      CHARACTER ZVANAM(3)*9
      DATA ZVANAM/'ELEVATION', 'VOLUME', 'AREA'/
*
*-----Common block for Z-V-A table. This common block must be
*            the same as the subroutine settab.
*
      INTEGER PTRETB(2, 100)
      REAL RETB(3, 5000)
      CHARACTER NDNAM(300)*12
      COMMON /ZVADAT/ PTRETB, RETB
      COMMON /NDNAME/ NDNAM
*
*-----Check whether there is Z-V-A for the current node
*
      N = 1
      DO WHILE (PTRETB(1,N) .NE. RESND .AND. PTRETB(1, N) .NE. 0)
          N = N + 1
      ENDDO
      IF (PTRETB(1,N) .NE. RESND) THEN
          GETZVA = .FALSE.
          GOTO 99
      ENDIF
      GETZVA = .TRUE.
      K = PTRETB(2, N)
      LIM = PTRETB(2, N+1) - 1
*
      VAL = ZVA(ZVAIDX)
      IF (VAL .LT. 0) VAL = 0.0
      IF (RETB(ZVAIDX, K) .GT. VAL .OR. RETB(ZVAIDX, LIM) .LT. VAL) THEN
          GETZVA = .FALSE.
          PRINT '(A)', CHAR(7)
          CALL STR_LEN(ZVANAM(ZVAIDX), I)
          WRITE (OU, 901) ZVANAM(ZVAIDX)(1:I), VAL, NDNAM(RESND), RESND,
&              ZVANAM(ZVAIDX)(1:I), RETB(ZVAIDX,K),
&              ZVANAM(ZVAIDX)(1:I), RETB(ZVAIDX, LIM)
          WRITE (*, 901) ZVANAM(ZVAIDX)(1:I), VAL, NDNAM(RESND), RESND,
&              ZVANAM(ZVAIDX)(1:I), RETB(ZVAIDX,K),
&              ZVANAM(ZVAIDX)(1:I), RETB(ZVAIDX, LIM)
901      FORMAT('***ERROR***'
&            /, ' TRANSFORMING ', A, ' = ', F10.2,
&            /, ' TO ITS CORRESPONDING PARAMETERS AT POND: ', A,
&            /, ' WITH THE NODAL NUMBER: ', I2,
&            /, ' THE MINIMUM ', A, ' IN Z-V-A TABLE = ', F10.2,
&            /, ' THE MAXIMUM ', A, ' IN Z-V-A TABLE = ', F10.2)
          STOP
*
          goto 99
      ENDIF
10      CONTINUE
      VAL1 = RETB(ZVAIDX,K)
      DIF = VAL1 - VAL
      IF (DIF .LT. 0) THEN
          IF (K .GT. LIM) THEN
              GETZVA = .FALSE.

```

```

                                GO TO 99
                                ENDIF
                                K = K + 1
                                GO TO 10
ELSE IF (DIF .EQ. 0) THEN
                                DO I = 1, 3
                                ZVA(I) = RETB(I, K)
                                ENDDO
                                GO TO 50
ELSE
                                X = VAL
                                X1 = RETB(ZVAIDX,K-1)
                                X2 = RETB(ZVAIDX,K)
                                DO I = 1, 3
                                Y1 = RETB(I, K-1)
                                Y2 = RETB(I, K)
                                ZVA(I) = Y1 + (Y2 - Y1) * (X - X1) / (X2 - X1)
                                ENDDO
                                GO TO 50
END IF
99 CONTINUE
50 RETURN
END

=====
* Name:      seteqs
* Purpose:   Read the Z-V-A regression equation coefficients into the
*            program from the file.
* Author:    Xiaodong Jian
* Date:      04/09/97
=====
SUBROUTINE SETEQS(NRES, NNODS, NDNAM, NDTYP, LDND,
&                IU, OU, SAVOPT)
IMPLICIT NONE
INTEGER NRES, NNODS, LDND, NDTYP(LDND), IU, OU, SAVOPT
CHARACTER NDNAM(LDND)*(*)
*
*-----Local variables
*
INTEGER I, N, NWND, NEQS
CHARACTER NAME*30, CTERM*100
LOGICAL CN
*
*-----Common block for Z-V-A equations for QNWR.
*      This common block must be the same as the subroutine geteqs
*
INTEGER ZVAPTR(2, 100)
REAL ZVAEQS(0:3, 10, 100)
COMMON /ZVAQNWR/ZVAPTR, ZVAEQS
*
NRES = 0
NNODS = 0
NWND = 0
C
J = 0
*
*-----Set output file titles
*
IF (SAVOPT .EQ. 0 .OR. SAVOPT .EQ. 1) THEN
WRITE(OU, 800)
800 FORMAT(
& //, 'SUMMARY OF POND ELEVATION-CAPACITY-AREA RELATIONS:',
& //, '=====',
& //, 'Z-V-A relations is expresses as regression equation')
ENDIF
*
*-----Read pond-node name
*
5 CONTINUE
NAME = ' '
DO WHILE (NAME .EQ. ' ')
READ (IU, '(A)', END = 90) NAME
ENDDO
IF (CN ('FINISH', NAME, 1)) THEN
GOTO 90
ENDIF
*
*-----Add a new node.
*
NRES = NRES + 1
NNODS = NNODS + 1
NWND = NNODS
CALL STR_CORS(NAME, 1)
NDNAM(NWND) = NAME
NDTYP(NWND) = 1
ZVAPTR(1, NRES) = NWND
ZVAPTR(2, NRES) = 0
*
*-----Read the base elevations, and coefficients of regression equations.
*
NEQS = 0
15 READ(IU, '(a)') CTERM
DO WHILE (CTERM .NE. ' ')
NEQS = NEQS + 1
BACKSPACE(IU)
READ (IU, *) N, (ZVAEQS(I, NEQS, NRES), I = 0, 3)

```

```

                READ (IU, '(a)', END= 20) CTERM
ENDDO
20 IF (NEQS .NE. 0) THEN
                ZVAPTR(2, NRES) = NEQS
ENDIF
*
*-----Read another pond node
*
        GOTO 5
*
90 CONTINUE
CLOSE(IU)
RETURN
END
=====
* Name:      geteqs
* Purpose:    Get the equations to interpret the Z-V-A for a given
*             node.
* Author:     Xiaodong Jian
* Date:       4/9/97
=====
SUBROUTINE GETEQS(GETZVA, RESND, ZVAIDX, ZVA, OU)
IMPLICIT NONE
INTEGER RESND, ZVAIDX, OU
REAL ZVA(3)
LOGICAL GETZVA
*
*-----Local variables
*
        INTEGER N
        INTEGER RES, NEQS, IFAULT
*
*-----Common block for Z-V-A equations for QNWR.
* This common block must be the same as the main program opends
* and subroutine getzva
*
        INTEGER ZVAPTR(2, 100)
        REAL ZVAEQS(0:3, 10, 100)
        CHARACTER NDNAM(300)*12
        COMMON /ZVAQNWR/ ZVAPTR, ZVAEQS
        COMMON /NDNAME/ NDNAM
*
*-----Check whether there is Z-V-A equations for the current node
*
        N = 1
        DO WHILE (ZVAPTR(1,N) .NE. RESND .AND. ZVAPTR(1, N) .NE. 0)
                N = N + 1
        ENDDO
        IF (ZVAPTR(1,N) .EQ. RESND .AND. ZVAPTR(2,N) .GT. 0) THEN
                RES = N
                NEQS = ZVAPTR(2,N)
                GETZVA = .FALSE.
                GOTO 99
        ELSE
                GETZVA = .FALSE.
                GOTO 99
        ENDIF
*
*-----Get Z, V, and A
*
        CALL CALZVA(NEQS, ZVAEQS(0, 1, RES), ZVAIDX, ZVA, IFAULT)
        IF (IFAULT .NE. 0) THEN
                GETZVA = .FALSE.
                WRITE(OU, 900)
        ELSE
                GETZVA = .TRUE.
        ENDIF
*
900 FORMAT('***Errorr*** in transforming Z-V-A')
*
99 CONTINUE
50 RETURN
END
SUBROUTINE CALZVA(NEQS, A, ZVAIDX, ZVAOUT, IERR)
IMPLICIT NONE
INTEGER NEQS, ZVAIDX, IERR
REAL A(0:3, NEQS)
REAL ZVAOUT(3)
*
        REAL VAL, ZVA(3)
        REAL Z1, Z2
        INTEGER I
*
        IF (ZVAIDX .EQ. 1) THEN ! Z --> V, AND A.
                ZVA(1) = ZVAOUT(1)
                CALL Z2VA(NEQS, A, ZVA, IERR)
        ELSE IF (ZVAIDX .EQ. 2 .OR. ZVAIDX .EQ. 3) THEN
                VAL = ZVAOUT(ZVAIDX)
                Z1 = A(0, 1)
*
*-----find z2
*
                ZVA(1) = A(0, NEQS)
                CALL Z2VA(NEQS, A, ZVA, IERR)
                DO WHILE (ZVA(ZVAIDX) .LT. VAL)
                        ZVA(1) = ZVA(1) + 1
                CALL Z2VA(NEQS, A, ZVA, IERR)

```

```

                                ENDDO
                                Z2 = ZVA(1)
*-----Binary search for the elevation.
*
                                DO WHILE (ABS(Z2 - Z1) .GT. 0.005)
                                    ZVA(1) = 0.5 * (Z1 + Z2)
                                    CALL Z2VA(NEQS, A, ZVA, IERR)
                                    IF (ZVA(ZVAIDX) .LT. VAL) THEN
                                        Z1 = ZVA(1)
                                    ELSE IF (ZVA(ZVAIDX) .GT. VAL) THEN
                                        Z2 = ZVA(1)
                                    ELSE
                                        GOTO 100
                                    ENDIF
                                ENDDO
100  ENDIF
    CONTINUE
    DO I = 1, 3
        IF (I .NE. ZVAIDX) THEN
            ZVAOUT(I) = ZVA(I)
        ENDIF
    ENDDO
    RETURN
END
SUBROUTINE Z2VA(NEQS, A, ZVA, IERR)
    IMPLICIT NONE
    INTEGER    NEQS, IERR
    REAL       A(0:3, NEQS), ZVA(3)
*
    REAL       X
    INTEGER    I
*
    IERR = 0
    DO 20 I = NEQS, 1, -1
        IF (ZVA(1) .GE. A(0,I)) THEN
            X = ZVA(1) - A(0,I)
            ZVA(2) = (A(3, I) * X + A(2,I)) * X + A(1,I)
            ZVA(3) = A(2,I) + X * (A(3,I) + A(3,I))
            GOTO 99
        ENDIF
    20 CONTINUE
    IERR = 1
99  RETURN
END
=====
* Name:      chdep
* Purpose:   Compute depth of flow using Manning's Equation
* Author:    Xiaodong Jian
* Date:      5/9/96
=====
SUBROUTINE CHDEP(ROUGH, DISCH, SLOPE, WIDTH, ML, MR, HMAX, H,
&                CONST, TOL, ITMX, IERR)
&
    IMPLICIT NONE
    REAL    ROUGH, DISCH, SLOPE, WIDTH, ML, MR, HMAX, H, TOL,
&          CONST
    INTEGER IERR, ITMX
*
    REAL    H1, H2, AREA, RAD, C, C1, EPS
    INTEGER I
    DATA   EPS /0.000001/
*
    IERR = 0
    IF (DISCH .LT. EPS) THEN
        H = 0.0
        GOTO 999
    ENDIF
*-----Compute quotient and initialize iteration values
*
    C = ROUGH * DISCH / SQRT(SLOPE) / CONST
    H1 = HMAX
    H = H1
    H2 = 0.0
*-----Start Iterations
*
    IERR = 0
    DO 100 I = 1, ITMX
        compue area and hydraulic radius
        AREA = H * (WIDTH + 0.5 * H * (ML + MR))
        RAD = AREA / (WIDTH + H * (SQRT(1.0+ML*ML) + SQRT(1.0+MR*MR)))
        Compute approximate quotient
        C1 = AREA * RAD**(2.0/3.0)
        Check convergence
        IF (ABS((C1-C)/C) .LE. TOL) THEN
            GOTO 999
        ENDIF
    100 CONTINUE

```

if No.

```

      IF (C1 .GT. C) THEN
      H = H2 + (H - H2) / 2.0
      ELSE
      IF (H+EPS .GE. H1) THEN
      IERR = 1
      H1 = H1 * 2
      ELSE
      H2 = H
      H = H2 + (H1 - H2) / 2.0
      ENDIF
      ENDIF

```

```

100 CONTINUE
      IERR = 2
999 RETURN
      END

```

```

=====
* Name:      fil_head
* Purpose:   Open a time series file and read headers for nodal data.
* Author:    Xiaodong Jian
=====

```

```

      SUBROUTINE FIL_HEAD(NNODS, NDNAM, LDND, NDTND, DTND, LDDTND,
&      DTYPE, UNITNM, LDUNIT, DTFLAG, DTFIL, FILNAM, NTLN,
&      IU, OU, CLASS, CTERM, COLSTR, LDCOL)
      IMPLICIT NONE
      INTEGER NNODS, LDND, NDTND, LDDTND, DTND(LDDTND, 3), DTYPE,
&      LDUNIT
      CHARACTER*(*) NDNAM(LDND), UNITNM(0:LDUNIT, 1), FILNAM, CLASS
      INTEGER NTLN, IU, OU
      LOGICAL DTFLAG, DTFIL

      INTEGER LDCOL
      CHARACTER CTERM*(*), COLSTR(LDCOL)*(*), MESS*50

      INTEGER DTUNIT
      INTEGER I, J, K, L, N, ND, NREC, NNDS, SL
      LOGICAL ERR, CN, CNINT, DUMMY, ISNUM
      INTEGER SAVOPT
      COMMON /SAVOPT/ SAVOPT

```

```

      DTYPE = 1
      DUMMY = .FALSE.
      MESS = 'ENTER DATA FILE FOR '//CLASS
      DTFIL = .FALSE.
      IF (FILNAM .EQ. ' ') THEN
      GOTO 999
      ENDIF
      CALL IO_OPFIL(IU, 1, FILNAM, MESS)

```

-----Skip title lines

```

      DO I = 1, NTLN
      READ (IU, *, END = 999)
      ENDDO

```

-----Read data unit code and type

```

      READ (IU, '(A)', END = 999) CTERM
      CALL STR_DIVD(CTERM, I, COLSTR, LDCOL, 0, ' ', ' ')
      READ (COLSTR(1), '(I3)') DTUNIT
      IF (ISNUM(COLSTR(2))) THEN
      READ (COLSTR(2), '(I3)') DTYPE
      ENDIF

```

-----Read nodal names

```

      NNDS = 0
      READ (IU, '(A)', END = 999) CTERM
      CALL STR_DIVD(CTERM, NNDS, COLSTR, LDCOL, 0, ' ', ' ')
      N = 0
      DO 30 I = 2, NNDS
      IF (CN(COLSTR(I), 'DEFAULT', 1)) THEN

```

-----Seasonal data are no longer valid.

```

      DUMMY = .TRUE.
      IF (NNDS .EQ. 2) THEN
      DO J = 1, NNDS
      DTND(J, 1) = J
      DTND(J, 2) = DTUNIT
      DTND(J, 3) = -J
      ENDDO
      ELSE

```

```

      IF (NDTND .GT. 0) THEN
      DO 25 J = 1, NNDS - 2
      DO L = 1, NDTND + N
      IF (DTND(L, 3) .EQ. J) THEN
      DO K = 1, 3
      DTND(J, K) = DTND(L, K)
      ENDDO

```

```

                                GOTO 25
                                ENDIF
                                ENDDO
25      CONTINUE
                                DO J = NNDS-2+1, NNODS
                                    DO K = 1, 3
                                        DTND(J,K) = 0
                                    ENDDO
                                ENDDO
                                ENDIF
*-----
                                L = NNDS - 2
                                DO J = 1, NNODS
                                    IF (.NOT. CNINT(J, NNDS-2, DTND(1,1), K)) THEN
                                        L = L + 1
                                        DTND(L, 1) = J
                                        DTND(L, 2) = DTUNIT
                                        DTND(L, 3) = -L
                                    ENDF
                                ENDDO
                                ENDF
                                NDTND = NNODS
                                GOTO 50
                                ELSE
                                    CALL NAMNUM(LDND, NDNAM, COLSTR(I), ND, 0, ERR)
                                    IF (ERR) THEN
                                        WRITE(*, 901) COLSTR(I), FILNAM
901      FORMAT( '***ERROR***NODAL NAME: ', A12,
&              ' IN THE FILE: ', A,
&              ', ' NOT FOUND IN THE NETWORK CONFIGURATION.')
                                        CALL EXIT
                                    ENDF
                                    IF (NDTND .GT. 0) THEN
                                        DO J = 1, NDTND
                                            IF (DTND(J, 1) .EQ. ND) THEN
                                                GOTO 20
                                            ENDF
                                        ENDDO
                                        N = N + 1
                                        J = NDTND + N
20      CONTINUE
                                ELSE
                                    J = I - 1
                                ENDF
                                DTND(J, 1) = ND
                                DTND(J, 2) = DTUNIT
                                DTND(J, 3) = I - 1
                                ENDF
30      CONTINUE
                                NNDS = NNDS - 1
                                IF (NDTND .GT. 0) THEN
                                    NDTND = NDTND + N
                                ELSE
                                    NDTND = NNDS
                                ENDF
*
50      CONTINUE
                                DTFLAG = .TRUE.
                                DTFIL = .TRUE.
*
*-----Find the number of data records in the data file
*
                                CALL NORECS(IU, NREC)
                                REWIND(IU)
                                DO I = 1, NTL+2
                                    READ(IU, *)
                                ENDDO
*
*-----Save the data file information.
*
                                IF (SAVOPT .EQ. 0 .OR. SAVOPT .EQ. 1) THEN
                                    CALL STR_LEN(CLASS, J)
                                    WRITE (OU, 800) CLASS, ('=', I = 1, J + 11)
800      FORMAT (
&          //, 'Summary for ', A, ' data file:'
&          //, '=====', 100A)
                                    IF (DUMMY) THEN
                                        WRITE (OU, 805) FILNAM, UNITNM(DTUNIT, DTTYE), NDTND, NREC
                                        WRITE (OU, 806) (NDNAM(DTND(I, 1)), I=1, NNDS-2), 'DEFAULT'
                                    ELSE
                                        WRITE (OU, 805) FILNAM, UNITNM(DTUNIT, DTTYE), NNDS, NREC
                                        CTERM = ' '
                                        J = 0
                                        L = 1
                                        DO I = 1, NNDS
                                            IF (CNINT(I, NDTND, DTND(1, 3), K)) THEN
                                                J = J + 1
                                                WRITE(CTERM((J-1)*10+1:J*10), '(A10)') NDNAM(DTND(K,1))
                                            ENDF
                                            IF (J .EQ. 5) THEN
                                                IF (L .EQ. 1) THEN
                                                    WRITE(OU, 807) CTERM
                                                ELSE
                                                    WRITE(OU, 808) CTERM

```

```

                ENDIF
                CTERM = ' '
                J = 0
                L = L + 1
            ENDIF
        ENDDO
        IF (SL(CTERM) .GT. 0 ) THEN
            IF (L .EQ. 1) THEN
                WRITE (OU, 807) CTERM
            ELSE
                WRITE (OU, 808) CTERM
            ENDIF
            CTERM = ' '
        ENDIF
    ENDIF
ENDIF

*
999 RETURN
805 FORMAT (
& //, 10X, '      File name: ', A
& //, 10X, '      Data unit: ', A
& //, 10X, '      Number of nodes: ', I4,
& //, 10X, '      Number of records: ', I4)
806 FORMAT( 10X, ' List of nodal name: ', 5A10, 10(/, 30X, 5A10))
807 FORMAT( 10X, ' List of nodal names: ', A50)
808 FORMAT( 31X, A50)
END

*-----
SUBROUTINE NORECS(IU, NREC)
IMPLICIT NONE
INTEGER IU, NREC, I

*
NREC = 0
5 READ (IU, *, END = 10)
NREC = NREC + 1
GOTO 5
10 DO I = 1, NREC + 1
                                BACKSPACE(IU)
ENDDO
RETURN
END

*-----
* Name:      Isnum
* Purpose: Check whether a string is a float or integer number.
*-----
LOGICAL FUNCTION ISNUM(STR)
IMPLICIT NONE
CHARACTER STR*(*)

*
INTEGER I, L, ICD
LOGICAL DECFLG, SIGNFG
ISNUM = .FALSE.
DECFLG = .FALSE.
SIGNFG = .FALSE.
IF (STR .EQ. ' ') GOTO 999
L = LEN(STR)

*
DO I = 1, L
                IF (STR(I:I) .NE. ' ') THEN
                    ICD = ICHAR(STR(I:I))
                    IF (ICD .GE. 48 .AND. ICD .LE. 57) THEN ! DIGITS 0 - 9.
                        CONTINUE
                    ELSE IF (ICD .EQ. 46 .AND. .NOT. DECFLG) THEN ! DECIMAL POINT.
                        DECFLG = .TRUE.
                    ELSE IF ((ICD .EQ. 43 .OR. ICD .EQ. 45)
& .AND. .NOT. SIGNFG) THEN ! + OR -
                        SIGNFG = .TRUE.
                    ELSE
                        GOTO 999
                    ENDIF
                ENDIF
            ENDDO
            ISNUM = .TRUE.
999 RETURN
END

*-----
LOGICAL FUNCTION INLIST(ND, NLST, LSTND, LOC)
IMPLICIT NONE
INTEGER ND, NLST, LSTND(NLST), LOC
INTEGER I
INLIST = .FALSE.
DO I = 1, NLST
                IF (ND .EQ. LSTND(I)) THEN
                    LOC = I
                    INLIST = .TRUE.
                    GOTO 999
                ENDIF
            ENDDO
999 RETURN
END

*-----
* Name:      gw_dat
* Purpose: Read groundwater altitude or flux data.
* Author: Xiaodong Jian

```



```

*-----Calculate seepage from a pond.
*
      ZVA(2) = INST(RES) ! INITIAL POND STORAGE
      IF (GWFLG .AND. GWTYPE .EQ. 2) THEN
*-----If required ground-water seepage is greater than
* the available water in a pond. It is assumed that
* the maximum ground-water seepage is 1/3 of the available
* water.
*
          RTERM = HGW * PERD
          NV = NINT(HGW * PERD * XF)
          IF (HGW .GT. 0.0 .AND. RTERM .GT. ZVA(2)) THEN
              NV = NINT(ZVA(2) / 3.0 * XF)
          ELSE
              NV = NINT(HGW * PERD * XF)
          ENDIF
          ELSE
*-----Find pond water-surface elevation and area
*
          IF (GETZVA(ND, 2, ZVA, 26)) THEN
              HSW = ZVA(1)
              AREA = ZVA(3)
          ENDIF
          RTERM = KY * (HSW - HGW) / DL * AREA * PERD
          NV = NINT(KY * (HSW - HGW) / DL * AREA * PERD * XF)
*-----If calculated ground-water seepage is greater than
* the available water in a pond. It is assumed that
* the maximum ground-water seepage is 1/3 of the available
* water.
*
          IF (RTERM .GT. ZVA(2)) THEN
              NV = NINT(ZVA(2) / 3.0 * XF)
          ELSE
              NV = NINT(RTERM * XF)
          ENDIF
          ENDIF
*-----Create the ground-water arc
*
          IF (NV .GT. 0) THEN ! LOSS WATER TO AQUIFER
              CALL ARCVAL(NARC, II, JJ, LO, HI, COST, ARTYP,
& LDARC, ND, SKSC, NV, NV, 0, 4)
              NDXAR(ND, 4) = NARC
          ELSE IF (NV .LT. 0) THEN ! GAIN WATER FROM AQUIFER.
& CALL ARCVAL(NARC, II, JJ, LO, HI, COST, ARTYP,
& LDARC, SKSC, ND, -NV, -NV, 0, -4)
              NDXAR(ND, 4) = -NARC
          ENDIF
100 CONTINUE
RETURN
END
*=====
* Name:      KLTR
* Purpose:   Out-of-kilter algorithm to find the optimal flow in a net-
*            work.
*=====
      SUBROUTINE KLTR (I, J, HI, LO, COST, FLOW, NNODS, NARCS,
& DEBUG, KARC, IFAULT)
      IMPLICIT NONE
      INTEGER NARCS, NNODS, I(NARCS), J(NARCS), KARC, IFAULT
      INTEGER HI(NARCS), LO(NARCS), COST(NARCS), FLOW(NARCS)
      LOGICAL DEBUG
*
      INTEGER MXNODS
      PARAMETER (MXNODS = 500)
      INTEGER PI(MXNODS), NA(MXNODS), NB(MXNODS)
      INTEGER A, AOK, COK, C, DEL, E, EPS, SRC, SNK
*
      INTEGER M, K, INF, IA, JA, N, LAB, CCOK, NI, NJ
*
      KARC = 0
      IFAULT = 0
      DEBUG = .FALSE.
*-----Check nodal numbers which cannot be negative, zero, or
* greater than MXNODS.
*
      DO K = 1, NARCS
          IF (I(K) .LE. 0 .OR. J(K) .LE. 0) THEN
              PRINT *, CHAR(7)
              WRITE(*, 805) I(K), J(K)
805      FORMAT('**ERROR** NODAL NUMBER CANNOT BE NEGATIVE OR LESS',
& 'THAN ZERO.')
&      WRITE(*, 806) MXNODS, I(K), J(K)
&      STOP !CALL EXIT
          ENDIF
          IF (I(K) .GT. MXNODS .OR. J(K) .GT. MXNODS) THEN
              PRINT *, CHAR(7)
              WRITE(*, 806) MXNODS, I(K), J(K)
806      FORMAT('**ERROR** NODAL NUMBER CANNOT BE GREATER THAN', I4,

```

```

&      /, 'NODAL NUMBERS ARE ', I4, ', ', I4)
      STOP !CALL EXIT
      ENDIF
ENDDO
DO 1 A = 1, NARCS
      FLOW (A) = 0
1  CONTINUE
DO 2 M = 0, NNODS
      PI (M) = 0
2  CONTINUE
DO 3 K = 0, 100
      NA (K) = 0
3  CONTINUE
DO 4 A = 1, NARCS
      IF (LO (A) .GT. HI (A)) THEN
          IFAULT = 1
          PRINT '(a, i5)', '***** ERROR *** Lower bound is higher than'
&          // 'the upper bound for arc: ', A
          PRINT '(a, i10)', 'The lower bound is ', LO(A)
          PRINT '(a, i10)', 'The upper bound is ', HI(A)
          END IF
4  CONTINUE
      IF (IFAULT .NE. 0) THEN
          RETURN
      ENDIF
      INF = 999999
      AOK = 0
10 CONTINUE
DO 20 A = 1, NARCS
      IA = I(A)
      JA = J(A)
      C = COST(A) + PI(IA) - PI(JA)
      IF ((FLOW(A) .LT. LO(A)) .OR. (C .LT. 0 .AND. FLOW(A) .LT.
& HI(A))) THEN
          SRC = J(A)
          SNK = I(A)
          E = +1
          GO TO 30
      ELSE IF ((FLOW(A) .GT. HI(A)) .OR. (C .GT. 0 .AND. FLOW(A) .GT.
& LO(A))) THEN
          SRC = I(A)
          SNK = J(A)
          E = -1
          GO TO 30
          END IF
20 CONTINUE
      GO TO 100
30 IF ((A .EQ. AOK) .AND. (NA(SRC) .NE. 0)) GO TO 25
      AOK = A
      DO 5 N = 1, NNODS
          NA(N) = 0
          NB(N) = 0
5  CONTINUE
      NA(SRC) = IABS(SNK) * E
      NB(SRC) = IABS(AOK) * E
25 COK = C
40 LAB = 0
      DO 35 A = 1, NARCS
          IA = I(A)
          JA = J(A)
*      !DETERMINE THE CANDIDATE ARC FOR THE TREE
          IF ((NA(IA) .EQ. 0 .AND. NA(JA) .EQ. 0) .OR. (NA(IA) .NE. 0
& .AND. NA(JA) .NE. 0)) GO TO 35
          C = COST(A) + PI(IA) - PI(JA)
          IF (NA(IA) .NE. 0) THEN
              IF (FLOW(A) .GE. HI(A) .OR.
& (FLOW(A) .GE. LO(A) .AND. C .GT. 0)) GO TO 35
              NA(JA) = I(A)
              NB(JA) = A
              ELSE IF (NA(IA) .EQ. 0) THEN
                  IF (FLOW(A) .LE. LO(A) .OR.
& (FLOW(A) .LE. HI(A) .AND. C .LT. 0)) GO TO 35
                  IA = I(A)
                  NA(IA) = -J(A)
                  NB(IA) = -A
                  END IF
                  LAB = 1
          IF (NA(SNK) .NE. 0) GO TO 50
35 CONTINUE
          IF (LAB .NE. 0) GO TO 40
          DEL = INF
          DO 45 A = 1, NARCS
              IA = I(A)
              JA = J(A)
&          IF ((NA(IA) .EQ. 0 .AND. NA(JA) .EQ. 0) .OR. (NA(IA) .NE. 0
              .AND. NA(JA) .NE. 0)) GO TO 45
              C = COST(A) + PI(IA) - PI(JA)
*          IF (NA(JA) .EQ. 0 .AND. FLOW(A) .LT. HI(A)) DEL = MIN0(DEL, C)
*          IF (NA(JA) .NE. 0 .AND. FLOW(A) .GT. LO(A)) DEL = MIN0(DEL, -C)
45 CONTINUE
          CCOK = COK
          IF (DEL .EQ. INF .AND. (FLOW(AOK) .EQ. HI(AOK) .OR. FLOW(AOK) .EQ.

```

```

&      LO(AOK)) DEL = ABS(CCOK)
IF (DEL .EQ. INF) THEN
      DEBUG = .TRUE.
      KARC = AOK * E
      GO TO 99
END IF
DO 6 N = 1, NNODS
      IF (NA(N) .EQ. 0) PI(N) = PI(N) + DEL
6    CONTINUE
GO TO 10
50   EPS = INF
NI = SRC
NJ = IABS(NA(NI))
A = IABS(NB(NI))
C = COST(A) + PI(NI) - PI(NJ)
IF (NB(NI) .GT. 0) THEN
      IF (FLOW(A) .LT. LO(A)) THEN
            EPS = MIN0(EPS, LO(A) - FLOW(A))
            END IF
      IF (C .LE. 0 .AND. FLOW(A) .LT. HI(A)) THEN
            EPS = MIN0(EPS, HI(A) - FLOW(A))
            END IF
ELSE IF (NB(NI) .LT. 0) THEN
      IF (FLOW(A) .GT. HI(A)) THEN
            EPS = MIN0(EPS, FLOW(A) - HI(A))
            END IF
      IF (C .GE. 0 .AND. FLOW(A) .GT. LO(A)) THEN
            EPS = MIN0(EPS, FLOW(A) - LO(A))
            END IF
END IF
NI = NJ
IF (NI .NE. SRC) GO TO 60
NJ = IABS(NA(NI))
A = IABS(NB(NI))
FLOW(A) = FLOW(A) + ISIGN(EPS, NB(NI))
NI = NJ
IF (NI .NE. SRC) GO TO 70
AOK = 0
GO TO 10
99   CONTINUE
100  IF (.NOT. DEBUG) GO TO 90
90   RETURN
800  FORMAT ('SOLUTION INFEASIBLE')
END
=====
* Name:      PRNINF
* Purpose:   Print the basic network information.
* Author:    Xiaodong Jian
=====
SUBROUTINE PRNINF (NDNAM, NDTYP, NDSEQ, NRES, TYPE, MXND,
&      MXARC, MXR,
&      II, JJ, HI, LO, COST, ARTYP, PTRE, REAR,
&      NNODS, PTDWAR, NDDWAR,
&      PERD, NARCS, C, XF, IOUT1)
IMPLICIT NONE
INTEGER MXARC, MXR, MXND
INTEGER II(MXARC), JJ(MXARC), HI(MXARC), LO(MXARC), COST(MXARC),
&      ARTYP(MXARC), PTRE(MXR), REAR(MXR),
&      PTDWAR(MXND, 2), NDDWAR(MXARC)
INTEGER NNODS, NARCS, IOUT1
REAL C, XF, PERD, X
CHARACTER NDNAM(MXND)*(*)
INTEGER NDTYP(MXND), NDSEQ(MXND)
INTEGER NRES, TYPE, N, I, J, RES, LIM1, LIM2, L
INTEGER ND, OND
INTEGER ARC, ZONE
CHARACTER TYP*4
CHARACTER*10 NCST, UCST, LCST, ONCST
*
X = PERD * C * XF
IF (TYPE .EQ. 0 .OR. TYPE .EQ. 1) THEN
*
*      WRITE THE NODE NAME AND TYPE
*
      WRITE (IOUT1, 21)
21   FORMAT (
&      //, 'Node name and type of basic network',
&      //, '=====',
&      //, '10X, ' Node Node Node',
&      //, '10X, ' number name type',
&      //, '10X, ' -----',
&      //, '10X, ' -----')
      DO 2 N = 1, NNODS
            IF (NDTYP(N) .EQ. 1) THEN
23          WRITE (IOUT1, 23) N, NDNAM(N), 'POND'
            ELSE
23          WRITE (IOUT1, 23) N, NDNAM(N), 'GENERAL'
            END IF
      CONTINUE
END IF
*
IF (TYPE .EQ. 0 .OR. TYPE .EQ. 2) THEN
*
*      WRITE THE ARCS AND BOUNDS OF A NETWORK

```

```

*
27      WRITE (IOUT1, 27)
&      FORMAT (
&      //, 'Basic network',
&      //, '=====',
&      //, T45, '      Flow boundary ',
&      //, T45, '-----',
&      //, T45, '      Lower      Upper',
&      //, T45, '      (cubic    (cubic',
&      //, 10X, '      From-    To-',
&      //, T45, '      feet per feet per Penalty',
&      //, 10X, 'Arc node node',
&      //, T45, '      second) second) coefficient Type',
&      //, 10X, '-----',
&      //, T45, '-----')
&      DO 3 J = 1, NARCS
&      WRITE (IOUT1, 29) J, NDNAM(II(J)), NDNAM(JJ(J)),
&      LO(J)/X, HI(J)/X, COST(J), ARTYP(J)
29      FORMAT (10X, I3, 2X, T16, A, T28, A, T45, 2F10.2, I15, I10)
3      CONTINUE
&      END IF
*
&      IF (TYPE .EQ. 0 .OR. TYPE .EQ. 3) THEN
*
*      WRITE THE PENALTY COEFFICIENTS OF RESERVOIR ZONES
*
&      WRITE (IOUT1, 31)
31      FORMAT (
&      //, 'Pond-zoning penalty coefficients',
&      //, '=====',
*
*      Find the maximum number of zones of a pond
*
&      J = 0
&      DO 36 RES = 1, NRES
&      L = PTRE(RES+1) - PTRE(RES)
&      IF (L .GT. J) J = L
36      CONTINUE
&      WRITE (IOUT1, 940) 'Name', ('Zone ', I, I = 1, J)
940     FORMAT(10X, A, T31, 10(4X, A4, I2))
&      WRITE (IOUT1, 942) ('-----', I = 1, J+1)
942     FORMAT(10X, A4, T31, 10(4X, A6))
&      DO 39 RES = 1, NRES
&      DO 4 N = 1, NNODS
&      IF (NDTYP(N) .EQ. 1 .AND. NDSEQ(N) .EQ. RES) GO TO 33
4      CONTINUE
33      CONTINUE
&      LIM1 = PTRE(RES)
&      LIM2 = PTRE(RES+1) - 1
&      WRITE (IOUT1, 945) NDNAM(N),
&      (COST(IABS( REAR(L) )), L= LIM1, LIM2)
945     FORMAT (10X, A, T31, 4I10/)
39      CONTINUE
*
*      WRITE OUT THE PENALTY COEFFICIENTS OF CHANNELS
*
&      WRITE (IOUT1, 950)
950     FORMAT(
&      //, 'Flow-zoning penalty coefficients',
&      //, '=====',
&      //, 10X, T35, '      Normal      Lower      Upper',
&      //, 10X, 'From-', T23, 'To',
&      //, T35, '      flow      flow      flow',
&      //, 10X, 'node', T23, 'node',
&      //, T35, '      zone      zone      zone',
&      //, 10X, '-----', T23, '-----',
&      //, T35, '-----')
&      DO 49 N = 1, NNODS
&      LIM1 = PTDWAR(N, 1)
&      LIM2 = PTDWAR(N, 2)
&      OND = 0
&      ONCST = ' '
&      NCST = ' '
&      LCST = ' '
&      UCST = ' '
&      DO I = LIM1, LIM2
&      ARC = NDDWAR(I)
&      WRITE(TYP, '(I4)') ARTYP(ABS(ARC))
*
*-----downstream node
*
&      IF (TYP(3:3) .EQ. '1') THEN
&      IF (ARC .GT. 0) THEN
&      ND = JJ(ARC+1)
&      ELSE
&      ND = II(-ARC+1)
&      ENDIF
&      ELSE
&      IF (ARC .GT. 0) THEN
&      ND = JJ(ARC)
&      ELSE
&      ND = II(-ARC)
&      ENDIF
&      ENDIF

```



```

*
      IF (FLWTL) THEN
          WRITE(IOUT, 950) NOP
950      FORMAT(
&          /,20X, 'CHANNEL FLOW FOR TIME STEP: ', I3,
&          /,20X, '=====',
&          /,
&          T50, 'FLOW', T65, 'TOTAL',
&          /,10X, 'FROM-NODE', T28, 'TO-NODE',
&          T50, '(A-FT)', T65, 'FLOW',
&          /,10X, '-----', T28, '-----',
&          T50, '-----', T65, '-----')
          FLWTL = .FALSE.
      ENDIF
      IF (ARC .GT. 0) THEN
          K = 1
          FN = II(ARC)
          TN = JJ(ARC)
      ELSE
          K = -1
          ARC = -ARC
          FN = JJ(ARC)
          TN = II(ARC)
      ENDIF
      IF (TN .NE. OJ) THEN
          OJ = TN
          FLW1 = K*FLOW(ARC)
      ELSE
          FLW1 = FLW1 + K*FLOW(ARC)
      ENDIF
      IF (.NOT. LAST) THEN
          ARCL = ARC + 1
          IF ( (FN .EQ. II(ARCL) .AND. TN .EQ. JJ(ARCL)) .OR.
&          (FN .EQ. JJ(ARCL) .AND. TN .EQ. II(ARCL)) ) GOTO 99
      ENDIF
      WRITE(IOUT, 900) NDNAM(FN), NDNAM(TN), FLW1, INT(FLW)
900      FORMAT(10X, A12, 5X, A12, 5X, I10, 5X, I10)
99      RETURN
      END
=====
* Name:      OUTPUT
* Purpose:    Print the arc flow status.
=====
      SUBROUTINE OUTPUT (M, N, HI, LO, COST, FLOW, NARCS)
      IMPLICIT NONE
      INTEGER NARCS, M(NARCS), N(NARCS), COST(NARCS), HI(NARCS),
&          LO(NARCS), FLOW(NARCS)
*
      INTEGER I
      WRITE (*, '(8X, A/)') 'NETWORK STATUS AND MINIMUM COST FLOW'
      WRITE (*, 905)
905      FORMAT (3X, 'ARC', 5X, 'I', 5X, 'J', 6X, 'COST', 5X, 'L.BND', 5X,
&          'U.BND', 6X, 'FLOW', /)
      DO 7 I = 1, NARCS
          WRITE(*, 910) I, M(I), N(I), COST(I), LO(I), HI(I), FLOW(I)
7      CONTINUE
910      FORMAT (1X, I5, 2I6, 4I10)
      RETURN
      END
=====
      SUBROUTINE OUTPUT2 (NDNAM, LDND, M, N, HI, LO, COST, FLOW,
&          ARTYP, NARCS)
      IMPLICIT NONE
      INTEGER LDND, NARCS, M(NARCS), N(NARCS), COST(NARCS),
&          HI(NARCS), LO(NARCS), ARTYP(NARCS), FLOW(NARCS)
      CHARACTER NDNAM(LDND)*(*)
*
      INTEGER I
      WRITE (*, '(8X, A/)') 'NETWORK STATUS AND MINIMUM COST FLOW'
      WRITE (*, 905)
905      FORMAT (3X, 'ARC', 1X, 'TYPE', 1X, 'FROM', 8X, 'TO', 16X, 'COST',
&          5X, 'L.BND', 5X, 'U.BND', 6X, 'FLOW', /)
      DO 7 I = 1, NARCS
          WRITE(*, 910) I, ARTYP(I), NDNAM(IABS(M(I))), NDNAM(IABS(N(I))),
&          COST(I), LO(I), HI(I), FLOW(I)
7      CONTINUE
910      FORMAT(1X, 2I5, 1X, 2A12, 4I10)
      RETURN
      END
=====
* Name:      FLWCK
* Purpose:    Check flow convergence.
=====
      SUBROUTINE FLWCK(NNODS, MXND,
&          PTDWAR, NDDWAR, MXARC,
&          FLOW, OFLOW, FCRIT, NOTCOV)
      IMPLICIT NONE
      INTEGER NNODS, MXND, MXARC
      INTEGER PTDWAR(MXND, 2), NDDWAR(MXARC)
      INTEGER FLOW(MXARC)
      INTEGER OFLOW(MXARC), FCRIT
      LOGICAL NOTCOV
*
      INTEGER I, N, LIM1, LIM2, ARC

```

```

*
*      Check convergence
*
NOTCOV = .FALSE.
DO 50 N = 1, NNODS
      LIM1 = PTDWAR(N, 1)
      LIM2 = PTDWAR(N, 2)
      DO I = LIM1, LIM2
      ARC = IABS (NDDWAR(I))
      IF (ABS(FLOW(ARC) - OFLOW(ARC)) .LT. FCRIT) THEN
        CONTINUE
      ELSE
        NOTCOV = .TRUE.
      ENDIF
      OFLOW(ARC) = FLOW(ARC)
      ENDDO
50  CONTINUE
   RETURN
   END
=====
* Name:      CTFLWB
* Purpose:   Adjust the lower flow bound for control arcs.
* Author:    Xiaodong Jian
* Date:      11/27/95
=====
SUBROUTINE CTFLWB(ARC, CTAR, NCTAR, CTARFW, LDCTAR, LO, LDARC,
& DLO, CTRFLW)
  IMPLICIT NONE
  INTEGER LDCTAR, LDARC, ARC, CTAR
  INTEGER NCTAR, CTARFW(LDCTAR, 3), LO(LDARC), DLO, DFB
  LOGICAL CTRFLW
*
  INTEGER I, A
*
  CTRFLW = .FALSE.
  DO I = 1, NCTAR
      A = CTARFW(I, 1)
      DFB = CTARFW(I, 3) - CTARFW(I, 2)
      IF ( (DFB .LT. DLO .AND. ARC .EQ. 0) .OR.
& (DFB .LT. DLO .AND. A .NE. CTAR) ) THEN
        CONTINUE
      ELSE
        CTAR = A
        CTRFLW = .TRUE.
        A = CTARFW(I, 1)
        IF (ARC .EQ. 0) THEN
          CTARFW(I, 2) = LO(A)
          LO(A) = INT(0.5 * (CTARFW(I, 2) + CTARFW(I, 3)))
        ELSE
          CTARFW(I, 3) = LO(A)
          LO(A) = INT(0.5 * (CTARFW(I, 2) + CTARFW(I, 3)))
        ENDIF
        GOTO 99
      ENDIF
  ENDDO
99  CONTINUE
   RETURN
   END
=====
* Name:      locflw
* Purpose:   Open a local inflow file.
=====
SUBROUTINE LOCFLW(NND, NDNAM, LDND, NIFW, IFWND, LDIFW, IFWCD,
& FILNAM, IU, OU, CTERM, COLSTR, LDCOL)
  IMPLICIT NONE
  INTEGER NND, LDND, NIFW, LDIFW, IFWND(LDIFW), IFWCD, IU, OU
  CHARACTER NDNAM(LDND)*(*), FILNAM*(*)
*
  INTEGER LDCOL
  CHARACTER CTERM*(*), COLSTR(LDCOL)*(*)
  INTEGER I, ND, NREC
  LOGICAL ERR
  CHARACTER UNITNM(0:2)*7
  DATA UNITNM/'ACRE-FT', 'FT^3/S', 'FT^3/D'/
*
  CALL IO_OPFIL(IU, 1, FILNAM, 'ENTER LOCAL NET INFLOW FILE: ')
*
*      Skip title lines
*
  NIFW = 0
  DO I = 1, 2
      READ (IU, *, END = 999)
  ENDDO
*
*      Read flow unit code
*
  READ (IU, *) IFWCD
*
*      Read the nodal name with local net inflow
*
  READ (IU, '(A)', END = 999) CTERM
  CALL STR_DIVD(CTERM, NIFW, COLSTR, LDCOL, 0, ' ')
  DO I = 2, NIFW
      CALL NAMNUM(NND, NDNAM, COLSTR(I), ND, 0, ERR)
  END

```

```

                                IF (ERR) THEN
                                WRITE(*, 901) COLSTR(I), FILNAM
901      FORMAT( '***ERROR***NODAL NAME: ', A12, ' IN THE FILE: ', A,
&              /, ' NOT FOUND IN THE NETWORK CONFIGURATION.')
                                STOP !CALL EXIT
                                ELSE
                                IFWND(I-1) = ND
                                ENDIF
                                ENDDO
                                NIFW = NIFW - 1
*
                                CALL NORECS(IU, NREC)
                                WRITE (OU, 800) FILNAM, UNITNM(IFWCD), NIFW,
&              NREC, (NDNAM(IFWND(I)), I = 1, NIFW)
800      FORMAT (
&              //, '=====',
&              //, 'LOCAL INCREMENTAL INFLOWS:',
&              //, '=====',
&              //, 20X, '      FILE NAME: ', A,
&              //, 20X, '      FLOW UNITS: ', A,
&              //, 20X, 'NUMBER OF NODES: ', I4,
&              //, 20X, 'NUMBER OF RECORD: ', I4,
&              //, 20X, 'LIST OF NODES: ', 4A10, 10(/, 36X, 4A10))
999      RETURN
      END
=====
* Name:      opmdf
* Purpose:   Open master data file.
=====
      SUBROUTINE OPMDF(FILNAM, LDFIL, CTERM, COLSTR, LDCOL)
      IMPLICIT NONE
      INTEGER LDFIL, LDCOL
      CHARACTER FILNAM(0:LDFIL)*(*), CTERM*(*), COLSTR(LDCOL)*(*)
*
      INTEGER CD, L, N, IU
      CHARACTER FILNM*30
      LOGICAL CN, EXIST
*
      IU = 9
      FILNM = ' '
      DO N = 1, LDFIL
                                FILNAM(N) = ' '
      ENDDO
*
*-----Open master data file
*
      IF (EXIST('OPONDS.CTR')) THEN
                                OPEN(10, FILE = 'OPONDS.CTR', STATUS = 'OLD')
                                READ (10, '(A)') FILNM
                                CLOSE(10, STATUS = 'delete')
      ENDIF
      CALL IO_RDSTR('Enter master file:', FILNM)
      CALL IO_OPFIL(IU, 1, FILNM, 'ENTER MASTER FILE: ')
      OPEN(11, FILE = 'OPONDS.CTR', STATUS = 'UNKNOWN')
      WRITE(11, '(A)') FILNM
      CLOSE(11)
*
      L = 0
5      CONTINUE
      READ (IU, '(A)', END = 99) CTERM
      L = L + 1
      IF (CTERM.EQ. ' ') GOTO 5
      CALL STR_DIVD(CTERM, N, COLSTR, LDCOL, 0, ' ', ' ')
      READ (COLSTR(1), '(I2)', ERR = 5) CD
      IF (CD.LT. 0 .OR. CD.GT. LDFIL) THEN
                                PRINT *, CHAR(7)
                                WRITE(*, 801) CD, FILNM
801      FORMAT('***ERROR***INVALID FILE CODE: ', I2,
&              ' IN THE FILE: ', A)
                                STOP !CALL EXIT
      ENDIF
      IF (CN(COLSTR(2), ':', 1)) GOTO 5
      FILNAM(CD) = COLSTR(2)
      IF (.NOT. EXIST(FILNAM(CD)) .AND. CD.LE. 20) THEN
                                PRINT *, CHAR(7)
                                CALL STR_LEN(FILNAM(CD), N)
                                WRITE(*, 802) FILNAM(CD)(1:N), CD, L, FILNM
802      FORMAT('***ERROR***FILE: ', A, ' WITH FILE CODE ', I2,
&              /, ' DOES NOT EXIST. CHECK RECORDS IN THE LINE ', I2,
&              ' IN THE FILE: ', A)
                                STOP !CALL EXIT
      ENDIF
      GOTO 5
99      CLOSE (IU)
      RETURN
      END
=====
* Name:      arcval
* Purpose:   Create an arc and assign values.
=====
      SUBROUTINE ARCVAL(NARCS, II, JJ, LO, HI, COST, ARTYP,
&              MXARC, I, J, L, U, C, TYP)
      IMPLICIT NONE
      INTEGER NARCS, MXARC

```



```

      INTEGER      II(MXARC), JJ(MXARC), LO(MXARC), HI(MXARC),
&      COST(MXARC), ARTYP(MXARC)
      INTEGER      I, J, L, U, C, TYP
*
      NARCS = NARCS + 1
      II(NARCS) = I
      JJ(NARCS) = J
      LO(NARCS) = L
      HI(NARCS) = U
      COST(NARCS) = C
      ARTYP(NARCS) = TYP
*
      RETURN
      END
=====
* Name:      Month
* Purpose:    TRANSFORM THE MONTH WITH NUMBER OR CHAR.
=====
      SUBROUTINE MONTH (TY, NUM, CHA)
      IMPLICIT NONE
      CHARACTER * 4, MOth(12), CHA
      INTEGER      TY, NUM, I
      DATA MOth / 'JAN.', 'FEB.', 'MAR.', 'APR.', 'MAY', 'JUN.',
&      'JUL.', 'AUG.', 'SEP.', 'OCT.', 'NOV.', 'DEC.' /
      IF (TY .EQ. 1) THEN
         DO 1 I = 1, 12
            IF (CHA .EQ. MOth(I)) THEN
               NUM = I
               GO TO 10
            END IF
1          CONTINUE
         ELSE IF (TY .EQ. 2) THEN
            CHA = MOth(NUM)
         ELSE
            WRITE (*, *) ' ***MONTH TRANSFORM IS FAILED IN TYPE' , TY
10          CONTINUE
         RETURN
         END
=====
      SUBROUTINE PNCK(PN, IU, FLAG, CTERM, COLSTR, LDCOL)
      IMPLICIT NONE
      INTEGER      PN, LDCOL, IU
      LOGICAL      FLAG
      CHARACTER     CTERM*(*), COLSTR(LDCOL)*(*)
      INTEGER      N
      LOGICAL      CN
*
      FLAG = .TRUE.
      CTERM = ''
      DO WHILE (CTERM .EQ. '')
         READ (IU, '(A)', END = 999) CTERM
      ENDDO
      CALL STR_DIVD(CTERM, N, COLSTR, LDCOL, 0, ' ', ')
      READ (COLSTR(2), '(I2)') N
      IF (CN(COLSTR(1), 'PART', 1) .AND. N .EQ. PN) THEN
         FLAG = .FALSE.
      ELSE
         IF (CN(COLSTR(1), 'PART', 1) .AND. N .GT. PN) THEN
            BACKSPACE(IU)
            GOTO 999
         ENDIF
         PRINT *, CHAR(7)
         WRITE(*, 801) PN
801      &      FORMAT( ' ***ERROR***ERROR IN PART ', I1,
&      ' OF NETWORK DATA FILE')
         CALL STR_LEN(CTERM, N)
         PRINT *, 'RECORD = ', CTERM(1:N)
         STOP !CALL EXIT
      ENDF
999      RETURN
      END
=====
      LOGICAL FUNCTION CNINT(ISEED, NOIX, IX, LOC)
      IMPLICIT NONE
      INTEGER      ISEED, NOIX, IX(NOIX), LOC
*
      INTEGER      I
*
      CNINT = .FALSE.
      DO I = 1, NOIX
         IF (ISEED .EQ. IX(I)) THEN
            CNINT = .TRUE.
            LOC = I
            GOTO 99
         ENDIF
99      ENDDO
      RETURN
      END
=====
      LOGICAL FUNCTION CN2INT(ISD1, ISD2, NOIX, IX1, IX2, LOC)
      IMPLICIT NONE
      INTEGER      ISD1, ISD2, NOIX, IX1(NOIX), IX2(NOIX), LOC
*

```

```

      INTEGER    I
*
      CN2INT = .FALSE.
      I = 1
      DO WHILE (I .LE. NOIX .AND. .NOT. CN2INT)
         IF (ISD1 .EQ. IX1(I) .AND. ISD2 .EQ. IX2(I)) THEN
            CN2INT = .TRUE.
            LOC = I
            ENDIF
            I = I + 1
         ENDIF
      ENDDO
      RETURN
END
99
=====
* Name:          fdwnd
* Purpose:       Find DownStream Node.
=====
      SUBROUTINE FDWND(ARC, II, JJ, LDARC, NSTRM, STRMAR, LDSTRM, DWND)
      IMPLICIT    NONE
      INTEGER     ARC, LDARC, II(LDARC), JJ(LDARC)
      INTEGER     LDSTRM, NSTRM, STRMAR(LDSTRM,0:6)
      INTEGER     DWND
*
      INTEGER     I, ARC2
      LOGICAL     CNINT
*
      IF (CNINT(ARC, NSTRM, STRMAR(1,1), I)) THEN
         ARC2 = STRMAR(I, 6)
      ELSE
         ARC2 = ARC
      ENDIF
      IF (ARC2 .GT. 0) THEN
         DWND = JJ(ARC2)
      ELSE
         DWND = II(-ARC2)
      ENDIF
      RETURN
END
=====
* Name:          dattb
* Purpose:       Read seasonal data matrix.
* Author:        Xiaodong Jian
* Date:          02/29/95
=====
      SUBROUTINE DATTB(NNODS, NDNAM, LDND, NDTND, DTND, LDDTND, UNITCD,
&                     NREC, DTTB, LDDTTB, DTFLAG,
&                     UNITNM, LDUNIT, FILNAM, IU, OU, PN, ENDFIL,
&                     TITLE, CTERM, COLSTR, LDCOL)
      IMPLICIT    NONE
      INTEGER     NNODS, LDND, NDTND, LDDTND, DTND(LDDTND, 3), NREC,
&                     LDDTTB
      REAL        DTTB(0:LDDTTB, LDDTND)
      INTEGER     UNITCD, LDUNIT, IU, OU, PN
      CHARACTER   NDNAM(LDND)*(*), UNITNM(0:LDUNIT)*(*), FILNAM*(*)
      LOGICAL     DTFLAG, ENDFIL
      INTEGER     LDCOL
      CHARACTER   TITLE*(*), CTERM*(*), COLSTR(LDCOL)*(*)
*
      INTEGER     I, J, K, L, ND, SL, NNDS
      LOGICAL     ERR, CN, CNINT, DUMMY
      INTEGER     SAVOPT
      COMMON      /SAVOPT/ SAVOPT
*
      DUMMY = .FALSE.
      DTFLAG = .FALSE.
      ENDFIL = .TRUE.
      NDTND = 0
      NREC = 0
*
*-----Read data unit and nodal names
*
      READ (IU, *, END = 999) UNITCD
      READ (IU, '(A)', END = 999) CTERM
      CALL STR_DIVD(CTERM, NNDS, COLSTR, LDCOL, 0, ' ', ')
      DO I = 2, NNDS
         IF (CN(COLSTR(I), 'DEFAULT', 1)) THEN
            DUMMY = .TRUE.
            NDTND = NNODS
            IF (NNDS .EQ. 2) THEN
               DO J = 1, NNODS
                  DTND(J, 1) = J
                  DTND(J, 2) = UNITCD
                  DTND(J, 3) = 0
               ENDDO
            ELSE
               L = NNDS - 2
               DO J = 1, NNODS
                  IF (.NOT. CNINT(J, NNDS-2, DTND(1,1), K)) THEN
                     L = L + 1
                     DTND(L, 1) = J
                     DTND(L, 2) = UNITCD
                     DTND(L, 3) = 0
                  ENDIF
               ENDDO
            ENDIF
         ENDIF
      ENDDO

```

```

                                ENDIF
                                GOTO 50
                                ELSE
CALL NAMNUM(LDND, NDNAM, COLSTR(I), ND, 0, ERR)
                                IF (ERR) THEN
                                WRITE(*, 850) COLSTR(I), FILNAM
850    FORMAT( '***ERROR***NODAL NAME: ',A12,' IN THE FILE: ',A,
    &        /, ' NOT FOUND IN THE NETWORK CONFIGURATION.')
                                STOP !CALL EXIT
                                ELSE
                                DTND(I-1, 1) = ND
                                DTND(I-1, 2) = UNITCD
                                DTND(I-1, 3) = 0
                                ENDIF
                                ENDIF

ENDDO
NDTND = NNDS - 1
*
*-----Read data table
*
50  CONTINUE
    READ (IU, '(A)', END = 99) CTERM
    IF (CN(CTERM, 'FINISH', 1)) GOTO 99
    CALL STR_DIVD(CTERM, J, COLSTR, LDCOL, 0, ' ')
    IF (J .NE. NNDS) THEN
        PRINT *, '***ERROR*** FILE = ', FILNAM
        PRINT *, '          FOR TIME = ', COLSTR(1)
        STOP !CALL EXIT
    ENDIF

    NREC = NREC + 1
    DO J = 1, NNDS - 1
        READ (COLSTR(J+1), '(F15.0)') DTTB(NREC, J)
    ENDDO
    GOTO 50
*
*-----Output Title
*
99  CONTINUE
    IF (DUMMY) THEN
        DO I = 1, NREC
        DO J = NNDS, NDTND
            DTTB(I, J) = DTTB(I, NNDS-1)
        ENDDO
        ENDDO
    ENDIF
    IF (NREC .GT. 0) THEN
        ENDFIL = .FALSE.
        DTFLAG = .TRUE.
        IF (SAVOPT .EQ. 0 .OR. SAVOPT .EQ. 1) THEN
            CALL STR_LEN(TITLE, J)
            CALL STR_LEN(UNITNM(UNITCD), K)
            WRITE(OU, 801) PN, TITLE (1:J), UNITNM(UNITCD)(1:K),
    &        ('=', I = 1, J+K+5)
801    FORMAT(//, 'Part ', I2, ': Seasonal ', A, ' in ', A,
    &        /, '=====', 100A)
*
            IF (.NOT. DUMMY) THEN
                DO K = 1, NDTND, 8
                IF ((K+7) .GT. NDTND) THEN
                    L = NDTND
                ELSE
                    L = K + 7
                ENDIF
                WRITE(OU, 805)
    &        (NDNAM(DTND(J,1))(1:SL(NDNAM(DTND(J,1))))), J = K, L)
                DO I = 1, NREC
                WRITE (OU, 810) I, (DTTB(I, J), J = K, L)
                ENDDO
                ENDDO
                ELSE
                WRITE(OU, 805) (NDNAM(DTND(J,1))(1:SL(NDNAM(DTND(J,1))))),
    &        J = 1, NNDS-2), 'DEFAULT'
                DO I = 1, NREC
                WRITE (OU, 810) I, (DTTB(I, J), J = 1, NNDS-1)
                ENDDO
                ENDDO
            ENDIF
805    FORMAT(/, 'SEASON', T10, 10A10)
810    FORMAT (I5, T10, 10F10.2)

        ENDFIL
    ENDIF
*
999  CONTINUE
    RETURN
    END
*=====
* Name:      RESSTG
* Purpose:   Calculate reservoir stges.
*=====
SUBROUTINE RESSTG(NND, NDTP, NDSEQ, LDND,
    &        RC, INST, PTREAR, LDRES, REAR, LDREAR,
    &        FLOW, LDARC, XF)
    IMPLICIT NONE
    INTEGER LDND, LDRES, LDREAR, LDARC

```

```

      INTEGER    NND, NDTP(LDND), NDSEQ(LDND)
      INTEGER    PTREAR(LDRES), REAR(LDREAR)
      INTEGER    FLOW(LDARC)
      REAL       RC(LDRES), INST(LDRES), XF
*
      INTEGER    N, I, LIM1, LIM2, RES, ARC, TYP
      REAL       STG
      DO 200 N = 1, NND
          TYP = NDTP(N)
          IF (TYP .EQ. 1) THEN
              RES = NDSEQ(N)
              STG = RC(RES)
              LIM1 = PTREAR(RES)
              LIM2 = PTREAR(RES + 1) - 1
              DO 45 I = LIM1, LIM2
                  ARC = REAR(I)
                  IF (ARC .GT. 0) THEN
                      STG = STG + REAL(FLOW(ARC)) / XF
                  ELSE
                      ARC = -ARC
                      STG = STG - REAL(FLOW(ARC)) / XF
                  END IF
              END DO
          45      CONTINUE
              INST(RES) = STG
          ENDIF
      200  CONTINUE
      RETURN
      END
=====
* Name:          ndbud
* Purpose:       calculate nodal water budgets.
=====
      SUBROUTINE NDBUD(FLOW, LDARC, RC, OINST, INST, RESDAT, LDRES,
&                    NND, NDTP, NDSEQ, PTDWAR, LDND, DWAR, LDDWAR,
&                    NDXAR, NODBUD, XF, OU)
      IMPLICIT NONE
      INTEGER    LDARC, LDRES, LDND, LDDWAR,
&              FLOW(LDARC), NND, NDTP(LDND), NDSEQ(LDND),
&              PTDWAR(LDND, 2), DWAR(LDDWAR)
      INTEGER    NODBUD(LDND, 0:10), NDXAR(LDND, 6), OU
      REAL       RC(LDRES), OINST(LDRES), INST(LDRES), RESDAT(LDRES, 0:2)
      REAL       XF
*
      INTEGER    ISGN
*
      REAL       FLW, ZVA(3)
      INTEGER    I, J, N, TYP, LIM1, LIM2, ARC, RES
      LOGICAL    GETZVA
*
      DO 200 N = 1, NND
*-----Initialize budget array
*
          DO J = 0, 8
              NODBUD(N, J) = 0
          ENDDO
          TYP = NDTP(N)
*
*-----Total releases from a current node.
*
          FLW = 0.0
          LIM1 = PTDWAR(N, 1)
          LIM2 = PTDWAR(N, 2)
          DO 50 J = LIM1, LIM2
              ARC = DWAR(J)
              IF (ARC .GT. 0) THEN
                  FLW = FLW + REAL(FLOW(ARC))
              ELSE
                  FLW = FLW - REAL(FLOW(-ARC))
              END IF
          50      CONTINUE
              IF (TYP .EQ. 1) THEN
*-----1. Reservoir budget
*
                  RES = NDSEQ(N)
                  NODBUD(N, 0) = NINT(OINST(RES) * XF)
*
*-----2. Local gain and loss
*
                  DO I = 1, 5
                      ARC = NDXAR(N, I)
                      IF (ARC .NE. 0) THEN
                          IF (I .EQ. 1) THEN
                              NODBUD(N, 2) = ISGN(ARC) * FLOW(IABS(ARC))
                              - NINT((OINST(RES) - RC(RES)) * XF)
                          ELSE IF (I .EQ. 2) THEN
                              NODBUD(N, 3) = ISGN(ARC) * FLOW(IABS(ARC))
                          ELSE IF (I .EQ. 3) THEN
                              NODBUD(N, 4) = ISGN(ARC) * FLOW(IABS(ARC))
                          ELSE IF (I .EQ. 4) THEN
                              NODBUD(N, 5) = ISGN(ARC) * FLOW(IABS(ARC))
                          ELSE IF (I .EQ. 5) THEN
                              NODBUD(N, 6) = FLOW(NDXAR(N, 5)) - FLOW(NDXAR(N, 6))
                          END IF
                      END DO
                  END IF
              END IF
          END DO
      END

```

```

ENDIF
ENDIF
ENDDO
*
*-----3. Downstream release
*
NODBUD(N,7) = FLW
*
*-----4. Final storage and reservoir elevatoin.
*
NODBUD(N,8) = NINT(INST(RES) * XF)
ZVA(2) = INST(RES)
IF (GETZVA(RES, 2, ZVA, OU)) THEN
NODBUD(N,9) = INT(ZVA(1)*100.0)
NODBUD(N,10) = INT(NODBUD(N,9) - RESDAT(RES,0)*100.0)
ELSE
PRINT *, '***ERROR*** RESERVOIR STAGE'
STOP !CALL EXIT
ENDIF
*
*-----5. Upstream Inflow: Q = EV + L + W + R + S - I - P - So
*
NODBUD(N,1) = NODBUD(N,3) + NODBUD(N, 5) + NODBUD(N,6)
& + NODBUD(N,7) + NODBUD(N, 8)
& - NODBUD(N,2) - NODBUD(N,4) - NODBUD(N,0) !INFLOW
ELSE IF (TYP .EQ. 2) THEN
DO I = 1, 6
ARC = NDXAR(N,I)
IF (ARC .NE. 0) THEN
IF (I .EQ. 1) THEN
NODBUD(N,2) = ISGN(ARC) * FLOW(IABS(ARC))
ELSE IF (I .EQ. 3) THEN
NODBUD(N,4) = ISGN(ARC) * FLOW(IABS(ARC))
ELSE IF (I .EQ. 5) THEN
NODBUD(N,6) = FLOW(NDXAR(N,5)) - FLOW(NDXAR(N,6))
ENDIF
ENDIF
ENDDO
*
*-----2. downstream release
*
NODBUD(N,7) = FLW
*
*-----3. Upstream Inflow: Q = W + R - I - Runoff
*
NODBUD(N,1) = NODBUD(N,6) + NODBUD(N,7) - NODBUD(N,2)
& - NODBUD(N,3)
ENDIF
200 CONTINUE
RETURN
END
*=====
* Name: prtnd
* Purpose: Print nodal budget.
*=====
SUBROUTINE PRND(NND, NDNAM, NDTYP, NODBUD, LDND, NOP, XF, XP,
& NSNBL, SNBLND, LDSNBL, SNBLFG, NDBFLG,
& ZEROFG, IOUT)
& IMPLICIT NONE
INTEGER LDND, NND, NDTYP(LDND), NODBUD(LDND, 0:10), NOP, IOUT,
& XP, LDSNBL
& REAL XF
& INTEGER NSNBL, SNBLND(LDSNBL, 2)
LOGICAL SNBLFG, NDBFLG, ZEROFG
CHARACTER NDNAM(LDND)*(*)
*
INTEGER I, N, K, OU
REAL EPS
CHARACTER FMT2*60, CTERM*150
LOGICAL CNINT
*
EPS = 5.0 * 10**(-XP)
ZEROFG = .TRUE.
*
*-----Output time step
*
IF (NDBFLG) THEN
WRITE(IOUT, 990) NOP
990 FORMAT (/,T30, 'Water budgets for time step:', I3,
& /,T30, '=====',
& /,T30, ' [--, not applicable]',
& /, T21, ' Initial Upstream Local Net',
& /, Evapo- Downstream Final',
& /, ration storage inflow inflow',
& /, runoff Seepage Withdrawal release storage',
& /, Final Water',
& /, ' Node',T16, 'Node',T21, ' (acre- (acre- (acre-',
& /, (acre- (acre- (acre- (acre- (acre- (acre-',
& /, stage depth',

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```

& /, 'No. name', T16, 'type', T21, '      feet)      feet)      feet)',
&      (feet)      (feet)      feet)      feet)      feet)      feet)',
&      (feet)      (feet)',
& /, '-----', T16, '-----', T21, '-----',
&      '-----',
&      '-----')
ENDIF
*
*-----Save nodal water budgets
*
IF (XP .GT. 0) THEN
      FMT2 = '(F10.0)'
      WRITE(FMT2(6:6), '(I1)') XP - 1
      DO N = 1, NND
      CTERM = ''
*
*      Nodal name and type
*
      WRITE(CTERM(1:20), '(I3, 1X, A12, 1X, I2, 2X)')
&      N, NDNAM(N), NDTYP(N)
*
*      Water budgets
*
      DO I = 0, 8
      IF (NDTYP(N) .EQ. 2 .AND. (I .EQ. 0 .OR. I .EQ. 3
&      .OR. I .EQ. 5 .OR. I .EQ. 8)) THEN
      WRITE(CTERM((I+2)*10+1:(I+3)*10), '(6x,a2,2x)') '---'
      ELSE IF (ABS(NODBUD(N, I)) .GT. 0 .OR. ZEROFG) THEN
      WRITE(CTERM((I+2)*10+1:(I+3)*10), FMT2)
&      REAL(NODBUD(N, I)) / XF
      ENDDO
      CALL STR_LEN(CTERM, I)
*
*      Final stage and water depth
*
      IF (NDBFLG) THEN
      IF (NDTYP(N) .EQ. 1) THEN
      WRITE(IOUT, '(A, 2F10.2)') CTERM(1:I),
&      REAL(NODBUD(N, 9))/100., REAL(NODBUD(N, 10))/100.
      ELSE
      WRITE(IOUT, '(A, 2x, 2(6x,a2,2x))') CTERM(1:I), '---', '---'
      ENDDO
      ENDDO
      IF (SNBLFG .AND. CNINT(N, NSNBL, SNBLND, K) ) THEN
      OU = SNBLND(K, 2)
      CTERM = ''
      DO I = 0, 8
      IF (ABS(NODBUD(N, I)) .GT. EPS) THEN
      WRITE(CTERM(I*10+1:(I+1)*10), FMT2)
&      REAL(NODBUD(N, I)) / XF
      ELSE
      WRITE(CTERM(I*10+1:(I+1)*10), FMT2) 0.0
      ENDDO
      CALL STR_LEN(CTERM, I)
      IF (NDTYP(N) .EQ. 1) THEN
      WRITE(OU, '(I4, 6X, A, 2F10.2)')
&      NOP, CTERM(1:I), REAL(NODBUD(N, 9))/100.,
&      REAL(NODBUD(N, 10))/100.
      ELSE
      WRITE(OU, '(I4, 6X, A, F10.2)') NOP, CTERM(1:I)
      ENDDO
      ENDDO
      ELSE
      DO N = 1, NND
      CTERM = ''
      WRITE(CTERM(1:20), '(I3, 1X, A12, 1X, I2, 2X)')
&      N, NDNAM(N), NDTYP(N)
      DO I = 0, 8
      IF (ABS(NODBUD(N, I)) .GT. 0 .OR. ZEROFG) THEN
      WRITE(CTERM((I+2)*10+1:(I+3)*10), '(I10)') NODBUD(N, I)
      ENDDO
      CALL STR_LEN(CTERM, I)
      IF (NDBFLG) THEN
      IF (NDTYP(N) .EQ. 1) THEN
      WRITE(IOUT, '(A, 2F10.2)') CTERM(1:I),
&      REAL(NODBUD(N, 9))/100.0, REAL(NODBUD(N, 10))/100.0
      ELSE
      WRITE(IOUT, '(A)') CTERM(1:I)
      ENDDO
      ENDDO
      IF (SNBLFG .AND. CNINT(N, NSNBL, SNBLND, K) ) THEN
      OU = SNBLND(K, 2)
      CTERM = ''
      DO I = 0, 8
      IF (ABS(NODBUD(N, I)) .GT. EPS) THEN
      WRITE(CTERM(I*10+1:(I+1)*10), '(I10)')
&      NODBUD(N, I)

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*          bottom information, and zones.
*
5  CONTINUE
   NLZ = 0
   NUZ = 0
   READ (IU, '(A)', END = 100) CTERM
   IF (CTERM .EQ. ' ') GOTO 5
   CALL STR_DIVD(CTERM, NCOL, COLSTR, LDCOL, 0, ' ', ' ')
   IF (CN(COLSTR(1), 'FINISH', 1)) GOTO 100
*
*-----1. Reservoir node name and its nodal number
*
   CALL NAMNUM(LDND, NDNAM, COLSTR(1), ND, 0, ERR)
   IF (ERR) THEN
     PRINT *, '***Error*** There is no Z-V-A table for pond node:',
   & COLSTR(1)
     PRINT *, 'which is specified in the pond zoning file.'
     STOP
   ENDIF
   NRES = NRES + 1
   PTRES(NRES) = ND
   NDSEQ(ND) = NRES
*
   IF (SAVOPT .EQ. 0 .OR. SAVOPT .EQ. 1) THEN
     WRITE (OU, 801) NDNAM(ND), ND
801  FORMAT(/, A, T15, I10, T40,
   & 'Pond name and corresponding node number')
   ENDIF
*
*-----2. Stage unit code
*
   READ (COLSTR(2), '(I2)') UNITCD
*
*-----3. Reservoir initial stroage, bottom elvation, poolbed
*          hydraulic conductivity, and poolbed thickness
*
   READ (COLSTR(3), '(F10.0)') INEL
   IF (UNITCD .EQ. 0) THEN
     ZVA(1) = INEL
     IF (.NOT. GETZVA(ND, 1, ZVA, OU)) THEN
       ERR = .TRUE.
       WRITE (OU, 1) NDNAM(ND)
1    FORMAT ('***ERROR***',
   & 'IN TRANSFORMING INITIAL POOR ELEVATION INTO STORAGE'
   & ', ' FOR RESERVOIR: ', A, ' AT NETWK_1')
       STOP
     END IF
     INST(NRES) = ZVA(2)
   ELSE IF (UNITCD .EQ. 1) THEN
     INST(NRES) = INEL
   ENDIF
*
*-----4. Reservoir bottom elevation and hydraulic properties.
*
   DO I = 0, 2
     READ(COLSTR(I+4), '(F10.0)') RESDAT(NRES, I)
   ENDDO
   IF (SAVOPT .EQ. 0 .OR. SAVOPT .EQ. 1) THEN
     WRITE (OU, 803) (RESDAT(NRES, I), I = 0, 2)
803  FORMAT(5X, 3F10.2, T40,
   & 'Bottom elevation, hydraulic conductivity, and thickness')
   ENDIF
*
*-----5. Reservoir rule curve
*
   READ (COLSTR(7), '(F10.0)') RCEL
   IF (UNITCD .EQ. 0) THEN
     ZVA(1) = RCEL
     IF (.NOT. GETZVA(ND, 1, ZVA, OU)) THEN
       ERR = .TRUE.
       WRITE (OU, 2) NDNAM(ND)
2    FORMAT ('***ERROR*** IN TRANSFORMING RULE CURVE ELEVATION',
   & ', ' INTO CORRESPONDING STORAGE FOR RESERVOIR: ', A)
       STOP !CALL EXIT
     END IF
     RC(NRES) = ZVA(2)
   ELSE
     RC(NRES) = RCEL
   ENDIF
   IF (INST(NRES) .LE. EPS) THEN
     INST(NRES) = RC(NRES)
   ENDIF
   IF (SAVOPT .EQ. 0 .OR. SAVOPT .EQ. 1) THEN
     IF (UNITCD .EQ. 0) THEN
       WRITE (OU, 805) INST(NRES), RCEL,
   & 'Rule-curve elevation, in feet'
     ELSE
       WRITE (OU, 805) INST(NRES), RC(NRES),
   & 'Rule-curve storage, in acre-feet'
     ENDIF
805  FORMAT(3X, F12.2, T40, 'Initial storage, in acre-feet',
   & ', 3X, F12.2, T40, A)
   ENDIF

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```

      INTEGER      IU, NTLS, IN, OU, PARTNO
      REAL         PERD, C, XF
*
      INTEGER      I, J, L, N, U, ZONE, UPND, OUPND, MDND, DWND, ODWND,
& REAL           NCOL, LDCOL, NRND, CST, LZONE, UZONE, TYP
      REAL         LVL1, LBND, UBND, RTERM, EPS
      PARAMETER    (LDCOL = 15)
      CHARACTER    CTERM*150, COLSTR(LDCOL)*20, FILNAM*(*)
      LOGICAL      CN, ERR, STRM, ITOJ, FLAG
      INTEGER      SAVOPT
      COMMON       /SAVOPT/ SAVOPT
      DATA        EPS /0.00001/
*
*-----Open flow network file
*
      IF (FILNAM .NE. ' ') THEN
          FLAG = .TRUE.
          IU = 9
          CALL IO_OPFIL(IU, 1, FILNAM, 'Enter network file: ')
          DO I = 1, NTLS
              READ (IU, *)
              ENDDO
      ELSE
          FLAG = .FALSE.
          IU = IN
          CTERM = ' '
          DO WHILE (CTERM .EQ. ' ')
              READ (IU, '(A)', END = 100) CTERM
              ENDDO
          CALL STR_DIVD(CTERM, I, COLSTR, LDCOL, 0, ' ', ' ')
          READ (COLSTR(2), '(I2)') N
          IF (CN(COLSTR(1), 'PART', 1) .AND. N .EQ. PARTNO) THEN
              READ (IU, *) !SKIP THE VARIABLE LIST
          ELSE
              IF (CN(COLSTR(1), 'PART', 1) .AND. N .GT. PARTNO) THEN
                  BACKSPACE(1)
                  GOTO 100
              ENDIF
              PRINT *, CHAR(7)
              PRINT *, '***ERROR***ERROR IN FLOW NETWORK DATA'
              STOP !CALL EXIT
          ENDIF
      ENDIF
*
*-----Initialize some array
*
      DO I = 1, LDSTRM
          DO J = 0, 4
              STRMCF(I, J) = 0.0
          ENDDO
          DO J = 0, 6
              STRMAR(I, J) = 0
          ENDDO
      ENDDO
*
      IF (SAVOPT .EQ. 0 .OR. SAVOPT .EQ. 1) THEN
          WRITE (OU, 800)
800      FORMAT (//, 'Part 2: Network flow configuration',
&            /, '=====', /)
          WRITE (OU, 801)
801      FORMAT( T26, '      Lower      Upper',
&            T58, '      Initial      Evaporation',
&            /, T26, '      boundary boundary ',
&            T58, '      storage Travel-      coefficient',
&            /, T25, '(cubic feet (cubic feet Penalty ',
&            T60, '(acre- time Weighting Seepage (inches ',
&            /, T25, 'per second) per second) coefficient',
&            T60, 'feet) (day) factor coefficient per day)',
&            /, '-----', T12, '-----',
&            T25, '-----',
&            T60, '-----')
      ENDIF
      NARCND = 0
      OUPND = 0
      ODWND = 0
      NDWAR = 0
      NRND = 0
5      CONTINUE
      READ (IU, '(A)', END = 100) CTERM
      IF (CTERM .EQ. ' ') GOTO 5
      CALL STR_DIVD(CTERM, NCOL, COLSTR, LDCOL, 0, ' ', ' ')
      IF (CN(COLSTR(1), 'FINISH', 1)) GOTO 100
      STRM = .FALSE.
      IF (NCOL .GT. 5) THEN
          DO I = 6, NCOL
              READ (COLSTR(I), '(F10.0)') RTERM
              IF (ABS(RTERM) .GT. EPS) THEN
                  STRM = .TRUE.
              ENDIF
          ENDDO
      ENDIF
*
*-----Upstream node and downstream node

```

```

*
CALL NAMNUM(LDND, NDNAM, COLSTR(1), UPND, 0, ERR)
IF (ERR) THEN
    NNODS = NNODS + 1
    NDNAM(NNODS) = COLSTR(1)
    CALL STR_CORS(NDNAM(NNODS), 1)
    NDTYP(NNODS) = 2
    UPND = NNODS
ENDIF
CALL NAMNUM(LDND, NDNAM, COLSTR(2), DWND, 0, ERR)
IF (ERR) THEN
    NNODS = NNODS + 1
    NDNAM(NNODS) = COLSTR(2)
    CALL STR_CORS(NDNAM(NNODS), 1)
    NDTYP(NNODS) = 2
    DWND = NNODS
ENDIF
IF (UPND .NE. OUPND) THEN
    NRND = NRND + 1
    PTDWAR(UPND, 1) = NDWAR + 1
    OUPND = UPND
    ODWND = 0
ENDIF
*
IF (DWND .NE. ODWND) THEN
    ZONE = 1
    UZONE = 0
    LZONE = 0
    ODWND = DWND
ELSE
    ZONE = ZONE + 1
ENDIF
*
*-----Determine flow bounds and flow direction.
*
READ (COLSTR(3), '(F10.0)') LBND
READ (COLSTR(4), '(F10.0)') UBND
READ (COLSTR(5), '(I10)') CST
IF (ZONE .EQ. 1) THEN
    ITOJ = .TRUE.
    LVL1 = UBND
    IF (LBND .GE. 0) THEN
        L = NINT(LBND * C * PERD * XF)
    ELSE
        L = 0
    ENDIF
    U = NINT(UBND * C * PERD * XF)
    IF (STRM) THEN
        NARCND = NARCND + 1
        MDND = LDND - NARCND
        NDNAM(MDND) = 'STRM'
        WRITE(NDNAM(MDND)(5:10), '(I3.3,I3.3)')UPND, DWND
    ENDIF
ELSE
    IF (UBND .GE. LVL1) THEN
        ITOJ = .TRUE.
        UZONE = UZONE + 1
    ELSE
        ITOJ = .FALSE.
        LZONE = LZONE + 1
    ENDIF
    L = 0
    U = NINT((UBND - LBND) * C * PERD * XF)
ENDIF
*
*-----Create flow arcs.
*
IF (IToj) THEN
    IF (STRM) THEN
        TYP = 110 + UZONE
        NSTRM = NSTRM + 1
        STRMAR(NSTRM, 0) = MDND
        CALL ARCVAL(NARCS, II, JJ, LO, HI, COST, ARTYP,
& LDARC, UPND, MDND, L, U, CST, TYP)
        NDWAR = NDWAR + 1
        DWAR(NDWAR) = NARCS
        PTDWAR(UPND, 2) = NDWAR
        STRMAR(NSTRM, 1) = NARCS
        CALL ARCVAL(NARCS, II, JJ, LO, HI, COST, ARTYP,
& LDARC, MDND, DWND, L, U, CST, TYP)
        STRMAR(NSTRM, 6) = NARCS
        DO I = 6, NCOL
            J = I - 6
            READ (COLSTR(I), '(F15.0)') STRMCF(NSTRM, J)
        ENDDO
    ELSE
        TYP = 100 + UZONE
        CALL ARCVAL(NARCS, II, JJ, LO, HI, COST, ARTYP,
& LDARC, UPND, DWND, L, U, CST, TYP)
        NDWAR = NDWAR + 1
        DWAR(NDWAR) = NARCS
        PTDWAR(UPND, 2) = NDWAR
    ENDIF
ELSE

```

```

        IF (STRM) THEN
            TYP = -110 - LZONE
            NSTRM = NSTRM + 1
            STRMAR(NSTRM, 0) = MDND
            CALL ARCVAL(NARCS, II, JJ, LO, HI, COST, ARTYP,
& LDARC, MDND, UPND, L, U, CST, TYP)
            NDWAR = NDWAR + 1
            DWAR(NDWAR) = -NARCS
            PTDWAR(UPND, 2) = NDWAR
            STRMAR(NSTRM, 1) = -NARCS
            CALL ARCVAL(NARCS, II, JJ, LO, HI, COST, ARTYP,
& LDARC, DWND, MDND, L, U, CST, TYP)
            STRMAR(NSTRM, 6) = -NARCS
            DO I = 6, NCOL
                J = I - 6
                READ (COLSTR(I), '(F15.0)') STRMCF(NSTRM, J)
            ENDDO
            ELSE
                TYP = -110 - LZONE
                CALL ARCVAL(NARCS, II, JJ, LO, HI, COST, ARTYP,
& LDARC, DWND, UPND, L, U, CST, TYP)
                NDWAR = NDWAR + 1
                DWAR(NDWAR) = -NARCS
                PTDWAR(UPND, 2) = NDWAR
            ENDIF

        ENDIF

*
*       if lower bound of the current flow range is less than zero,
*       then the current arc is control arc and the lower flow bound
*       will be determined.
*
        IF (LBND .LT. 0) THEN
            NCTAR = NCTAR + 1
            CTARFW(NCTAR, 1) = NARCS
            CTARFW(NCTAR, 2) = 0
            CTARFW(NCTAR, 3) = - NINT(LBND * C * REAL(PERD) * XF)
        ENDIF

*
        IF (SAVOPT .EQ. 0 .OR. SAVOPT .EQ. 1) THEN
            IF (STRM) THEN
                WRITE(OU, 805) NDNAM(UPND), NDNAM(DWND), LBND, UBND, CST,
& (STRMCF(NSTRM, I), I = 0, 4)
805      FORMAT(2A12, T26, 2F10.2, I10, 5F10.2)
            ELSE
                WRITE(OU, 806) NDNAM(UPND), NDNAM(DWND), LBND, UBND, CST
806      FORMAT(2A12, T26, 2F10.2, I10)
            ENDIF
        ENDIF

*
*       next arc
100      GOTO 5
        CONTINUE
        IF (FLAG) THEN
            CLOSE (IU)
        ENDIF
        RETURN
        END

*=====
* Name:      strm_dat
* Purpose:   Read stream geometry data.
* Author:    Xiaodong Jian
* Date:      12/05/95
*=====
        SUBROUTINE STRM_DAT(NDNAM, LDND, II, JJ, LDARC,
& NSTRM, STRMAR, LDSTRM,
& NSTR, STRDIR, STRDAT, LDSTR,
& FILNAM, NTLS, IN, OU, PARTNO)
        IMPLICIT NONE
        INTEGER LDND
        CHARACTER NDNAM(LDND)*(*), FILNAM*(*)
        INTEGER LDARC, II(LDARC), JJ(LDARC), NTLS
        INTEGER NSTRM, LDSTRM, STRMAR(LDSTRM, 0:6)
        INTEGER NSTR, LDSTR, STRDIR(LDSTR, 3)
        REAL STRDAT(LDSTR, 9)

*
        INTEGER LDCOL
        PARAMETER (LDCOL = 15)
        CHARACTER CTERM*150, COLSTR(LDCOL)*15
        INTEGER ND, UPND, DWND
        INTEGER I, J, K, N, IN, IU, OU, PARTNO
        LOGICAL ERR, CN, FLAG
        INTEGER SAVOPT
        COMMON /SAVOPT/ SAVOPT

*
        NSTR = 0

*
*-----open data file and read title lines
*
        IF (FILNAM .NE. ' ') THEN
            FLAG = .TRUE.
            IU = 9
            CALL IO_OPFIL(IU, 1, FILNAM, 'ENTER THE STREAM GEOMETRIC FILE: ')
            DO I = 1, NTLS
                READ (IU, *, END = 100)
            END DO
        END IF
    
```

```

      ENDDO
ELSE
      FLAG = .FALSE.
      IU = IN
      CTERM = \
      DO WHILE (CTERM .EQ. \ \)
      READ (IU, \ (A)', END = 999) CTERM
      ENDDO
      CALL STR_DIVD(CTERM, I, COLSTR, LDCOL, 0, \ ,')
      READ (COLSTR(2), \ (I2)') N
      IF (CN(COLSTR(1), 'PART', 1) .AND. N .EQ. PARTNO) THEN
      READ (IU, *) !SKIP THE VARIABLE LIST
      ELSE
      IF (CN(COLSTR(1), 'PART', 1) .AND. N .GT. PARTNO) THEN
      BACKSPACE(1)
      GOTO 100
      ENDIF
      PRINT *, CHAR(7)
      PRINT *, '***ERROR***ERROR IN STREAM DATA'
      STOP !CALL EXIT
      ENDIF
ENDIF
*
IF (SAVOPT .EQ. 0 .OR. SAVOPT .EQ. 1) THEN
      WRITE(OU, 800)
800  FORMAT(//, 'PART 3: Canal geometry data',
&      //, '=====')
&      /,
&      /, T25, \
&      /, Riverbed
&      /, T25, \
&      /, hydraulic
&      /, T25, \
&      /, Bottom Canal conductivity Riverbed Entry
&      /, 'F-node', T13, 'T-node', T25, \ Roughness length Canal,
&      /, width Side depth (feet thickness elevation',
&      /, 'name', T13, 'name', T24, 'coefficient (feet) slope',
&      /, (feet) slope (feet) per day (feet) (feet)',
&      /, '-----', T13, '-----', T24, '-----')
&
ENDIF
*
*-----Read data
*
5  CONTINUE
READ (IU, \ (A)', END = 100) CTERM
IF (CTERM .EQ. \ \) GOTO 5
CALL STR_DIVD(CTERM, K, COLSTR, LDCOL, 0, \ ,')
IF (CN(COLSTR(1), 'FINISH', 1)) GOTO 100
IF (K .LT. 7) THEN
      PRINT *, CHAR(7)
      PRINT *, '***ERROR*** INCOMPLETE DATA SET IN GEOMETRIC FILE'
      STOP !CALL EXIT
ENDIF
NSTR = NSTR + 1
DO J = 1, K
      IF (J .LE. 2) THEN
      CALL NAMNUM(LDND, NDNAM, COLSTR(J), ND, 0, ERR)
      STRDIR(NSTR, J) = ND
      ELSE
      READ(COLSTR(J), \ (F15.0)') STRDAT(NSTR, J-2)
      ENDIF
ENDIF
ENDDO
*
IF (SAVOPT .EQ. 0 .OR. SAVOPT .EQ. 1) THEN
      WRITE (OU, 805) (NDNAM(STRDIR(NSTR,J)), J=1,2),
&      (STRDAT(NSTR,J), J=1,9)
805  FORMAT(A12, A12, F10.4, F10.2, F10.6, F10.2, F10.6, 4F10.2)
ENDIF
GOTO 5
100 CONTINUE
IF (FLAG) CLOSE(IU)
*
* Find the middle stream node between the upstream node
* and downstream node.
*
DO 20 I = 1, NSTR
      DO J = 1, NSTRM
      UPND = II(IABS(STRMAR(J, 1)))
      DWND = JJ(IABS(STRMAR(J, 6)))
      IF(UPND .EQ. STRDIR(I, 1) .AND. DWND .EQ. STRDIR(I, 2)) THEN
      STRDIR(I, 3) = STRMAR(J, 0)
      GOTO 20
      ENDIF
      ENDDO
      STRDIR(I, 3) = 0
20  CONTINUE
999 RETURN
END
*=====
* Name:      strm_rout
* Purpose:   Channel Routing.
*=====
SUBROUTINE STRM_ROUT(
&      NARCS, II, JJ, HI, LO, COST, FLOW, ARTYP, MXARC,

```

```

&      NSTRM, STRMCF, STRMAR, LDSTRM,
&      NSTR, STRDIR, STRDAT, LDSTR,
&      NGWND, GWND, GWLVL, LDGWND,
&      NEV, EVND, LDEV, EVIDX, EVTB, LDEVTB,
&      SKSC, PERD, XF)
IMPLICIT
INTEGER
INTEGER
&      NARCS, MXARC, NSTRM, LDSTRM, SKSC
&      II(MXARC), JJ(MXARC), HI(MXARC), LO(MXARC),
&      COST(MXARC), ARTYP(MXARC), FLOW(MXARC)
REAL
INTEGER
INTEGER
REAL
INTEGER
REAL
&      STRMCF(LDSTRM, 0:4)
&      STRMAR(LDSTRM, 0:6)
&      NSTR, LDSTR, STRDIR(LDSTR, 3)
&      STRDAT(LDSTR, 9)
&      NGWND, LDGWND, GWND(LDGWND)
&      GWLVL(LDGWND)
*
INTEGER
REAL
&      NEV, LDEV, EVND(LDEV,3), EVIDX, LDEVTB
&      EVTB(0:LDEVTB, LDEV), EV, AREA
*
REAL
&      PERD, XF
*
INTEGER
INTEGER
&      I, J, ND, ND1, ND2, ARC, ISGN, S, VAL, ARC1, ARC2
&      OND1, OND2
*
REAL
&      K, X, IFLW, OFLW, ROUGH, LENGTH, SLOPE, WIDTH,
&      ML, MR, HMAX, DISCH
&      IERR
&      REAL
&      H, HGW, KY, DY, ZB, FAC
*
LOGICAL
REAL
DATA
&      FLAG, EVFLAG
&      EPS
&      EPS /0.0001/
*
OND1 = 0
OND2 = 0
DO 100 S = 1, NSTRM
*
*-----arc and node information
*
ND = STRMAR(S, 0)
ARC1 = STRMAR(S, 1)
ARC2 = STRMAR(S, 6)
IF (ARC1 .GT. 0) THEN
ND1 = II(ARC1)
ND2 = JJ(ARC2)
ELSE
ND1 = JJ(-ARC1)
ND2 = II(-ARC2)
ENDIF
*
*-----The channel surface evaporation occurs only in the normal flow
*      range because flow are supposed in the normal flow range.
*
IF (ND1 .NE. OND1 .OR. ND2 .NE. OND2) THEN
OND1 = ND1
OND2 = ND2
EVFLAG = .TRUE.
ELSE
EVFLAG = .FALSE.
ENDIF
*
*-----channel geometric data
*
FLAG = .FALSE.
DO I = 1, NSTR
AREA = 0.0
IF (ND .EQ. STRDIR(I, 3)) THEN
ROUGH = STRDAT(I, 1)
LENGTH = STRDAT(I, 2)
SLOPE = STRDAT(I, 3)
WIDTH = STRDAT(I, 4)
ML = STRDAT(I, 5)
MR = ML
HMAX = STRDAT(I, 6)
KY = STRDAT(I, 7)
DY = STRDAT(I, 8)
ZB = STRDAT(I, 9) - 0.5 * LENGTH * SLOPE
FLAG = .TRUE.
*
*-----Using Manning's equation to estimate water depth and width
*
IFLW = FLOW(ABS(ARC1)) / PERD * 43560 / XF
OFLW = FLOW(ABS(ARC2)) / PERD * 43560 / XF
DISCH = 0.5 * (IFLW + OFLW) / 86400.0
CALL CHDEP(ROUGH, DISCH, SLOPE, WIDTH, ML, MR, HMAX, H,
&      1.486, 0.001, 200, IERR)
&      WIDTH = WIDTH + ML*H + MR*H
&      IF (H .GT. EPS) THEN
&      AREA = WIDTH * LENGTH
&      ENDIF
&      IF (IERR .EQ. 0) THEN
&      CONTINUE
&      ELSE IF (IERR .EQ. 1) THEN
PRINT *, '***WARNING*** H > HMAX FOR CHANNEL: ', S

```

```

CONTINUE
  ENDIF
  GOTO 10
ENDIF
ENDDO

10    CONTINUE
*
*-----A. Channel seepage
*
      VAL = 0
      IF (STRMCF(S, 3) .GT. EPS) THEN
        ARC = STRMAR(S, 1)
        VAL = NINT(ISGN(ARC) * STRMCF(S, 3) * FLOW(IABS(ARC)))
      ELSE IF (STRMCF(S, 3) .LT. -EPS .AND. FLAG) THEN
*
*-----2. Estimate Groundwater level if available
*
      IF (NGWND .GT. 0) THEN
        HGW = 0.0
        J = 0
        DO I = 1, NGWND
          IF (ND1 .EQ. GWND(I) .OR. ND2 .EQ. GWND(I)) THEN
            J = J + 1
            HGW = HGW + GWLVL(I)
          ENDIF
        ENDDO
        IF (J .GT. 0) THEN
          HGW = HGW / REAL(J)
        ENDIF
      ENDIF
*
*-----3. calculate seepages
*
      IF (ZB .GT. EPS .AND. HGW .GT. ZB) THEN
        H = H + ZB - HGW
      ENDIF
      VAL = NINT(KY * H / DY * LENGTH * WIDTH * PERD/43560. * XF)
    ENDIF
20    CONTINUE
      IF (ABS(VAL) .GT. EPS) THEN
        IF (VAL .GT. 0) THEN
          CALL ARCVL(NARCS, II, JJ, LO, HI, COST, ARTYP,
&            MXARC, ND, SKSC, VAL, VAL, 0, 10)
        ELSE IF (VAL .LT. 0) THEN
          CALL ARCVL(NARCS, II, JJ, LO, HI, COST, ARTYP,
&            MXARC, SKSC, ND, -VAL, -VAL, 0, -10)
        ENDIF
        STRMAR(S, 3) = ISGN(VAL) * NARCS
      ELSE
        STRMAR(S, 3) = 0
      ENDIF
*
*-----B. Channel water surface evaporation. Surface evaporation coeffiecints
* are obtained in two ways: (1) If the channel water surface
* evaporation coefficient is specified, this value will be used
* for whole simulation period, and (2) If the ev value is not specified
* the values in the upstream and downstream nodes will be averaged.
*
      IF (ABS(STRMCF(S, 4)) .GT. EPS .AND. EVFLAG .AND.
&        AREA .GT. EPS) THEN
        EV = 0.0
        IF (STRMCF(S, 4) .GT. 0) THEN
          EV = STRMCF(S, 4)
        ELSE IF (NEV .GT. 0) THEN
          J = 0
          DO I = 1, NEV
            IF (ND1 .EQ. EVND(I, 1) .OR. ND2 .EQ. EVND(I, 1)) THEN
              J = J + 1
              IF (EVND(I, 2) .EQ. 0) THEN
                FAC = 1.0 / 25.4
              ELSE IF (EVND(I, 2) .EQ. 2) THEN
                FAC = 12.0
              ELSE
                FAC = 1.0
              ENDIF
              IF (EVND(I, 3) .NE. 0) THEN
                EV = EV + EVTB(0, I) * FAC
              ELSE
                EV = EV + EVTB(EVIDX, I) * FAC
              ENDIF
            ENDIF
          ENDDO
          IF (J .GT. 0) THEN
            EV = EV / J
          ENDIF
        ENDIF
        IF (EV .GT. EPS) THEN
          VAL = NINT(EV * PERD / 12.0 * AREA / 43560.0 * XF)
          CALL ARCVL(NARCS, II, JJ, LO, HI, COST, ARTYP,
&            MXARC, ND, SKSC, VAL, VAL, 0, 11)
          STRMAR(S, 4) = ISGN(VAL) * NARCS
        ELSE
          STRMAR(S, 4) = 0
        ENDIF
      ENDIF

```

```

                                ENDIF
*
*-----C. Channel Routing
*
*                                IF (ABS(STRMCF(S,1)) .GT. EPS) THEN
*-----Initial storage arc
*
*                                VAL = NINT(STRMCF(S, 0) * XF)
*                                IF (VAL .GT. 0) THEN
*                                CALL ARCVAL(NARCS, II, JJ, LO, HI, COST, ARTYP,
&                                MXARC, SKSC, ND, VAL, VAL, 0, 8)
*                                STRMAR(S, 2) = ISGN(VAL) * NARCS
*                                ELSE IF (VAL .LT. 0) THEN
&                                CALL ARCVAL(NARCS, II, JJ, LO, HI, COST, ARTYP,
*                                MXARC, ND, SKSC, -VAL, -VAL, 0, -8)
*                                STRMAR(S, 2) = ISGN(VAL) * NARCS
*                                ELSE
*                                STRMAR(S, 2) = 0
*                                ENDIF
*
*-----Final Storage arc
*
*                                ARC = STRMAR(S, 1)
*                                IFLW = FLOW(ABS(ARC1)) / PERD * 43560 / XF ! FLOW RATE IN CFD
*                                OFLW = FLOW(ABS(ARC2)) / PERD * 43560 / XF
*                                K = STRMCF(S,1)
*                                X = STRMCF(S,2)
*                                VAL = NINT(K * (X * IFLW + (1.0 - X) * OFLW)/43560.0 * XF)
*                                VAL = ISGN(ARC) * VAL
*
*                                IF (VAL .GT. 0) THEN
&                                CALL ARCVAL(NARCS, II, JJ, LO, HI, COST, ARTYP,
*                                MXARC, ND, SKSC, VAL, VAL, 0, 9)
*                                STRMAR(S, 5) = ISGN(VAL) * NARCS
*                                ELSE IF (VAL .LT. 0) THEN
&                                CALL ARCVAL(NARCS, II, JJ, LO, HI, COST, ARTYP,
*                                MXARC, SKSC, ND, -VAL, -VAL, 0, -9)
*                                STRMAR(S, 5) = ISGN(VAL) * NARCS
*                                ELSE
*                                STRMAR(S, 5) = 0
*                                ENDIF
*                                ENDIF
100 CONTINUE
*
99 RETURN
END
=====
* Name:      strm_kx
* Purpose:   Estimate routing coefficients: travel time K and weighting
*            factor x if these coefficient are not specified.
*=====
SUBROUTINE STRM_KX(NSTRM, STRMAR, STRMCF, LDSTRM,
& NSTR, STRDIR, STRDAT, LDSTR)
IMPLICIT NONE
INTEGER NSTRM, LDSTRM, NSTR, LDSTR, METHOD
INTEGER STRMAR(LDSTRM,0:6), STRDIR(LDSTR, 3)
REAL STRMCF(LDSTRM, 0:3), STRDAT(LDSTR, 9)
*
REAL N, L, S, W, M, D, R, P
REAL K, X, EPS
INTEGER I, J, ND
DATA EPS/0.00001/
*
DO 100 J = 1, NSTRM
ND = STRMAR(J, 0)
IF (STRMCF(J, 1) .LT. EPS) THEN
DO I = 1, NSTR
IF (ND .EQ. STRDIR(I, 3)) GOTO 50
ENDDO
GOTO 100
50 CONTINUE
METHOD = -NINT(STRMCF(J, 1))
N = STRDAT(I, 1)
L = STRDAT(I, 2)
S = STRDAT(I, 3)
W = STRDAT(I, 4)
M = STRDAT(I, 5)
D = STRDAT(I, 6)
*
IF (METHOD .EQ. 1) THEN
K = 0.7872 * L * W**0.4 * N**0.6 / S**0.3 / 86400.0
X = 0.0
ELSE IF (METHOD .EQ. 2) THEN
P = W + 2.0 * D * SQRT(1.0 + M * M)
R = (W + M * D) * D / P
K = L * N / (1.49 * (R**0.6667) * SQRT(S)) / 86400.0
X = 1.49 * SQRT(S) / (N * P**0.6667)
ENDIF
STRMCF(J, 1) = K
STRMCF(J, 2) = X
ENDIF
100 CONTINUE
RETURN

```



```

      END
=====
* Name:      tws_dat
* Purpose:    Get seasonal target water demands.
=====
      SUBROUTINE TWS_DAT(NNODS, NDNAM, LDND, NWSND, WSND, LDWS, NPER,
&      WSTB, LDWSTB, WSUNIT, WSFLAG, PN, FILNAM, NTLN,
&      IN, OU, UNITNM, LDUNIT, CTERM, COLSTR, LDCOL)
      IMPLICIT NONE
      INTEGER NNODS, LDND, LDCOL
      CHARACTER*(*) NDNAM(LDND), FILNAM, CTERM, COLSTR(LDCOL)
      INTEGER NWSND, LDWS, WSND(LDWS, 3), WSUNIT, NPER, LDWSTB
      REAL WSTB(0:LDWSTB, LDWS)
      INTEGER NTLN, IN, PN
      LOGICAL WSFLAG

      INTEGER I, NREC, IU, OU
      LOGICAL FLAG, ENDFIL
      INTEGER LDUNIT
      CHARACTER UNITNM(0:LDUNIT)*(*)

      -----Open data file and skip title lines
      IF (FILNAM .NE. ' ') THEN
         FLAG = .TRUE.
         IU = 9
         CALL IO_OPFIL(IU, 1, FILNAM, 'Enter water demand file: ')
         DO I = 1, NTLN !SKIP TITLE LINES
            READ (IU, *, END = 99)
         ENDDO
      ELSE
         FLAG = .FALSE.
         IU = IN
         CALL PNCK(PN, IU, ENDFIL, CTERM, COLSTR, LDCOL)
         IF (ENDFIL) GOTO 99
      ENDIF

      -----Read units, nonal names, and data from a file and
      print these information into general output file.
      CALL DATB(NNODS, NDNAM, LDND, NWSND, WSND, LDWS, WSUNIT,
&      NREC, WSTB, LDWSTB, WSFLAG,
&      UNITNM, LDUNIT, FILNAM, IU, OU, PN, ENDFIL,
&      'target water demands',
&      CTERM, COLSTR, LDCOL)
      IF (NREC .LT. NPER) THEN
         WRITE (OU, 805) NPER, NREC
805      FORMAT('***WARNING*** NO. OF SEASONAL RECORDS ARE LESS',
&      ' THAN NO. OF SEASONS.',
&      ' NO. OF RECORDS: ', I3,
&      ' NO. OF SEASONS: ', I3)
      ENDIF

99      CONTINUE
      IF (FLAG) CLOSE(IU)
      RETURN
      END
=====
* Name:      tws_arc
* Purpose:    Read current target water demand and create
              corresponding TWS arcs.
=====
      SUBROUTINE TWS_ARC(NARC, II, JJ, LO, HI, COST, ARTYP, LDARC,
&      NWSND, WSND, LDWS, WSTB, LDWSTB, WSFIL,
&      NDXAR, LDXAR, MTH, SKSC, PERD, MXCST, XF, IU,
&      CTERM, COLSTR, LDCOL)
      IMPLICIT NONE
      INTEGER NARC, LDARC,
&      II(LDARC), JJ(LDARC), LO(LDARC), HI(LDARC),
&      COST(LDARC), ARTYP(LDARC)
      INTEGER NWSND, LDWS, WSND(LDWS, 3), LDWSTB
      REAL WSTB(0:LDWSTB, LDWS)
      LOGICAL WSFIL
      INTEGER LDXAR, NDXAR(LDXAR, 6)

      INTEGER IU, MTH, SKSC, MXCST
      REAL PERD, XF

      INTEGER LDCOL
      CHARACTER CTERM*(*), COLSTR(LDCOL)*(*)

      INTEGER I, J, N, ND, NNDS, NV
      REAL UC(0:2)
      LOGICAL TDFLAG

      -----Data unit conversion factor (ac-ft)
      UC(0) = 1.0
      UC(1) = PERD * 86400.0 / 43560.0
      UC(2) = PERD / 43560.0

      -----Read time-dependent target water demands from a file.
      TDFLAG = .FALSE.

```

```

      IF (WSFIL) THEN
          READ (IU, '(A)', END = 50) CTERM
          CALL STR_DIVD(CTERM, NNDS, COLSTR, LDCOL, 0, ' ', ' ')
          TDFLAG = .TRUE.
      ENDIF
50  CONTINUE
*
*-----Create the target water demand arcs
*
      DO I = 1, NWSND
          ND = WSND(I, 1)
          IF (WSND(I, 3) .EQ. 0) THEN
              N = MTH
          ELSE
              N = 0
          IF (TDFLAG) THEN
              IF (WSND(I, 3) .LT. 0) THEN
                  READ(COLSTR(NNDS), '(F10.0)') WSTB(0, I)
              ELSE
                  J = WSND(I, 3) + 1
                  READ (COLSTR(J), '(F10.0)') WSTB(0, I)
              ENDIF
          ELSE
              WSTB(0, I) = 0.0
          ENDIF
          ENDIF
          NV = NINT(WSTB(N, I) * UC(WSND(I, 2)) * XF)
          CALL ARCVL(NARC, II, JJ, LO, HI, COST, ARTYP,
& LDARC, ND, SKSC, NV, NV, 0, 6)
          NDXAR(ND, 5) = NARC
          CALL ARCVL(NARC, II, JJ, LO, HI, COST, ARTYP,
& LDARC, SKSC, ND, 0, NV, MXCST*10, -6)
          NDXAR(ND, 6) = NARC
      ENDDO
99  RETURN
      END
*=====
* Name:      NVAR
* Purpose:   GET THE NET INFLOW TO THE ALL NODES AND CREATE THE NET
*            FLOW ARCS
*=====
      SUBROUTINE NVAR(NNODS, NDNAM, NDTP, NDSEQ, LDND,
& NARCS, II, JJ, HI, LO, COST, ARTYP, LDARC,
& MTH, IN_IFW, LAST, MXR,
& INST, RC, SKSC,
& NDXAR, PERD, XF,
& NIFW, IFWND, LDIFW,
* PTRE, LDRES, REAR, REZN, LDREAR,
* NRCND, RCND, LDRC, RCTB, LDRCTB, RCFLAG, RCFIL,
* IN_RC,
& APRX, IOUT1,
& RPOOL, LDPL, CTERM, COLSTR, LDCOL)
      IMPLICIT NONE
      LDARC, LDND, MXR, LDRCTB
      INTEGER II(LDARC), JJ(LDARC), HI(LDARC), LO(LDARC),
& COST(LDARC), ARTYP(LDARC)
      INTEGER NNODS, SKSC, NARCS, IOUT1
      REAL RC(MXR)
      REAL APRX, INST(MXR)
      LOGICAL LAST
      CHARACTER NDNAM(LDND)*(*)
      INTEGER NDTP(LDND), NDSEQ(LDND), NDXAR(LDND, 6)
      INTEGER MTH, IN_IFW, RES
      INTEGER NIFW, LDIFW, IFWND(LDIFW, 3)
      INTEGER LDRES, PTRE(LDRES), LDREAR, REAR(LDREAR)
      REAL REZN(LDREAR)
      INTEGER NRCND, LDRC, RCND(LDRC), IN_RC
      REAL RCTB(0:LDRCTB, LDRC)
      LOGICAL RCFLAG, RCFIL
*
      INTEGER LDPL, LDCOL
      REAL RPOOL(LDPL)
      CHARACTER CTERM*(*), COLSTR(LDCOL)*(*)
      LOGICAL DEBUG
*
      REAL PERD, XF
*
      REAL NVS
      INTEGER NV
      INTEGER I, J, K, L, NNDS
      REAL UC(0:2)
*
      DEBUG = .FALSE.
      LAST = .FALSE.
*
      read Incremental inflow to each node for current time step.
*
      DO I = 1, NNODS
          RPOOL(I) = 0.0
      ENDDO
*
      UC(0) = 1.0
      UC(1) = PERD * 86400.0 / 43560.0
      UC(2) = PERD / 43560.0

```

```

*
READ (IN_IFW, '(A)', END = 50) CTERM
CALL STR_DIVD(CTERM, NNDS, COLSTR, LDCOL, 0, ' ')
*
DO I = 1, NIFW
      IF (IFWND(I, 3) .GT. 0) THEN
            J = IFWND(I, 3) + 1
            READ (COLSTR(J), '(F10.0)') RPOOL(IFWND(I,1))
            ELSE IF (IFWND(I, 3) .LT. 0) THEN
            READ (COLSTR(NNDS), '(F10.0)') RPOOL(IFWND(I,1))
            ENDIF
      RPOOL(IFWND(I,1)) = RPOOL(IFWND(I,1)) * UC(IFWND(I, 2))
ENDDO
*
*-----Modify the rule cure
*
50  CONTINUE
    IF (RCFLAG) THEN
          CALL RC_ARC(NDNAM, NDSEQ, LDND, HI, LDARC,
&          PTRE, RC, LDRES, REAR, REZN, LDREAR,
&          NRCND, RCND, LDRC, RCTB, LDRCTB, RCFIL,
&          MTH, XF, IN_RC, CTERM, COLSTR, LDCOL)
    ENDIF
*
*-----Create the NV arcs
*
DO 60 K = 1, NNODS
  IF (NDTYP(K) .EQ. 1) THEN
        RES = NDSEQ(K)
        NVS = (INST(RES) + RPOOL(K) - RC(RES)) * XF
        IF (NVS .GT. 0.) THEN
              NV = NVS + APRX
        CALL ARCVAL(NARCS, II, JJ, LO, HI, COST, ARTYP,
&          LDARC, SKSC, K, NV, NV, 0, 3)
        NDXAR(K, 1) = NARCS
        ELSE IF (NVS .LT. 0.) THEN
              NV = NVS - APRX
        CALL ARCVAL(NARCS, II, JJ, LO, HI, COST, ARTYP,
&          LDARC, K, SKSC, -NV, -NV, 0, -3)
        NDXAR(K, 1) = -NARCS
        ENDIF
  ELSE IF (NDTYP(K) .EQ. 2) THEN
        NVS = RPOOL(K) * XF
        IF (NVS .GT. 0.) THEN
              NV = NVS + APRX
        CALL ARCVAL(NARCS, II, JJ, LO, HI, COST, ARTYP,
&          LDARC, SKSC, K, NV, NV, 0, 7)
        NDXAR(K, 1) = NARCS
        ELSE IF (NVS .LT. 0.) THEN
              NV = NVS - APRX
        CALL ARCVAL(NARCS, II, JJ, LO, HI, COST, ARTYP,
&          LDARC, K, SKSC, -NV, -NV, 0, -7)
        NDXAR(K, 1) = -NARCS
        ENDIF
  ENDIF
60  CONTINUE
  IF (.NOT. DEBUG) GO TO 70
  WRITE (IOUT1, 850)
  WRITE (IOUT1, 860)
  DO 63 L = 1, NARCS
        WRITE (IOUT1, 870) L, II(L), JJ(L), LO(L), HI(L), COST(L)
63  CONTINUE
65  LAST = .TRUE.
70  RETURN
850  FORMAT (5X, '--DEBUG FOR NVAR--', ///)
860  FORMAT (10X, 'ARCS', 4X, 'II', 5X, 'JJ', 5X, 'LO BOND',
&          5X, 'HI BOND', 10X, 'COST', 5X, 'FLOW', ///)
870  FORMAT (11X, I2, 5X, I2, 5X, I2, 4X, I10, 3X, I10, 5X, I6, 8X, I2)
  END
*
*=====
* Name:      fx_fil
* Purpose:   Open a fixed flow data file for some arcs.
*=====
SUBROUTINE FX_FIL(NDNAM, LDND, II, JJ, ARTYP, LDARC,
&          PTDWAR, LDPTDW, DWAR, LDDWAR,
&          NFXAR, FXAR, LDFXAR,
&          FXUNIT, UNITNM, LDUNIT, FXFLAG,
&          FILNAM, NTLS, IU, OU,
&          CTERM, COLSTR, LDCOL)
  IMPLICIT NONE
  INTEGER LDND, LDARC, II(LDARC), JJ(LDARC), ARTYP(LDARC)
  INTEGER LDPTDW, LDDWAR, PTDWAR(LDPTDW, 2), DWAR(LDDWAR)
  INTEGER NFXAR, LDFXAR, FXAR(0:2, LDFXAR), FXUNIT, IU, OU
  INTEGER LDUNIT, NTLS
  CHARACTER NDNAM(LDND)*(*), UNITNM(0:LDUNIT)*(*), FILNAM*(*)
  LOGICAL FXFLAG
*
  INTEGER LDCOL
  CHARACTER CTERM(*), COLSTR(LDCOL)*(*)
*
  INTEGER I, J, K, L, ND, ND1, ND2, I1, I2, ISGN
  INTEGER ZONE, ARC, ARC1, TYP, NREC, SL
  LOGICAL ERR, STRM

```

```

      INTEGER      SAVOPT
      COMMON      /SAVOPT/ SAVOPT
*
      FXFLAG = .FALSE.
      IF (FILNAM .EQ. ' ') GOTO 99
      CALL IO_OPFIL(IU, 1, FILNAM, 'ENTER FIXED FLOW FILE: ')
*
*-----Skip title lines
*
      NFXAR = 0
      DO I = 1, NTL5
          READ (IU, *, END = 99)
      ENDDO
*
*-----Flow units
*
      READ (IU, *, END = 99) FXUNIT
*
*-----Read upstream and downstream nodal names
*
      DO I = 1, 2
          READ (IU, '(A)', END = 99) CTERM
          CALL STR_DIVD(CTERM, L, COLSTR, LDCOL, 0, ' ', ' ')
          DO J = 1, L
              CALL NAMNUM(LDND, NDNAM, COLSTR(J), ND, 0, ERR)
              IF (ERR) THEN
                  WRITE(*, 901) COLSTR(J), FILNAM
901      FORMAT( '***ERROR***NODAL NAME: ', A12,
&              ' IN THE FILE: ', A,
&              ', NOT FOUND IN THE NETWORK CONFIGURATION.')
                  STOP !CALL EXIT
              ELSE
                  FXAR(I, J-I+1) = ND
              ENDIF
          ENDDO
          NFXAR = L - 1
      ENDDO
*
*-----Find corresponding arc number.
*
      DO 50 J = 1, NFXAR
          ND1 = FXAR(1, J)
          ND2 = FXAR(2, J)
          I1 = PTDWAR(ND1, 1)
          I2 = PTDWAR(ND1, 2)
          DO 20 I = I1, I2
              ARC = DWAR(I)
          *
*-----Arc type
*
          TYP = ARTYP(ABS(ARC))
          WRITE(CTERM, '(I4)') TYP
          IF (CTERM(2:2) .NE. '1') THEN
              PRINT *, '*** ERROR ** INVALID ARC TYPE'
              GOTO 99
          ENDIF
          IF (CTERM(3:3) .EQ. '1') THEN
              STRM = .TRUE.
          ELSE
              STRM = .FALSE.
          ENDIF
          READ (CTERM(4:4), '(I1)') ZONE
          ZONE = ISGN(TYP) * ZONE
*
*-----Downstream node
*
          ARC1 = ARC
          IF (STRM) THEN
              ARC = ARC + ISGN(ARC)
          ENDIF
          IF (ARC .GT. 0) THEN
              ND = JJ(ARC)
          ELSE
              ND = II(-ARC)
          ENDIF
*
*-----check the current downstream node
*
          IF (ND .EQ. ND2) THEN
              FXAR(0, J) = ARC1
              GOTO 50
          ENDIF
20      CONTINUE
50      CONTINUE
      FXFLAG = .TRUE.
*
*-----output file information
*
      IF (SAVOPT .EQ. 0 .OR. SAVOPT .EQ. 1) THEN
          CALL NORECS(IU, NREC)
          WRITE (OU, 800) FILNAM, UNITNM(FXUNIT), NFXAR, NREC
800      FORMAT (
&          //, 'SUMMARY FOR FIXED-FLOW DATA FILE: ',
&          /, '=====',

```

```

&      /, 12X, \          FILE NAME: \, A
&      /, 12X, \          UNITS: \, A
&      /, 12X, \    NUMBER OF NODES: \, I4,
&      /, 12X, \    NUMBER OF RECORDS: \, I4)
      DO J = 1, NFXAR, 5
      K = J + 4
      IF ((J+4) .GT. NFXAR) THEN
      K = NFXAR
      ELSE
      K = J + 4
      ENDIF
      WRITE (OU, 805) (NDNAM(FXAR(1, I))(1:SL(NDNAM(FXAR(1, I)))),
&      I = J, K)
      WRITE (OU, 806) (NDNAM(FXAR(2, I))(1:SL(NDNAM(FXAR(2, I)))),
&      I = J, K)
      WRITE (OU, 808) (FXAR(0, I), I = J, K)
805      FORMAT ('      UPSTREAM NODAL NAME: \, 5A10)
806      FORMAT ('      DOWNSTREAM NODAL NAME: \, 5A10)
808      FORMAT ('      ARC NUMBER: \, 5I10)
      ENDDO
      ENDIF
99      CONTINUE
      RETURN
      END

```

```

*=====
* Name:      fixflw
* Purpose:    Read fixed flows and assign these flows into arc flow
*             bounds.
*=====
      SUBROUTINE FX_ARC(HI, LO, COST, LDARC, NFXAR, FXAR, LDFXAR,
&      FXUNIT, XF, PERD, IU,
&      RPOOL, LDPL, CTERM, COLSTR, LDCOL)
      IMPLICIT NONE
      INTEGER LDARC, HI(LDARC), LO(LDARC), COST(LDARC)
      INTEGER NFXAR, LDFXAR, FXAR(0:2, LDFXAR), FXUNIT, IU
      REAL XF, PERD
*
      INTEGER LDCOL, LDPL
      REAL RPOOL(LDPL)
      CHARACTER CTERM*(*), COLSTR(LDCOL)*(*)
*
      REAL UC
      INTEGER I, J, ARC
*
      DO I = 1, NFXAR
      RPOOL(I) = 0.0
      ENDDO
      READ (IU, '(A)', END = 99) CTERM
      CALL STR_DIVD(CTERM, I, COLSTR, LDCOL, 0, ',')
      IF (I .NE. (NFXAR + 1)) THEN
      PRINT *, CHAR(7)
      PRINT *, '***ERROR*** FIXED FLOW FILE FOR TIME = ', COLSTR(1)
      STOP !CALL EXIT
      ENDIF
      IF (FXUNIT .EQ. 0) THEN
      UC = 1.0
      ELSE IF (FXUNIT .EQ. 1) THEN
      UC = PERD * 86400.0 / 43560.0
      ELSE IF (FXUNIT .EQ. 2) THEN
      UC = PERD / 43560.0
      ENDIF
*
      DO I = 1, NFXAR
      READ (COLSTR(I+1), '(F15.0)') RPOOL(I)
      ENDDO
*
      DO J = 1, NFXAR
      ARC = FXAR(0, J)
      COST(ARC) = 0
      HI(ARC) = NINT(RPOOL(J) * UC * XF)
      LO(ARC) = NINT(RPOOL(J) * UC * XF)
      ENDDO
99      RETURN
      END

```

```

*=====
* Name:      sabl
* Purpose:    Open files for single arc budget list.
*=====
      SUBROUTINE SABL(NDNAM, LDND, PTDWAR, II, JJ, NDDWAR, LDARC,
&      NSTRM, STRMAR, LDSTRM,
&      NSABL, SABLND, LDSABL, SABLFG,
&      FILNAM)
      IMPLICIT NONE
      INTEGER LDND, LDARC, LDSTRM, LDSABL, NSABL
      PTDWAR(LDND, 2), II(LDARC), JJ(LDARC),
&      NDDWAR(LDARC), NSTRM, STRMAR(LDSTRM, 0:6),
&      SABLND(LDSABL, 3)
      CHARACTER NDNAM(LDND)*(*), FILNAM*(*)
      LOGICAL SABLFG
*      ! SABL -- SINGLE ARC BUDGET LIST.
*
      CHARACTER CTERM*50, UPDW(2)*12, OUTFIL*12
      INTEGER I, J, NL(2), L1, L2, ND, ARC, OU, IU
      LOGICAL ERR

```



```

SNBLFG = .FALSE.
IF (FILNAM .EQ. ' ') GOTO 99
SNBLFG = .TRUE.
IU = 9
OU = 40
NSNBL = 0
CALL IO_OPFIL(IU, 1, FILNAM, 'ENTER SNBL FILE: ')
5 CONTINUE
READ (IU, '(A)', END = 99) NAME
CALL NAMNUM(NNODS, NDNAM, NAME, ND, 0, ERR)
IF (.NOT. ERR) THEN
    NSNBL = NSNBL + 1
*
*-----Open budget output file
*
    OU = OU + 1
    OUTFIL = 'b'//NAME
    CALL STR_LEN(OUTFIL, L)
    OUTFIL = OUTFIL(1:L) // '.out'
    CALL STR_CORS(OUTFIL, 0)
    SNBLND(NSNBL, 1) = ND
    SNBLND(NSNBL, 2) = OU
    CALL IO_OPFIL(OU, 3, OUTFIL, ' ')
    WRITE (OU, 900) NDNAM(ND)
*
*-----Open outflow file for a given node.
*
    OU = OU + 1
    SNBLND(NSNBL, 3) = OU
    OUTFIL = 'r'//NAME
    CALL STR_CORS(OUTFIL, 0)
    CALL STR_LEN(OUTFIL, L)
    OUTFIL = OUTFIL(1:L) // '.out'
    CALL IO_OPFIL(OU, 3, OUTFIL, ' ')
    CALL STR_LEN(NDNAM(ND), L)
    WRITE(OU, 905) NDNAM(ND), ('=', I = 1, 14+L)
    CALL FDWNDS(ND, II, JJ, LDARC, PTDWAR, LDND, NDDWAR, LDDWAR,
& NSTRM, STRMAR, LDSTRM, NDWNDS, DWNDS)
    WRITE(OU, 906) (NDNAM(DWNDS(I))(1:SL(NDNAM(DWNDS(I))))),
& I = 1, NDWNDS)
906 FORMAT(/, 5X, 20(4X, A12, 4X))
    WRITE(OU, '(5x, 10(1x, 19a1))') ((' ', J = 1, 19), I=1, NDWNDS)
    WRITE(OU, '(5x, 10(10x, a10))') (('cubic', I = 1, NDWNDS)
    WRITE(OU, '(5x, 10(a10, a10))')
& (('acre-', 'feet per', I = 1, NDWNDS)
& WRITE(OU, '(a5, 20a10)') 'No.',
& ('feet', 'second', I = 1, NDWNDS)
& WRITE(OU, '(a5, 20a10)') '---',
& ('-----', '-----', I = 1, NDWNDS)
&
& ENDIF
GOTO 5
99 CONTINUE
900 FORMAT(/, T30, 'Water Budgets for ', A
& /, T30, '=====',
& /, T11, 'Initial Upstream Local net',
& 'Evapo- Downstream Final',
& 'Final',
& /, T11, 'storage inflow inflow',
& 'ration Runoff Seepage Withdrawal release storage',
& /, T11, '(acre- (acre- (acre- (acre- (acre- (acre-',
& '(acre- (acre- (acre- (acre- (acre- (acre-',
& 'stage depth',
& /, 'No.', T11, 'feet) feet) feet) feet) feet)',
& '(feet) feet) feet) feet) feet) feet)',
& '(feet) (feet)',
& /, '-----', T11, '-----',
& '-----')
905 FORMAT(/, T5, 'Outflows from ', A, /, T5, 100A)
RETURN
END
*
* Name: fdwns
* Purpose: Find all downstream nodes.
*
SUBROUTINE FDWNDS(ND, II, JJ, LDARC, PTDWAR, LDND, NDDWAR, LDDWAR,
& NSTRM, STRMAR, LDSTRM, NDWNDS, DWNDS)
& IMPLICIT NONE
& INTEGER ND, LDARC, LDND, LDDWAR, LDSTRM,
& II(LDARC), JJ(LDARC),
& PTDWAR(LDND, 2), NDDWAR(LDDWAR),
& NSTRM, STRMAR(LDSTRM, 0:6)
& INTEGER NDWNDS, DWNDS(15)
*
& INTEGER I, J, K, I1, I2, ARC
& LOGICAL CNINT
*
I1 = PTDWAR(ND, 1)
I2 = PTDWAR(ND, 2)
NDWNDS = 0
DO I = 1, 15
    DWNDS(I) = 0
ENDDO
DO I = I1, I2

```

```

                                ARC = NDDWAR(I)
                                IF (ARC .GT. 0) THEN
CALL FDWND(ARC, II, JJ, LDARC, NSTRM, STRMAR, LDSTRM, J)
                                IF (.NOT. CNINT(J, 15, DWNDS, K)) THEN
                                    NDWNS = NDWNS + 1
                                    DWNDS(DNWNS) = J
                                ENDIF
                                ENDIF
ENDDO
RETURN
END

*=====
* Name:          svinfo
* Purpose:       Print the file save information on screen.
*=====
SUBROUTINE SVINFO(FILNAM, LDFIL, NHY, NSNBL, NSABL,
& NDBFLG, ARBFLG, HYBFLG)
  IMPLICIT NONE
  INTEGER LDFIL, NHY, NSNBL, NSABL
  CHARACTER*(*) FILNAM(0:LDFIL)
  LOGICAL NDBFLG, ARBFLG, HYBFLG
  PRINT '(A)', CHAR(7)
  WRITE(*, 901) FILNAM(26)
  IF (NDBFLG) THEN
    WRITE(*, '(5X, A20, A)') FILNAM(27),
    'Nodal budget summary for each time step.'
  &
  ENDIF
  IF (ARBFLG) THEN
    WRITE(*, '(5X, A20, A)') FILNAM(28),
    'Channel routing results.'
  &
  ENDIF
  IF (NHY .GT. 0 .AND. HYBFLG) THEN
    WRITE(*, 902) FILNAM(29)
  ENDIF
  IF (NSNBL .GE. 1) THEN
    WRITE(*, 904) NSNBL, NSNBL
  ENDIF
  IF (NSABL .EQ. 1) THEN
    WRITE(*, 905)
  ELSE IF (NSABL .GT. 1) THEN
    WRITE(*, 906) NSABL
  ENDIF
  RETURN
901 FORMAT(' Summary of output files: ',
& /,
& /, 5X, 'File name', T26, 'Description',
& /, 5X, '-----', T26, '-----',
1 /, 5X, A20, 'General data file summary and network ',
1 'configuration.')
902 FORMAT(
1 5X, A20, 'Flows through outlet structure')
904 FORMAT(' There are ', I3, ' nodal budget files in time series ',
& /, ' format with name convention bndname.out.',
& /, ' There are ', I3, ' outflow files in time series',
& /, ' format with the name convention rndname.out.')
905 FORMAT(
& 5X, 'arbud001.out', T26, 'Channel routing in time series ',
1 'format for a selected arc')
906 FORMAT(
& 5X, 'arbud001.out', T26, 'Channel routing in time series ',
1 'format for selected arcs',
1 /, 5X, ' through',
2 /, 5X, 'arbud', I3.3, '.out')
END

*=====
SUBROUTINE BEEP
PRINT '(A)', CHAR(7)
RETURN
END

*=====
* Name:          io_opfil
* Purpose:       Open a file
*=====
SUBROUTINE IO_OPFIL(UNIT, INOUT, FILNAM, STRING)
  IMPLICIT NONE
  INTEGER UNIT, INOUT, IOS, I, J, LENGTH
  CHARACTER *(*) FILNAM, STRING
  CHARACTER STATS*7, ACCES*10, FIL_NAM2*100
  LOGICAL YES
  IF (LEN(FILNAM) .LE. 100) THEN
    FIL_NAM2 = FILNAM
    LENGTH = 100
  END IF

  *
  * open data file
  *
  STATS = 'UNKNOWN'
  ACCES = 'SEQUENTIAL'
  IF (INOUT .EQ. 1) THEN
    STATS = 'OLD'
  ELSE IF (INOUT .EQ. 2) THEN
    STATS = 'NEW'
  ELSE IF (INOUT .EQ. 4) THEN
    STATS = 'SCRATCH'
  
```



```

ELSE IF (INOUT .EQ. 6) THEN
    ACCES = 'APPEND'
END IF
IF (FIL_NAM2(1:1) .NE. ' ') GO TO 10
2 IF (STRING .NE. ' ') THEN
    PRINT *, STRING
ELSE
    IF (INOUT .EQ. 1) THEN
        PRINT *, 'ENTER INPUT DATA FILE NAME: '
    ELSE IF (INOUT .EQ. 2) THEN
        PRINT *, 'ENTER OUTPUT FILE NAME: '
    ELSE
        PRINT *, 'ENTER SCRATCH FILE NAME: '
    END IF
END IF
READ '(A)', FIL_NAM2
IF (FIL_NAM2(1:1) .EQ. CHAR(27)) THEN
    STOP
ENDIF
DO 3 I = 1, LENGTH
    IF (FIL_NAM2(I:I) .NE. ' ') GO TO 4
3 CONTINUE
PRINT *, '***** ILLEGAL FILE NAME ***** ZERO LENGTH FILE NAME'
GO TO 2
4 CONTINUE
IF ((I-1) .EQ. 0) THEN
    GO TO 10
ELSE
    DO 5 J = 1, LENGTH
        IF (J .LE. (LENGTH-I+1)) THEN
            FIL_NAM2(J:J) = FIL_NAM2(J+I-1:J+I-1)
        ELSE
            FIL_NAM2(J:J) = ' '
        END IF
5 CONTINUE
END IF
10 OPEN (UNIT, FILE = FIL_NAM2, STATUS = STATS, IOSTAT = IOS,
& ACCESS = ACCES)
IF (IOS .NE. 0) THEN
    PRINT *, CHAR(7)
    IF (INOUT .EQ. 1) THEN
        PRINT *, '*****ERROR***** FILE DOES NOT EXIST. FILE: ',
& FIL_NAM2
        PRINT *, ' PLEASE TYPE ANY KEY TO TRY AGAIN OR TYPE CTR-C',
& ' QUIT'
        READ '(A)'
        GO TO 2
    ELSE IF (INOUT .EQ. 2) THEN
        PRINT *, '*****WARNING***** THERE ALREADY EXISTS FILE: ',
& FIL_NAM2
        PRINT *, ' DO YOU WANT TO OVERWRITE IT? (Y) '
        IF (YES()) THEN
            STATS = 'UNKNOWN'
            GO TO 10
        ELSE
            GO TO 2
        END IF
    END IF
END IF
IF (LEN(FILNAM) .GT. 6) THEN
    FILNAM = FIL_NAM2
ENDIF
RETURN
END
=====
* Name:      Yes
* Purpose:   Response yes or no from keyboard.
=====
LOGICAL FUNCTION YES()
IMPLICIT NONE
CHARACTER YESNO*1
YES = .FALSE.
*
YESNO = ' '
5 READ '(A)', YESNO
IF (YESNO .EQ. 'Y' .OR. YESNO .EQ. 'y' .OR. YESNO .EQ. ' ') THEN
    YES = .TRUE.
    GO TO 10
ELSE IF (YESNO .EQ. 'N' .OR. YESNO .EQ. 'n') THEN
    YES = .FALSE.
    GO TO 10
ELSE
    PRINT *, CHAR(7)
    GO TO 5
END IF
10 RETURN
END
=====
* Name:      No
* Purpose:   Response no or yes from keyboard.
=====
LOGICAL FUNCTION NO()
IMPLICIT NONE
CHARACTER YESNO*1

```

```

NO = .FALSE.
*
YESNO = ' '
5 READ '(A)', YESNO
IF (YESNO.EQ. 'N' .OR. YESNO.EQ. 'n' .OR. YESNO.EQ. ' ') THEN
    NO = .TRUE.
    GO TO 10
ELSE IF (YESNO.EQ. 'Y' .OR. YESNO.EQ. 'y') THEN
    NO = .FALSE.
    GO TO 10
ELSE
    PRINT *, CHAR(7)
    GO TO 5
END IF
10 RETURN
END
=====
* Name:      io_rdint
* Purpose:    assign a default value or read an integer value
=====
SUBROUTINE IO_RDINT(String, DEFVAL, VAR)
IMPLICIT NONE
INTEGER DEFVAL, VAR, W, L
CHARACTER STRING(*), TERM*10, FMT * 30
REAL TINY
DATA TINY/1E-3/
*
FMT = '(1X,A,A,I , A)'
GOTO 10
CONTINUE
PRINT *, '***ERROR*** ONLY INTERGER NUMBER IS VALID. TRY AGAIN.'
10 IF (ABS(DEFVAL) .GT. 0) THEN
    W = INT( LOG10(ABS(DEFVAL)+TINY) )+ 1
ELSE
    W = 1
ENDIF
IF (W .GE. 10) THEN
    WRITE(FMT(10:11), '(I2)') W
ELSE
    WRITE(FMT(10:10), '(I1)') W
ENDIF
*
CALL STR_LEN(String, L)
WRITE (*, FMT) STRING(1:L), '(', DEFVAL, ')'
READ '(A)', TERM
IF (TERM.EQ. ' ') THEN
    VAR = DEFVAL
ELSE
    READ (TERM, '(I10)', ERR = 5) VAR
ENDIF
RETURN
END
=====
* Name:      io_rdnnum
* purpose:    Assign a default value or read an real value
=====
SUBROUTINE IO_RDNUM(String, DEFVAL, VAR, D)
IMPLICIT NONE
REAL DEFVAL, VAR
INTEGER W, D
CHARACTER STRING(*), TERM*20, FMT * 30
REAL TINY
DATA TINY/1E-10/
*
FMT = '(1X,A,A,F , A)'
IF (ABS(DEFVAL) .GT. TINY) THEN
    W = INT( LOG10(ABS(DEFVAL)) )
    IF (W .GT. 0) THEN
        W = W + D + 3
    ELSE
        W = 3 + D
    ENDIF
    IF (W .LT. 10) THEN
        WRITE(FMT(10:12), '(I1, A1, I1)') W, '.', D
    ELSE
        WRITE(FMT(10:13), '(I2, A1, I1)') W, '.', D
    ENDIF
ELSE
    WRITE(FMT(10:12), '(I1, A1, I1)') D+3, '.', D
ENDIF
CONTINUE
5 WRITE (*, FMT) STRING, '(', DEFVAL, ')'
READ '(A)', TERM
IF (TERM.EQ. ' ') THEN
    VAR = DEFVAL
ELSE
    READ (TERM, '(F15.0)', ERR = 5) VAR
ENDIF
RETURN
END
=====
* Name:      str_len
* Purpose:    Find the ending position of a character string
=====

```

```

SUBROUTINE STR_LEN(STRING, STRLEN)
IMPLICIT NONE
CHARACTER STRING*(*)
INTEGER STRLEN
*
IF (STRING .EQ. ' ') THEN
    STRLEN = 0
ELSE
    STRLEN = LEN(STRING)
    DO WHILE (STRING(STRLEN:STRLEN) .EQ. ' ')
        STRLEN = STRLEN - 1
    ENDDO
ENDIF
RETURN
END
=====
* Name:      str_divd
* purpose:   Divide a string into substrings with multiple
*            delimiters.
*=====
SUBROUTINE STR_DIVD(STRING, NOCOL, COLSTR, LDCOL, IALIGN, DELIM)
IMPLICIT NONE
INTEGER LDCOL, NOCOL, IALIGN
CHARACTER STRING*(*), COLSTR(LDCOL)*(*), DELIM*(*)
*
CHARACTER DLIM(15)*1
INTEGER STRING_LEN, COLSTR_LEN, I, J, K, L, NODLIM
LOGICAL ISDLIM
*
    Find the no. of delimiters
J = LEN(DELIM)
DO I = J, 1, -1
    IF (DELIM(I:I) .NE. ' ') GOTO 5
ENDDO
I = 1
NODLIM = I
DO I = 1, NODLIM
    DLIM(I) = DELIM(I:I)
ENDDO
*
    Find the main string defined length and output substring length
DO I = 1, LDCOL
    COLSTR(I) = ' '
END DO
STRING_LEN = LEN(STRING)
COLSTR_LEN = LEN(COLSTR(1))
NOCOL = 0
*
    Find the end position of the input main string
I = STRING_LEN
DO WHILE(STRING(I:I) .EQ. ' ')
    I = I - 1
END DO
IF (I .EQ. 1) THEN
    IF (STRING(1:1) .NE. ' ') THEN
        NOCOL = 1
        COLSTR(1) = STRING(1:1)
        ENDIF
        GO TO 99
    ELSE
        STRING_LEN = I
    END IF
    J = 0
*
    Find the beginning position of a substring
10 CONTINUE
    I = J+1
11 CONTINUE
    IF (I .GT. STRING_LEN) GO TO 99
    DO L = 1, NODLIM
        IF (STRING(I:I) .EQ. DLIM(L)) THEN
            I = I + 1
            GOTO 11
        ENDIF
    ENDDO
    do while(string(i:i) .eq. ' ')
        i = i + 1
    enddo
*
    Find end position of a substring
J = I + 1
NOCOL = NOCOL + 1
DO WHILE (J .LE. STRING_LEN)
    IF (ISDLIM(STRING(J:J), NODLIM, DLIM)) THEN
        GOTO 20
    ENDIF
    J = J + 1
ENDDO
20 J = J - 1

```

```

*
*          assign the substring to an associated array
*
COLSTR(NOCOL) = ' '
IF (IALIGN .EQ. 1) THEN
      K = J - I + 1
      K = (COLSTR_LEN - K) / 2
      IF (K .EQ. 0) K = 1
      COLSTR(NOCOL)(K:(J-I)+K) = STRING(I:J)
ELSE IF (IALIGN .EQ. 2) THEN
      K = COLSTR_LEN - (J - I)
      COLSTR(NOCOL)(K:COLSTR_LEN) = STRING(I:J)
ELSE
      COLSTR(NOCOL) = STRING(I:J)
ENDIF
J = J + 1
GO TO 10
99 CONTINUE
RETURN
END
LOGICAL FUNCTION ISDLIM(CH, NDLIM, DLIM)
IMPLICIT NONE
INTEGER NDLIM
CHARACTER*1 CH, DLIM(NDLIM)
*
*   INTEGER I
*
ISDLIM = .FALSE.
DO I = 1, NDLIM
      IF (CH .EQ. DLIM(I)) THEN
            ISDLIM = .TRUE.
            RETURN
      ENDIF
ENDDO
RETURN
END
=====
* Name:      io_rdstr
* Purpose:    Assign a default string or read a new string
*=====
SUBROUTINE IO_RDSTR(STRING, VAR)
IMPLICIT NONE
CHARACTER STRING*(*), VAR*(*), TERM*150
INTEGER SL, VL, TL
*
CALL STR_LEN(STRING, SL)
IF (VAR .EQ. ' ') THEN
      PRINT *, STRING(1:SL)
ELSE
      CALL STR_LEN(VAR, VL)
      PRINT *, STRING(1:SL), ' (' , VAR(1:VL), ' )'
ENDIF
READ '(A)', TERM
IF (TERM .NE. ' ') THEN
      CALL STR_LEN(TERM, TL)
      VAR = TERM(1:TL)
ENDIF
RETURN
END
=====
* Name:      cn
* Purpose:    Check whether a substring is contained in a string
*=====
LOGICAL FUNCTION CN(STRING, SUBSTR, CODE)
IMPLICIT NONE
CHARACTER STRING*(*), SUBSTR*(*)
INTEGER CODE
INTEGER J1, J2, K1, K2
*
CHARACTER RTERM * 150, KEY * 150
*
CN = .FALSE.
RTERM = STRING
KEY = SUBSTR
IF (CODE .NE. 0) THEN
      CALL STR_CORS(RTERM, 0)
      CALL STR_CORS(KEY, 0)
ENDIF
*
CALL STR_POS(RTERM, J1, J2)
CALL STR_POS(KEY, K1, K2)
IF (KEY(K1:K2) .EQ. RTERM(J1:J2)) THEN
      CN = .TRUE.
ENDIF
RETURN
END
=====
* Name:      str_pos
* Purpose:    Find the beginning and end position of a string
*=====
SUBROUTINE STR_POS(STRING, IPT1, IPT2)
IMPLICIT NONE
CHARACTER * (*) STRING
INTEGER IPT1, IPT2

```

```

      INTEGER I
      IPT2 = LEN(STRING)
      DO I = IPT2, 1, -1
          IF (STRING(I:I) .NE. ' ') GO TO 5
      END DO
      IPT2 = I
      DO I = 1, IPT2
          IF (STRING(I:I) .NE. ' ') GO TO 10
      END DO
      IPT1 = I
      RETURN
      END
=====
* Name:      str_cors
* Purpose:   convert a string into capital latter or small latters.
=====
      SUBROUTINE STR_CORS(CLINE, ICTR)
      IMPLICIT NONE
      CHARACTER CLINE*(*)
      INTEGER    ICTR, CODE, L, I

      CALL STR_LEN(CLINE, L)
      DO I = 1, L
          CODE = ICHAR(CLINE(I:I))
          IF (ICTR .EQ. 0) THEN
              IF ( (CODE .GE. 65) .AND. (CODE .LE. 90) ) THEN
                  CLINE(I:I) = CHAR(CODE+32)
              ELSEIF (ICTR .EQ. 1) THEN
                  IF ( (CODE .GE. 97) .AND. (CODE .LE. 122) ) THEN
                      CLINE(I:I) = CHAR(CODE-32)
                  ENDIF
              ENDIF
          ENDIF
      END DO
      RETURN
      END
=====
* Name:      str_no
* purpose:   Return the number of substrings with multiple delimiters
=====
      SUBROUTINE STR_NO(STRING, NOSUB, DELIM)
      IMPLICIT NONE
      INTEGER NOSUB
      CHARACTER*(*) STRING, DELIM
      CHARACTER * 1 DLIM(15)
      INTEGER STRING_LEN, I, J, L, NODLIM

      *
      *      Initialize
      *
      NOSUB = 0

      *
      *      Find the no. of delimiters
      *
      J = LEN(DELIM)
      DO I = J, 1, -1
          IF (DELIM(I:I) .NE. ' ') GOTO 5
      END DO
      I = 1
      5 NODLIM = I
      DO I = 1, NODLIM
          DLIM(I) = DELIM(I:I)
      END DO

      *
      *      Find the defined length for main string
      *
      STRING_LEN = LEN(STRING)

      *
      *      Find the true length of the input main string
      *
      I = STRING_LEN
      DO WHILE(STRING(I:I) .EQ. ' ')
          I = I - 1
      END DO
      IF (I .EQ. 1) THEN
          IF (STRING(1:1) .NE. ' ') THEN
              NOSUB = 1
          ELSE
              NOSUB = 0
          ENDIF
          GO TO 99
      ELSE
          STRING_LEN = I
      END IF
      J = 0

      10 CONTINUE

      *
      *      find the beginning position of a substring
      *
      I = J+1
      15 CONTINUE
      IF (I .GT. STRING_LEN) GO TO 99
      IF (STRING(I:I) .EQ. ' ') THEN
          I = I + 1

```

```

                                GOTO 15
ELSE
                                DO L = 1, NODLIM
                                IF (STRING(I:I) .EQ. DLIM(L)) THEN
                                    I = I + 1
                                    GOTO 15
                                ENDDO
                                ENDDO
ENDIF
*
*           find end position of a substring
*
J = I + 1
NOSUB = NOSUB + 1
DO WHILE (J .LE. STRING_LEN)
                                DO L = 1, NODLIM
                                IF (STRING(J:J) .EQ. DLIM(L)) GOTO 10
                                ENDDO
                                J = J + 1
ENDDO
99 RETURN
END
=====
*
FUNCTION ISGN (NUM)
IMPLICIT NONE
INTEGER ISGN, NUM
*
IF (NUM .GT. 0) THEN
                                ISGN = 1
ELSE IF (NUM .LT. 0) THEN
                                ISGN = -1
ELSE
                                ISGN = 0
ENDIF
RETURN
END
=====
* Name:      exist
* Purpose:    check file status.
=====
*
LOGICAL FUNCTION EXIST(FILNAM)
IMPLICIT NONE
CHARACTER FILNAM*(*)
INQUIRE (FILE = FILNAM, EXIST = EXIST)
RETURN
END
=====
*
FUNCTION SL(STRING)
IMPLICIT NONE
CHARACTER STRING*(*)
INTEGER SL
*
IF (STRING .EQ. ' ') THEN
                                SL = 0
ELSE
                                SL = LEN(STRING)
                                DO WHILE (STRING(SL:SL) .EQ. ' ')
                                    SL = SL - 1
                                ENDDO
ENDIF
RETURN
END
=====
* Name:      hydr_dat
* Purpose:    Read hydraulic structure data.
=====
*
SUBROUTINE HYDR_DAT(NNODS, NDNAM, LDND, JJ, LDARC,
&                  PTDWAR, LDPTDW, DWAR, LDDWAR,
&                  NSTRM, STRMAR, LDSTRM,
&                  NHYTP, HYTP, LDHYTP,
&                  NHY, HYDIR, HYTPCD, HYDAT, LDHY,
&                  FILNAM, NTLS, IN, OU, PARTNO,
&                  CTERM, COLSTR, LDCOL)
IMPLICIT NONE
INTEGER NNODS, LDND, LDARC, JJ(LDARC)
CHARACTER NDNAM(LDND)*(*)
INTEGER LDPTDW, PTDWAR(LDPTDW, 2), LDDWAR, DWAR(LDDWAR)
INTEGER NSTRM, LDSTRM, STRMAR(LDSTRM, 0:6)
INTEGER NHY, LDHY
CHARACTER HYDIR(LDHY, 0:2)*(*)
INTEGER HYTPCD(LDHY, 0:1)
REAL HYDAT(LDHY, 5)
CHARACTER*(*) FILNAM
INTEGER NTLS, IN, OU, PARTNO
*
INTEGER LDCOL
CHARACTER CTERM*(*), COLSTR(LDCOL)*(*)
*
INTEGER I, J, N, K, IU, LIM1, LIM2, ND, ND2, UPND, DWND, ARC
LOGICAL ERR, CN, FLAG
*
INTEGER NHYTP, LDHYTP
CHARACTER HYTP(0:LDHYTP)*(*)
INTEGER SAVOPT

```

```

COMMON      /SAVOPT/ SAVOPT
*
*-----Assign hydraulic types
*
      NHYTP = 6
      HYTP(0) = ' '
      HYTP(1) = 'Sharp-crested weir'
      HYTP(2) = 'Gate spillway'
      HYTP(3) = 'Sluice gate'
      HYTP(6) = 'Pipe'
*
      IF (FILNAM .NE. ' ') THEN
          FLAG = .TRUE.
          IU = 9
          CALL IO_OPFIL(IU, 1, FILNAM, 'ENTER STRUCTURE FILE: ')
          DO I = 1, NTL5
              READ (IU, *, END = 100)
          ENDDO
      ELSE
          FLAG = .FALSE.
          IU = IN
          CTERM = ' '
          DO WHILE (CTERM .EQ. ' ')
              READ (IU, '(A)', END = 100) CTERM
          ENDDO
          CALL STR_DIVD(CTERM, I, COLSTR, LDCOL, 0, ' ', ' ')
          READ (COLSTR(2), '(I2)') N
          IF (CN(COLSTR(1), 'PART', 1) .AND. N .EQ. PARTNO) THEN
              READ (IU, *) 'SKIP THE VARIABLE LIST'
          ELSE
              IF (CN(COLSTR(1), 'PART', 1) .AND. N .GT. PARTNO) THEN
                  BACKSPACE(1)
                  GOTO 100
              ENDIF
              PRINT *, CHAR(7)
              PRINT *, '***ERROR***ERROR IN STRUCTURE DATA'
              STOP 'CALL EXIT'
              ENDIF
          ENDIF
      ENDIF
*
*-----Output title
*
      IF (SAVOPT .EQ. 0 .OR. SAVOPT .EQ. 1) THEN
          WRITE(OU, 800)
800      FORMAT(//, 'Part 4: Hydraulic-structure data',
&          /, '=====',
&          /, T57,
&          /, 'Weir      Weir      ',
&          /, T57,
&          /, 'width    height    ',
&          /, ' ',
&          /, 'Base    or pipe or pipe    Pipe-    Pipe-    ', T57,
&          /, 'Structure F-node T-node    Structure', T57,
&          /, 'elevation diameter length friction entry loss',
&          /, 'name      name      name      type', T57,
&          /, '(feet)  (feet)  (feet)  factor    factor',
&          /, '-----', T57,
&          /, '-----')
      ENDIF
*
*-----Read data
*
      NHY = 0
      CONTINUE
      READ (IU, '(A)', END = 100) CTERM
      CALL STR_DIVD(CTERM, K, COLSTR, LDCOL, 0, ' ', ' ')
      IF (CN(COLSTR(1), 'FINISH', 1)) GOTO 100
      IF (K .LT. 6) GOTO 999
      NHY = NHY + 1
      DO J = 1, K
          IF (J .LE. 3) THEN
              READ(COLSTR(J), '(A)') HYDIR(NHY, J-1)
          ELSE IF (J .EQ. 4) THEN
              READ (COLSTR(J), '(I15)') HYTPCD(NHY, 0)
          ELSE
              READ(COLSTR(J), '(F15.0)') HYDAT(NHY, J-4)
          ENDIF
      ENDDO
      IF (SAVOPT .EQ. 0 .OR. SAVOPT .EQ. 1) THEN
          WRITE (OU, 805) (HYDIR(NHY, J), J = 0, 2),
&          HYTP(HYTPCD(NHY, 0)),
&          (HYDAT(NHY, J), J = 1, 5)
805      FORMAT(3A12, A20, 3F10.2, 2F10.5)
      ENDIF
      GOTO 5
100      CONTINUE
      IF (FLAG) CLOSE(IU)
*
*      Search the downstream arc for normal flow range.
*
      DO 30 I = 1, NHY
          CALL NAMNUM(LDND, NDNAM, HYDIR(I,1), UPND, 0, ERR)
          CALL NAMNUM(LDND, NDNAM, HYDIR(I,2), DWND, 0, ERR)
          LIM1 = PTDWAR(UPND, 1)

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```

        LIM2 = PTDWAR(UPND, 2)
        DO J = LIM1, LIM2
            ARC = IABS(DWAR(J))
            ND = JJ(ARC)
            IF (ND .GT. NNODS) THEN !THE CURRENT ARC IS A STREAM ARC.
                DO K = 1, NSTRM
                    IF (ND .EQ. STRMAR(K, 0)) THEN
                        ND2 = JJ( IABS(STRMAR(K, 6)))
                        IF (ND2 .EQ. DWND) THEN
                            HYTPCD(I, 1) = ARC
                            GOTO 30
                        ENDIF
                    ENDIF
                ENDDO
            ELSE IF (ND .EQ. DWND) THEN
                HYTPCD(I, 1) = ARC
                GOTO 30
            ENDIF
        ENDDO
30    CONTINUE
999  RETURN
    END
=====
* Name:      hydr_hite
* Purpose:   Calculate gate opening height or sharp-crested weir
*            height for a given discharge.
=====
      SUBROUTINE HYDR_HITE(
&          ARCBUD, LDARC,
&
&          NHY, HYTPCD, HYDAT, HYOUT, LDHY,
&          PERD, CONST, XF)
      IMPLICIT NONE
      INTEGER LDARC, ARCBUD(LDARC, 0:5)
      INTEGER NHY, LDHY, HYTPCD(LDHY, 0:1)
      REAL HYDAT(LDHY, 2), HYOUT(LDHY, 3)
      REAL PERD, CONST, XF
*
      REAL FLW, W, B, P, H1, HD, E, EPS
      INTEGER N, TYP, ARC
      INTEGER IFAULT
      DATA EPS/1.0E-10/
*
      LOGICAL CHECK
      COMMON /CHECK/ CHECK
*
      DO 100 N = 1, NHY
          TYP = HYTPCD(N,0)
          ARC = HYTPCD(N,1)
*
          IF (TYP .LE. 0) THEN
              GOTO 100
*
          -----Sharp-crested weir with fixed weir height,
          It is no needed to calculate the weir height.
          ELSE IF (TYP .EQ. 1 .AND. HYDAT(N, 3) .GT. 0) THEN
              GOTO 100
*
          -----Fixed gate opening height, no need to calculate
          the gate opening height.
          ELSE IF ((TYP .GE. 2 .AND. TYP .LE. 3) .AND.
&              HYDAT(N,5) .GT. 0) THEN
              GOTO 100
          ENDIF
*
          -----Find downstream flows
          FLW = ARCBUD(ARC, 1) / PERD / CONST / XF !RELEASE RATE IN CFS
          HYOUT(N,1) = FLW
*
          -----Calculate gate opening height or sharp-crested weir height
          H1 = HYOUT(N, 2) - HYDAT(N, 1)
          B = HYDAT(N, 2)
          W = HYDAT(N, 4)
*
          IF (ABS(FLW) .LT. EPS) THEN
              CONTINUE
          ELSE IF (TYP .EQ. 1) THEN
              CALL WEIRHITE(FLW, H1, 0.0, B, P, W, 0.0, TYP, IFAULT)
              HYOUT(N,3) = P
          ELSE IF (TYP .EQ. 2 .OR. TYP .EQ. 3) THEN
              HD = HYDAT(N, 3)
              CALL GATEHITE(FLW, H1, 0.0, B, E, HD, TYP, IFAULT)
              HYOUT(N,3) = E
          ENDIF
      100 CONTINUE
      RETURN
      END
=====
* Name:      hydr_prn

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```

* Purpose:      Print the results for flow through structure.
*=====
SUBROUTINE HYDR_PRN(HYTP, LDHYTP,
&                NHY, HYDIR, HYTPCD, HYDAT, HYOUT, LDHY, NOP,
&                HYBFLG, IOUT)
  IMPLICIT      NONE
  INTEGER      LDHYTP, NHY, LDHY
  CHARACTER    HYTP(0:LDHYTP)*(*), HYDIR(LDHY, 0:2)*(*)
  INTEGER      HYTPCD(LDHY, 0:1)
  REAL         HYDAT(LDHY, 2), HYOUT(LDHY, 3)
  INTEGER      NOP, IOUT
  LOGICAL      HYBFLG

  INTEGER      J, N

  IF (HYBFLG) THEN
        WRITE (IOUT, 900) NOP
        DO N = 1, NHY
        WRITE (IOUT, 901) (HYDIR(N, J), J = 0, 2),
&                HYTP(HYTPCD(N, 0)),
&                (HYDAT(N, J), J = 1, 2),
&                (HYOUT(N, J), J = 1, 3)
        ENDDO
  ENDIF
  RETURN
900  FORMAT(/, 20X, 'Parameters for hydraulic structures: ', I3,
&        /, 20X, '-----',
&        /, 20X, ' [-999.99, not flow under gate]',
&        /, T61, 'Discharge Upstream Gate-opening',
&        /, 'Upstream Downstream',
&        /, T61, 'Base Weir (cubic Water or weir ',
&        /, 'Structure node node', T41, 'Structure',
&        /, T61, 'elevation length feet per elevation height ',
&        /, 'name name name', T41, 'type',
&        /, T61, '(feet) (feet) second (feet) (feet)',
&        /, '-----', T41, '-----',
&        /, T61, '-----')
901  FORMAT(3A12, T41, A20, T61, 5F10.2)
END
*=====
* Name:      hydr_ofw
* Purpose:   Calculate overflow on weir or through pipe. Flows are dependent
*           on ponds stages, and structure types and materials.
*           In other words, flows through structures can not be
*           controlled.
*=====
SUBROUTINE HYDR_OFW(NDSEQ, LDND,
&                II, HI, LO, OHI, LDARC,
&                NHY, HYTPCD, HYDAT, HYOUT, LDHY,
&                OINST, LDRES,
&                PERD, CONST, XF, OU)
  IMPLICIT      NONE
  INTEGER      LDND, NDSEQ(LDND),
&                LDARC, II(LDARC), HI(LDARC), LO(LDARC)
  INTEGER      OHI(LDARC)
  INTEGER      NHY, LDHY, HYTPCD(LDHY, 0:1)
  REAL         HYDAT(LDHY, 5), HYOUT(LDHY, 3)
  INTEGER      LDRES
  REAL         OINST(LDRES), PERD, CONST, XF
  INTEGER      OU

  REAL         B, W, P, H1, ZVA(3), D, L, F, E, LOFLW, HIFLW, HD
  INTEGER      N, TYP, ARC, ND, RES, IFAULT
  LOGICAL      GETZVA, CHECK
  CHARACTER    NDNAM(300)*12
  COMMON      /NDNAME/NDNAM

  DO 100 N = 1, NHY
        TYP = HYTPCD(N, 0)
        ARC = HYTPCD(N, 1)
        ND = II(ARC)

*-----Calculate upstream water depth
*
        RES = NDSEQ(ND)
        ZVA(2) = OINST(RES)
        IF (GETZVA(ND, 2, ZVA, OU)) THEN
                H1 = ZVA(1)
        ELSE
                PRINT *, CHAR(7)
                PRINT *, '***ERROR*** CONVERTING STORAGE: ', ZVA(2)
                PRINT *, ' INTO ITS ELEVATION FOR RESERVOIR: ', RES
                STOP !CALL EXIT
        ENDIF
        HYOUT(N, 2) = H1
        H1 = HYOUT(N, 2) - HYDAT(N, 1)

*-----Calculate the flow through the structure or the
*           maximum flow through the structures.
*
        IF (H1 .LE. 0.0) THEN

*           The water head is lower than the base elevation

```

```

                                LOFLW = 0.0
                                HIFLW = 0.0
ELSE IF (TYP .EQ. 0) THEN
                                LOFLW = 0.0
                                HIFLW = OHI(ARC)
ELSE IF (TYP .EQ. 1) THEN
*-----1. Flow over sharp-crested weir
*
                                B = HYDAT(N, 2)          !WEIR LENGTH.
                                P = HYDAT(N, 3)          !WEIR HEIGHT.
                                IF (P .LT. 0.0) THEN
*
*       The weir height is to be determined. It is assumed that
*       for given upstream water depth, the maximum flow over weir
*       occurs when there is no weir, i.e. p = 0.
*
                                IF (ABS(P) .GE. H1) THEN
                                    LOFLW = 0.0
                                    ELSE
CALL WEIRFLW(LOFLW, H1+P, 0.0, B, -P, 0.0, 0.0, TYP,
&                                IFAULT)
                                    ENDIF
CALL WEIRFLW(HIFLW, H1, 0.0, B, 0.0, 0.0, 0.0, TYP,
&                                IFAULT)
                                    ELSE IF (H1 .LT. P) THEN
*
*       If the water level is lower than the top of the weir, no
*       flow is over the weir.
*
                                LOFLW = 0.0
                                HIFLW = 0.0
                                ELSE
*
*       The weir height is fixed. The flow over the weir is
*       determined from the upstream level and weir height.
*       Free outfall flow is assumed.
*
CALL WEIRFLW(LOFLW, H1-P, 0.0, B, P, 0.0, 0.0, TYP,
&                                IFAULT)
                                HIFLW = LOFLW
                                IF (IFAILT .NE. 0) THEN
CALL PRINT *, '***Error*** Calculation the flow over the'//
&                                'sharp-crested weir: ', N
                                STOP
                                ENDIF
                                HYOUT(N, 3) = P
                                ENDIF
ELSE IF (TYP .EQ. 2 .OR. TYP .EQ. 3) THEN
*-----2. Flow under gates on spillway or broad-crested weir
*
                                B = HYDAT(N, 2)          !WEIR LENGTH
                                P = HYDAT(N, 3)          !WEIR HEIGHT
                                W = HYDAT(N, 4)          !WEIR THICKNESS
                                E = HYDAT(N, 5)          !GATE OPENING HEIGHT
                                IF (TYP .EQ. 2) THEN
                                    HD = HYDAT(N, 3)      !DESIGN WATER HEAD FOR SPILLWAY
                                    ENDIF
                                IF (E .GT. 0) THEN
CALL GATEFLW(LOFLW, H1, 0.0, B, E, HD, TYP, IFAULT)
                                IF (IFAILT .EQ. 1) THEN !GATE OPENING TOO HIGH, WEIR FLOW.
CALL WEIRFLW(LOFLW, H1, 0.0, B, P, W, HD, TYP, IFAULT)
                                ELSE IF (IFAILT .EQ. 2) THEN
CALL PRINT *, '***Error*** Invalid gate type for structure'
&                                '//' no: ', N
                                STOP
                                ENDIF
                                HIFLW = LOFLW
                                HYOUT(N, 3) = E
                                ELSE
*
*-----The maximum flow under gate on spillway or broad-crested
*       weir is the same as the flow over weir without gates under
*       the same water head.
*
                                LOFLW = 0.0
CALL WEIRFLW(HIFLW, H1, 0.0, B, P, W, HD, TYP, IFAULT)
                                ENDIF
                                ELSE IF (TYP .EQ. 6) THEN
*-----Flow through a short pipe
*
                                D = HYDAT(N, 2)
                                L = HYDAT(N, 3)
                                F = HYDAT(N, 4)
                                E = HYDAT(N, 5)
CALL PIPE_FLW(0, LOFLW, H1, D, L, F, E, 0.0)
                                HIFLW = LOFLW
                                ENDIF
                                HYOUT(N, 1) = LOFLW          !IN CFS
                                LO(ARC) = NINT(LOFLW * PERD * CONST * XF)
                                HI(ARC) = NINT(HIFLW * PERD * CONST * XF)
*

```

```

                                CHECK = .FALSE.
100  CONTINUE
    RETURN
    END
=====
* Name:      resev_dat
* Purpose:   Get seasonal reservoir surface water evaporation coefficient.
=====
SUBROUTINE RESEV_DAT(NNODS, NDNAM, LDND, NEV, EVND, LDEV,
&                  NPER, EVTB, LDEVTB, EVUNIT, EVFLAG,
&                  FILNAM, NTLN, IN, OU, PN,
&                  UNITNM, LDUNIT, CTERM, COLSTR, LDCOL)
    IMPLICIT NONE
    INTEGER NNODS, LDND, LDEVTB
    CHARACTER*(*) NDNAM(LDND), FILNAM
    INTEGER NEV, LDEV, EVND(LDEV, 3), NPER, EVUNIT
    INTEGER NTLN, IN, IU, OU, PN
    REAL EVTB(0:LDEVTB, LDEV)
    LOGICAL EVFLAG
    INTEGER LDUNIT, LDCOL
    CHARACTER UNITNM(0:LDUNIT)*(*), CTERM*(*), COLSTR(LDCOL)*(*)

    INTEGER I, NREC
    LOGICAL FLAG, ENDFIL

    EVFLAG = .FALSE.

*-----Open data file and skip title lines
    IF (FILNAM .NE. ' ') THEN
        FLAG = .TRUE.
        IU = 9
        CALL IO_OPFIL(IU, 1, FILNAM, 'ENTER SEASONAL EV DATA FILE: ')
        DO I = 1, NTLN
            !SKIP TITLE LINES
            READ (IU, *, END = 99)
        ENDDO
    ELSE
        FLAG = .FALSE.
        IU = IN
        CALL PNCK(PN, IU, ENDFIL, CTERM, COLSTR, LDCOL)
        IF (ENDFIL) GOTO 99
    ENDIF

*-----Read units, nonal names, and data from a file and
* print these information into general output file.
    CALL DATTB(NNODS, NDNAM, LDND, NEV, EVND, LDEV, EVUNIT,
&            NREC, EVTB, LDEVTB, EVFLAG,
&            UNITNM, LDUNIT, FILNAM, IU, OU, PN, ENDFIL,
&            'water-surface evaporation coefficients',
&            CTERM, COLSTR, LDCOL)
    IF (NREC .LT. NPER) THEN
        WRITE (OU, 805) NPER, NREC
805    FORMAT('***WARNING*** NO. OF SEASONAL RECORDS ARE LESS',
&          ' THAN NO. OF SEASONS.',
&          ' NO. OF RECORDS: ', I3,
&          ' NO. OF SEASONS: ', I3)
    ENDIF
99    CONTINUE
    IF (FLAG) CLOSE(IU)
    RETURN
    END
=====
* Name:      resev_arc
* Purpose:   Read current evaporation coefficients and create
*            corresponding EV arcs.
=====
SUBROUTINE RESEV_ARC(NDTYP, NDSEQ, LDND,
&                  NARC, II, JJ, LO, HI, COST, ARTYP, LDARC,
&                  INST, LDRES,
&                  NDXAR, LDXAR, NEV, EVND, LDEV, EVTB, LDEVTB,
&                  MTH, SKSC, PERD, EVTIM, XF, IU,
&                  CTERM, COLSTR, LDCOL)
    IMPLICIT NONE
    INTEGER LDND, NDTYP(LDND), NDSEQ(LDND)
    INTEGER NARC, LDARC,
&          II(LDARC), JJ(LDARC), LO(LDARC), HI(LDARC),
&          COST(LDARC), ARTYP(LDARC)
    INTEGER LDRES
    REAL INST(LDRES)
    INTEGER LDXAR, NDXAR(LDXAR, 6)
    INTEGER NEV, LDEV, EVND(LDEV, 3), LDEVTB
    REAL EVTB(0:LDEVTB, LDEV), PERD, XF
    INTEGER IU, MTH, SKSC
    LOGICAL EVTIM

    INTEGER LDCOL
    CHARACTER CTERM*(*), COLSTR(LDCOL)*(*)

    INTEGER I, J, N, ND, NNDS, RES, NV
    REAL ZVA(3), UC(0:2), RTERM
    LOGICAL GETZVA, TDFLAG

*-----Unit conversion factor (day * ft)

```

```

*
UC(0) = PERD / 304.8
UC(1) = PERD / 12.0
UC(2) = PERD
*
*-----Read current surface water evaporation coefficient
*
TDFLAG = .FALSE.
IF (EVTIM) THEN
    READ (IU, '(A)', END = 50) CTERM
    CALL STR_DIVD(CTERM, NNDS, COLSTR, LDCOL, 0, ' ', ' ')
    TDFLAG = .TRUE.
ENDIF
50 CONTINUE
*
*-----Calculate the reservoir surface evaporation
*
DO I = 1, NEV
    ND = EVND(I,1)
    IF (EVND(I,3) .EQ. 0) THEN
        N = MTH
    ELSE
        N = 0
    IF (TDFLAG) THEN
        IF (EVND(I, 3) .LT. 0) THEN
            READ (COLSTR(NNDS), '(F10.0)') EVTB(0,I)
        ELSE
            J = EVND(I,3) + 1
            READ (COLSTR(J), '(F10.0)') EVTB(0,I)
        ENDIF
    ELSE
        EVTB(0,I) = 0.0
    ENDIF
    ENDIF
    IF (NDTYP(ND) .EQ. 1) THEN
        RES = NDSEQ(ND)
        ZVA(2) = INST(RES)
        IF (GETZVA(ND, 2, ZVA, 26)) THEN
            RTERM = EVTB(N,I) * UC(EVND(I,2)) * ZVA(3)
        *
        * If the available water is less than the calculated evaporation,
        * the evaporation is set to equal to one half of the available water
        *
        IF (RTERM .GT. ZVA(2)) THEN
            NV = NINT(ZVA(2) * 0.5 * XF)
        ELSE
            NV = NINT(RTERM * XF)
        ENDIF
    *
    CALL ARCVAL(NARC, II, JJ, LO, HI, COST, ARTYP,
    & LDARC, ND, SKSC, NV, NV, 0, 4)
    NDXAR(ND, 2) = NARC
    ELSE
        PRINT *, '***Error*** Error in transforming water volume'
        PRINT *, ' to its corresponding elevation and area.'
        PRINT *, ' Pond: ', ND, ' Water volume: ', ZVA(2)
        STOP
    ENDIF
    ENDIF
99 ENDDO
RETURN
END
*=====
* Name:      rnof_dat
* Purpose:   Read data for surface runoff.
*=====
SUBROUTINE RNOF_DAT(NDNAM, LDND,
& NRNOF, RNOFND, LDRNOF, RNOFTB, LDRFTB, A5DR,
& FILNAM, NTLN, IN, OU, PN,
& CTERM, COLSTR, LDCOL)
    IMPLICIT NONE
    INTEGER LDND
    CHARACTER NDNAM(LDND)*(*)
    INTEGER NRNOF, LDRNOF, RNOFND(LDRNOF), LDRFTB
    REAL RNOFTB(LDRNOF, 0:LDRFTB), A5DR(LDRNOF, 5)
    CHARACTER FILNAM*(*)
    INTEGER NTLN, IN, OU, PN
*
    INTEGER LDCOL
    CHARACTER CTERM*(*), COLSTR(LDCOL)*(*)
*
    INTEGER I, J, K, IU
    LOGICAL ERR, CN, FLAG, ENDFIL
    INTEGER SAVOPT
    COMMON /SAVOPT/ SAVOPT
*
    IF (FILNAM .NE. ' ') THEN
        FLAG = .TRUE.
        IU = 9
        CALL IO_OPFIL(IU, 1, FILNAM, 'ENTER RUNOFF DATA FILE: ')
        DO I = 1, NTLN
            READ (IU, *, END = 100)
        ENDDO
    ELSE

```

```

                                FLAG = .FALSE.
                                IU = IN
CALL PNCK(PN, IU, ENDFIL, CTERM, COLSTR, LDCOL)
                                IF (ENDFIL) GOTO 99
                                READ (IU, *) ! SKIP THE VARIABLE LIST LINE
ENDIF
*
*-----Output title
*
      IF (SAVOPT .EQ. 0 .OR. SAVOPT .EQ. 1) THEN
        WRITE(OU, 800) PN
800    FORMAT(//, 'Part', I2, ': Surface-runoff data',
& //, T13, '-----',
& //, T11, 'Initial criterion 5-day rainfall',
& //, T13, '-----',
& //, T13, 'Antecedent      Dry      Wet      SCS',
& //, T11, 'Drainage',
& //, T11, '5-day rainfall condition condition curve',
& //, T13, 'area',
& //, 'Name',
& //, T13, '(inches) (inches) (inches) number',
& //, T13, '(acres)',
& //, T13, '-----',
& //, T13, '-----')
      ENDIF
*
*-----Read data
*
      NRNOF = 0
5      CONTINUE
      READ (IU, '(A)', END = 100) CTERM
      CALL STR_DIVD(CTERM, K, COLSTR, LDCOL, 0, ' ')
      IF ((K-2) .GT. LDRFTB) THEN
        PRINT '(A)', CHAR(7)
        PRINT *, '***ERROR*** THE DIMENSION OF RUNOFF TABLE IS NOT BIG'
& // ' ENOUGH'
        STOP !CALL EXIT
      ENDIF
      IF (CN(COLSTR(1), 'FINISH', 1)) GOTO 100
      NRNOF = NRNOF + 1
      DO J = 1, K
        IF (J .EQ. 1) THEN
          CALL NAMNUM(LDND, NDNAM, COLSTR(1), RNOFND(NRNOF), 0, ERR)
        ELSE
          READ (COLSTR(J), '(F15.0)') RNOFTB(NRNOF, J-2)
        ENDIF
      ENDDO
*
*-----Assign intial 5-day antecedent rainfall (daily).
*
      DO J = 1, 5
        A5DR(NRNOF, J) = RNOFTB(NRNOF, 0) / 5.0
      ENDDO
      GOTO 5
100    CONTINUE
      IF (FLAG) CLOSE(IU)
*
*-----Print the data
*
      IF (SAVOPT .EQ. 0 .OR. SAVOPT .EQ. 1) THEN
        DO I = 1, NRNOF
          WRITE(OU, 901) NDNAM(RNOFND(I)), (RNOFTB(I,J), J=0,LDRFTB)
901    FORMAT(A12, 10F12.2)
        ENDDO
      ENDIF
99    RETURN
      END
*
*=====
* Name:      rnof_arc
* Purpose:   Generate surface-runoff arcs.
*=====
      SUBROUTINE RNOF_ARC(NDTYP, NDSEQ, LDND,
& NARC, II, JJ, LO, HI, COST, ARTYP, LDARC,
& INST, LDRES,
& NDXAR, LDXAR, NRAIN, RAINND, LDRAIN, RAINTY,
& NRNOF, RNOFND, LDRNOF, RNOFTB, LDRFTB,
& A5DR, PERD,
& SKSC, XF, IU,
& CTERM, COLSTR, LDCOL)
      IMPLICIT NONE
      INTEGER LDND, NDTYP(LDND), NDSEQ(LDND)
      INTEGER NARC, LDARC,
& II(LDARC), JJ(LDARC), LO(LDARC), HI(LDARC),
& COST(LDARC), ARTYP(LDARC)
      INTEGER LDRES
      REAL INST(LDRES)
      INTEGER LDXAR, NDXAR(LDXAR, 6)
      INTEGER NRAIN, LDRAIN, RAINND(LDRAIN, 3), RAINTY
      INTEGER NRNOF, LDRNOF, RNOFND(LDRNOF), LDRFTB
      REAL RNOFTB(LDRNOF, 0:LDRFTB), A5DR(LDRNOF, 5), PERD
      INTEGER IU, SKSC
      REAL XF

```

```

      INTEGER      LDCOL
      CHARACTER    CTERM*(*), COLSTR(LDCOL)*(*)
*
      INTEGER      I, NRN, ND, NNDS, RES, NV, RFND
      REAL          ZVA(3), UC(0:2,2), RNOF, RNOF2, RAIN, AREA, SCSCN,
&
      LOGICAL      MXRET
*
      LOGICAL      GETZVA
*
      UC(0,1) = 12.0          ! FT    --> INCHES
      UC(1,1) = 1.0          ! INCHES --> INCHES
      UC(2,1) = 0.03937      ! MM     --> INCHES
      UC(0,2) = 1.0          ! AC-FT/D --> AC-FT/D
      UC(1,2) = 86400./43560. ! CFS   --> AC-FT/D
      UC(2,2) = 1.0/43560    ! CFD    --> AC-FT/D
*
      DO I = 1, NRAIN
                                NDXAR(ND, 3) = 0
      ENDDO
*
*-----Read the rainfall data
*
      READ (IU, '(A)', END = 999) CTERM
      CALL STR_DIVD(CTERM, NNDS, COLSTR, LDCOL, 0, ' ', ')
      DO NRN = 1, NRAIN
                                ND = RAINND(NRN, 1)
                                IF (RAINND(NRN, 3) .LT. 0) THEN
                                  READ (COLSTR(NNDS), '(F15.0)') RAIN
                                ELSE
                                  READ (COLSTR(NRN+1), '(F15.0)') RAIN
                                ENDIF
                                IF (RAINY .EQ. 1) THEN
                                  RAIN = RAIN * UC(RAINND(NRN, 2), 1) ! CONVERT TO INCHES
*
*-----Find the drainage area and SCS curve number
*
                                RFND = 1
                                AREA = 0.0
                                DO WHILE (RFND .LE. NRNOF .AND. RNOFND(RFND) .NE. ND)
                                  RFND = RFND + 1
                                ENDDO
                                IF (RFND .GT. NRNOF) THEN
                                  AREA = 0.0
                                  SCSCN = 0.0
                                ELSE
                                  AREA = RNOFTB(RFND, 4)
                                CALL SCS_CN(SCSCN, RFND, RNOFTB, LDRNOF, LDRFTB, A5DR,
&
                                  RAIN, PERD)
                                ENDIF
*
*-----Find the reservoir water surface area
*
                                ZVA(3) = 0.0
                                RNOF2 = 0.0
                                IF (NDTYP(ND) .EQ. 1) THEN
                                  RES = NDSEQ(ND)
                                  ZVA(2) = INST(RES)
                                  IF (GETZVA(ND, 2, ZVA, 26)) THEN
                                    RNOF2 = RAIN / 12.0 * ZVA(3) * XF
                                  ENDIF
                                ENDIF
*
*-----surface runoff
*
                                AREA = AREA - ZVA(3)
                                RNOF = 0.0
                                IF (AREA .GT. 0.0000001) THEN
                                  MXRET = 1000.0 / SCSCN - 10.0
                                  IF (RAIN .GT. 0.2 * MXRET) THEN
                                    RNOF = (RAIN - 0.2 * MXRET) * (RAIN - 0.2 * MXRET)
&
                                    / (RAIN + 0.8 * MXRET) / 12.0 * AREA * XF
                                  ELSE
                                    RNOF = 0.0
                                  ENDIF
                                ENDIF
*
*-----Create runoff arc
*
                                NV = NINT (RNOF + RNOF2)
                                ELSE IF (RAINY .EQ. 2) THEN
                                  RAIN = RAIN * UC(RAINND(NRN, 2), 2) ! TO AC-FT/D
                                  NV = NINT(RAIN * PERD * XF)
                                ENDIF
                                IF (NV .GT. 0) THEN
                                  CALL ARCVL(NARC, II, JJ, LO, HI, COST, ARTYP,
&
                                  LDARC, SKSC, ND, NV, NV, 0, 5)
                                  NDXAR(ND, 3) = NARC
                                ENDIF
      ENDDO
999  RETURN
      END
*=====
* Name:      SCS_CN
* Purpose:   Calculate the SCS curve number based on antecedent 5-day
*            rainfall conditions.

```

```

=====
      SUBROUTINE SCS_CN(CN, RFND, RNOFTB, LDRNOF, LDRFTB, A5DR, RAIN,
&                      PERD)
      IMPLICIT      NONE
      INTEGER      RFND, LDRNOF, LDRFTB
      REAL         CN, RNOFTB(LDRNOF, 0:LDRFTB), A5DR(LDRNOF, 5), RAIN
      REAL         PERD

      INTEGER      I
      REAL         A5DR0

      CN = RNOFTB(RFND, 3)
      A5DR0 = RNOFTB(RFND, 0)
      IF (A5DR0 .LT. RNOFTB(RFND, 1)) THEN
         CN = 4.2 * CN / (10.0 - 0.058 * CN)
      ELSE IF (A5DR0 .GT. RNOFTB(RFND, 2)) THEN
         CN = 23.0 * CN / (10.0 + 0.13 * CN)
      ENDIF

      *-----Update the 5-day antecedent rainfalls
      IF (INT(PERD) .GT. 4) THEN
         DO I = 1, 5
            A5DR(RFND, I) = RAIN / PERD
         ENDDO
      ELSE
         DO I = 5, INT(PERD)+1, -1
            A5DR(RFND, I) = A5DR(RFND, I-1)
         ENDDO
         DO I = 1, INT(PERD)
            A5DR(RFND, I) = RAIN / PERD
         ENDDO
      ENDIF
      A5DR0 = 0.0
      DO I = 1, 5
         A5DR0 = A5DR0 + A5DR(RFND, I)
      ENDDO
      RNOFTB(RFND, 0) = A5DR0
      RETURN
      END
=====
* Name:      weirflw
* Purpose:    Calculate flow over a sharp-crested weir.
=====
      SUBROUTINE WEIRFLW(Q, H, HT, B, P, W, HD, WEIRTYPE, IFAULT)
      IMPLICIT      NONE
      INTEGER      WEIRTYPE, IFAULT
      REAL         Q, H, HT, B, P, W, HD

      REAL         CD

      *-----Determine the coefficients of discharge
      IFAULT = 0
      IF (WEIRTYPE .EQ. 1) THEN
         CALL CDSCW(CD, H, P)
      ELSE IF (WEIRTYPE .EQ. 2) THEN
         CALL CDRCW(CD, H, HD)
      ELSE IF (WEIRTYPE .EQ. 3) THEN
         CALL CDBCW(CD, H, HT, P, W)
      ELSE
         IFAULT = 1
      ENDIF
      Q = CD * B * SQRT(2.0 * 32.17) * H**1.5
      RETURN
      END
=====
* Name:      weirhite
* Purpose:    Determine the weir height for a given flwo
=====
      SUBROUTINE WEIRHITE(Q, H, HT, B, P, W, HD, WEIRTYPE, IFAULT)
      IMPLICIT      NONE
      REAL         Q, H, HT, B, P, W, HD
      INTEGER      WEIRTYPE, IFAULT

      REAL         X1, X2, TOL, QQ
      DATA        TOL / 0.0001/

      IFAULT = 0
      IF (WEIRTYPE .NE. 1) THEN ! ONLY SHARP-CRESED WEIR IS SUPPORTED.
         IFAULT = 2
         RETURN
      ENDIF

      * Note: h is the water depth approaching the weir, referring to
      *       the base of weir, i.e. river bed.

      *-----Find the weir height.
      X1 = 0.0
      X2 = H
      QQ = 0.0
      DO WHILE(ABS(QQ - Q) / Q .GT. TOL)

```

```

                                P = 0.5 * (X1 + X2)
                                CALL WEIRFLW(QQ, H-P, HT-P, B, P, W, HD, WEIRTP, IFAULT)
                                IF (QQ .GT. Q) THEN !INCREASE THE HEIGHT OF WEIR TO REDUCE FLOW
                                    X1 = P
                                ELSE IF (QQ .LT. Q) THEN !DECREASE THE HEIGHT OF WEIR
                                    X2 = P
                                ENDIF
                                ENDDO
*
* RETURN
* END
*====
* Calculation of coefficient of discharge for sharp-crested weir
*====
SUBROUTINE CDSCW(CD, H, P)
IMPLICIT NONE
REAL CD, H, P
*
*-----Discharge coefficient for free outflow
*
CD = 1.06*((14.14*P/(8.15*P+H))**10.0+(H/(H+P))**15.0)**(-0.1)
CD = 2.0 / 3.0 * CD
RETURN
END
*====
* Calculation of coefficient of discharge for round-crested weir
*====
SUBROUTINE CDRCW(CD, H, HD)
IMPLICIT NONE
REAL CD, H, HD
*
REAL HHD
*
*-----Discharge coefficient for free outflow
*
CD = 0.502
IF (HD .GT. 0.0001 .AND. H .GT. 0.0001) THEN
    HHD = H / HD
    CD = 0.3849 + 0.3849 * 4.0 * HHD / (9.0 + 5.0 * HHD)
ENDIF
RETURN
END
*====
* Calculation of coefficient of discharge for broad-crested weir
*====
SUBROUTINE CDBCW(CD, H, HT, P, W)
IMPLICIT NONE
REAL CD, H, HT, P, W
*
INTEGER ISHAPE, IFWTYP
REAL CS, CC, PH, HTH, HW
*
*-----Coefficient of discharge for free outflow
*
IF (W .GT. 0.00001) THEN
    HW = H / W
    CD = 0.5 + 0.1 * ((HW**5+1500.0*HW**13)/(1.0+1000*HW**3))**0.1
    CD = 2.0 / 3.0 * CD
ELSE
    PH = P/H
    ISHAPE = 0
    IF (ISHAPE .EQ. 0) THEN ! VERTICAL FACE WITH ROUNDED ENTRANCE.
        IF (PH .LE. 3) THEN
            CD = 0.36 + 0.01 * (3.0 - PH) / (1.2 + 1.5 * PH)
        ELSE
            CD = 0.36
        ENDIF
    ELSE IF (ISHAPE .EQ. 1) THEN ! VERTICAL FACE.
        IF (PH .LE. 3) THEN
            CD = 0.32 + 0.01 * (3.0 - PH) / (0.46 + 0.75 * PH)
        ELSE
            CD = 0.32
        ENDIF
    ENDIF
ENDIF
*
*-----Estimate the submerged coefficient
* The equation is the regressed equation obtained out of
* data from Chengdu, 1977.
*
HTH = HT / H
CS = 1.0
IF (HTH .GE. 0.8) THEN
    IFWTYP = 2
    IF (HTH .LT. 0.86) THEN
        CS = 1.6892 - 0.8571 * HTH
    ELSE
        CS = 1.2504 - 0.06897 * HTH / (1.0 - 0.9388 * HTH)
    ENDIF
    IF (CS .GT. 1.0) THEN
        CS = 1.0
    ENDIF
ENDIF

```



```

*
CC = 1.0
CD = CD * CS * CC
RETURN
END
=====
* Name:      gateflw
* Purpose:   Calculate discharge under gates.
* Author:    Xiaodong Jian
* Date:      12/10/96
=====
SUBROUTINE GATEFLW(Q, H, HT, B, E, HD, GATETY, IFAULT)
IMPLICIT NONE
REAL Q, H, HT, B, E, HD
INTEGER GATETY, IFAULT

REAL CD, EH, EPS
INTEGER IFWTYP
DATA EPS /1.0E-10/

*-----If e = 0, find the maximum flow under gate
*
IFault = 0
IF (ABS(E) .LT. EPS) THEN
    IF (GATETY .EQ. 2) THEN                !SPILLWAY
        E = 0.75 * H
    ELSE IF (GATETY .EQ. 3) THEN          !BROAD WERI
        E = 0.65 * H
    ENDIF
ENDIF

*-----Check gate opeing height.
*
EH = E / H
IF (GATETY .EQ. 2 .AND. EH .GT. 0.75+EPS) THEN
    IFault = 1
ELSE IF (GATETY .EQ. 3 .AND. EH .GT. 0.65+EPS) THEN
    IFault = 1
ENDIF

*-----Calculate the discharge coefficient.
*
IF (GATETY .EQ. 2) THEN ! VERTICAL FLAT GATE ON SPILLWAY.
    CALL CDGTSP(CD, H, E, HD)
ELSE IF (GATETY .EQ. 3) THEN ! VERTICAL FLAT GATE ON BROAD-CRESTED WEIR.
    CALL CDGTBW(CD, H, HT, E, IFWTYP)
ELSE
    IFault = 2
    RETURN
ENDIF

*-----Calculate the discharge under gate
*
Q = CD * B * E * SQRT(2.0 * 32.17 * H)

RETURN
END
=====
* Name:      gatehite
* Purpose:   Calculate the gate opening height for a given discharge
*            under gates.
* Author:    Xiaodong Jian
* Date:      01/24/97
=====
SUBROUTINE GATEHITE(Q, H, HT, B, E, HD, GATETY, IFAULT)
IMPLICIT NONE
REAL Q, H, HT, B, E, HD
INTEGER GATETY, IFAULT

REAL X1, X2, TOL, QQ
DATA TOL / 0.0001/

*-----maximum flow under gate.
*
E = 0.0
CALL GATEFLW(QQ, H, HT, B, E, HD, GATETY, IFAULT)
IF (QQ .LT. Q) THEN
    E = -999.99
    IFault = 1
    RETURN
ENDIF

*-----Find the gate opening height.
*
X1 = 0
X2 = H
DO WHILE(ABS(QQ - Q) / Q .GT. TOL)
    E = 0.5 * (X1 + X2)
    CALL GATEFLW(QQ, H, HT, B, E, HD, GATETY, IFAULT)
    IF (QQ .GT. Q) THEN
        X2 = E
    ELSE IF (QQ .LT. Q) THEN
        X1 = E
    ENDIF
ENDWHILE

```

```

      ENDDO
*
*
      RETURN
      END
=====
* Name:      cdgtbw
* Purpose:    Compute the discharge coefficient for flow under sluice-
*             gate on the broad weir.
* Author:     Xiaodong Jian
* Date:       12/17/96
=====
      SUBROUTINE CDGTBW(CD, H, HT, E, IFWTYP)
      IMPLICIT NONE
      REAL    CD, H, HT, E
      INTEGER  IFWTYP

      REAL    HMAX

      IFWTYP = 1
      CD = 0.611 * ((H - E) / (H + 15.0*E))**0.072

*-----Check flow type
*
      IF (E .NE. 0) THEN
         HMAX = 0.81 * HT * (HT / E)**0.72
         IF (H .GT. HT .AND. H .LT. HMAX) THEN ! SUBMERGED FLOW
            IFWTYP = 2
            CD = CD * (H - HT)**0.7 / (0.32 * (0.81 * HT * (HT/E)**0.72
            & - H)**0.7 + (H - HT)**0.7)
         ENDIF
      ELSE
         IFWTYP = 0
      ENDIF
      RETURN
      END
=====
* cdgtsp -- Calculate the discharge coefficient for plane gate on
*          spillway
=====
      SUBROUTINE CDGTSP(CD, H, E, HD)
      IMPLICIT NONE
      REAL    CD, H, E, HD

      REAL    EH, EHD, EPS
      DATA   EPS /0.0001/

      EH = E / H

      IF (HD .GT. EPS) THEN
         EHD = E / HD
         CD = 0.495 / EH * (1.0 - (1.0 - EH)**1.5) * (0.1667 + EHD)**0.1111
      ELSE
         CD = 0.65 - 0.186 * EH
      ENDIF
      RETURN
      END
=====
* Name:      pipe_flw
* Purpose:    Calculate flows through a pipe.
=====
      SUBROUTINE PIPE_FLW(PIPTYP, FLW, H, DIAM, LENG, FRIC, ENLOS,
      &                    ROUGH)
      IMPLICIT NONE
      INTEGER  PIPTYP
      REAL    FLW, H, DIAM, LENG, FRIC, ENLOS, ROUGH

      REAL    DISCH, R, J, A

      IF (PIPTYP .EQ. 0) THEN !SHORT PIPE
         DISCH = 1.0 / SQRT(1.0 + FRIC * LENG / DIAM + ENLOS)
         FLW = DISCH * (3.14159 * DIAM**2 / 4.0) * SQRT(2.0 * 32.17 * H)
      ELSE
         !LONG PIPE
*-----Long pipe: Q = K J**0.5, where K = A C R**0.5, A -- area
* C -- Chezy coefficient (or manning coefficient)
* J -- Hydraulic slope.
* then Q = 1.486 * A / rough * R**(2/3) * J**0.5
*
         R = DIAM / 4.0
         J = H / LENG
         A = 3.14159 * DIAM * DIAM / 4.0
         FLW = 1.486 * A * R**0.6667 * SQRT(J) / ROUGH
      ENDIF
      RETURN
      END
=====
* Name:      fb_dat
* Purpose:    Open a seasonal flow bound data file for selected arcs.
=====
      SUBROUTINE FB_DAT(II, JJ, ARTYP, LDARC, NDNAM, LDND,
      &                  NFBAR, FBAR, LDFBAR, FBTB, LDFBTB,
      &                  FBUNIT, FBFLAG, FILNAM, NTLN, IN, PN, OU,
      &                  PTDWAR, LDPTDW, NDDWAR, LDDWAR,

```

```

&          UNITNM, LDUNIT, CTERM, COLSTR, LDCOL)
IMPLICIT   NONE
INTEGER    LDARC, II(LDARC), JJ(LDARC), ARTYP(LDARC), LDND
CHARACTER  NDNAM(LDND)*(*), FILNAM*(*)
INTEGER    NFBAR, LDFBAR, FBAR(0:7, LDFBAR), FBUNIT
INTEGER    LDFBTB, LDPTDW, LDDWAR
REAL       FBTB(0:LDFBTB, LDFBAR)
LOGICAL    FBFLAG
INTEGER    PTDWAR(LDPTDW, 2), NDDWAR(LDDWAR)
INTEGER    NTL5, IN, PN, OU, LDUNIT, LDCOL
CHARACTER*(*) UNITNM(0:LDUNIT), CTERM, COLSTR(LDCOL)

*
INTEGER    IU
INTEGER    NREC, SL
INTEGER    I, J, K, L, M, N
LOGICAL    ERR, FLAG, ENDFIL
LOGICAL    CN
INTEGER    SAVOPT
COMMON     /SAVOPT/ SAVOPT

FBFLAG = .FALSE.
NFBAR = 0

*-----Open data file and skip title lines
*
IF (FILNAM .NE. ' ') THEN
    FLAG = .TRUE.
    IU = 9
    CALL IO_OPFIL(IU, 1, FILNAM, 'ENTER SEASONAL FLOW BOUND FILE: ')
    DO I = 1, NTL5
        READ (IU, *, END = 99)
    ENDDO
ELSE
    FLAG = .FALSE.
    IU = IN
    CALL PNCK(PN, IU, ENDFIL, CTERM, COLSTR, LDCOL)
    IF (ENDFIL) GOTO 99
ENDIF
CALL FB_HED(NDNAM, LDND, II, JJ, ARTYP, LDARC,
&          NFBAR, FBAR, LDFBAR, FBUNIT,
&          ENDFIL,
&          PTDWAR, LDPTDW, NDDWAR, LDDWAR, FILNAM, IU,
&          CTERM, COLSTR, LDCOL, ERR)
IF (ERR) STOP !CALL EXIT
IF (ENDFIL) GOTO 99
FBFLAG = .TRUE.

*-----Read data
*
NREC = 0
CONTINUE
30  READ (IU, '(A)', END = 99) CTERM
    IF (CN(CTERM, 'FINISH', 1)) GOTO 99
    CALL STR_DIVD(CTERM, J, COLSTR, LDCOL, 0, ' ', ' ')
    IF (J .NE. (NFBAR + 1)) THEN
        PRINT *, '***ERROR*** FILE = ', FILNAM
        PRINT *, '                FOR TIME = ', COLSTR(1)
        STOP !CALL EXIT
    ENDIF

    NREC = NREC + 1
    DO J = 1, NFBAR
        READ (COLSTR(J+1), '(F10.0)') FBTB(NREC, J)
    ENDDO
    GOTO 30
99  CONTINUE
    IF (FLAG) CLOSE(IU)

*-----Print FB information into the general output file
*
IF (NREC .GT. 0) THEN
    ENDFIL = .FALSE.
    FBFLAG = .TRUE.
    IF (SAVOPT .EQ. 0 .OR. SAVOPT .EQ. 1) THEN
        CALL STR_LEN(UNITNM(FBUNIT), K)
        WRITE(OU, 801) PN, UNITNM(FBUNIT)(1:K),
&          ('=', I = 1, K)
801  &          FORMAT(//, 'Part ', I2, ': Seasonal flow bounds, in ', A,
&          /, '=====', 10A)

        DO K = 1, NFBAR, 8
            IF ((K+7) .GT. NFBAR) THEN
                L = NFBAR
            ELSE
                L = K + 7
            ENDIF
        ENDIF
    ENDIF

*-----1. Upstream and downstream nodal names
*
WRITE(OU, 805) ' Upstream node:',
&          (NDNAM(FBAR(1,J))(1:SL(NDNAM(FBAR(1,J))))), J = K, L)
&          WRITE(OU, 805) ' Downstream node:',
805  &          (NDNAM(FBAR(2,J))(1:SL(NDNAM(FBAR(2,J))))), J = K, L)
    FORMAT(A, T20, 10A12)

```

```

*
*-----2. Flow bounds index
*
      WRITE (OU, 806) (FBAR(3, J), J = K, L)
806      FORMAT(' Flow zone index:', T20, 10I12)
*
*-----3. Arc number
*
      WRITE (OU, 807) (FBAR(0, J), J = K, L)
807      FORMAT('
                                Arcs:', T20, 10I12)
*
                                DO N = 4, 5
                                M = 1
                                CTERM = ' '
                                DO J = K, L
                                IF (FBAR(N,J) .NE. 0) THEN
                                WRITE(CTERM(M:M+11), '(I12)') FBAR(N,J)
                                ENDF
                                M = M + 12
                                ENDDO
                                IF (CTERM .NE. ' ') THEN
809      FORMAT ('
                                EXTENDED ARC:', T20, A)
                                ENDF
                                ENDDO
*
*-----4. Data matrix
*
                                DO I = 1, NREC
                                WRITE (OU, 810) I, (FBTB(I, J), J = K, L)
810      FORMAT (5X, I5, T20, 10F12.2)
                                ENDDO
                                ENDDO
                                ENDF
*
      RETURN
      END
=====
* Name:      fb_fil
* Purpose:   Open a time series file header.
=====
      SUBROUTINE FB_FIL(NDNAM, LDND, II, JJ, ARTYP, LDARC,
&      PTDWAR, LDPTDW, NDDWAR, LDDWAR,
&      NFBAR, FBAR, LDFBAR, FBUNIT,
&      UNITNM, LDUNIT, FBFLAG, FBFIL, FILNAM,
&      NTLN, IU, OU, CTERM, COLSTR, LDCOL)
      IMPLICIT NONE
      INTEGER LDND, LDARC, II(LDARC), JJ(LDARC), ARTYP(LDARC)
      INTEGER LDPTDW, LDDWAR, PTDWAR(LDPTDW, 2), NDDWAR(LDDWAR)
      INTEGER NFBAR, LDFBAR, FBAR(0:7, LDFBAR), FBUNIT, LDUNIT
      CHARACTER*(*) NDNAM(LDND), UNITNM(0:LDUNIT), FILNAM
      INTEGER NTLN, IU, OU
      LOGICAL FBFLAG, FBFIL
      INTEGER LDCOL
      CHARACTER CTERM*(*), COLSTR(LDCOL)*(*)
*
      INTEGER I, J, K, L, M, NREC, SL, N, FBIDX(100)
      LOGICAL ERR, ENDFIL
      INTEGER SAVOPT
      COMMON /SAVOPT/ SAVOPT
*
      FBFIL = .FALSE.
      IF (FILNAM .EQ. ' ') THEN
                                GOTO 99
      ENDF
      CALL IO_OPFIL(IU, 1, FILNAM, 'ENTER FB DATA FILE: ')
*
*-----Read title lines, flow unit, upstream and downstream nodal names,
* and flow zone index. And find the corresponding arc and nodal
* numbers.
*
      DO I = 1, NTLN
                                READ (IU, *)
      ENDDO
      CALL FB_HED(NDNAM, LDND, II, JJ, ARTYP, LDARC,
&      NFBAR, FBAR, LDFBAR, FBUNIT,
&      ENDFIL,
&      PTDWAR, LDPTDW, NDDWAR, LDDWAR, FILNAM, IU,
&      CTERM, COLSTR, LDCOL, ERR)
      IF (ERR) STOP !CALL EXIT
      IF (ENDFIL) GOTO 99
      FBFIL = .TRUE.
      FBFLAG = .TRUE.
*
      IF (FBFIL) THEN
*
*-----No. of time-dependent arcs
*
                                DO I = 1, 100
                                FBIDX(I) = 0
                                ENDDO
                                N = 0
                                DO I = 1, NFBAR

```

```

      J = FBAR(7,I)
      IF (J .NE. 0) THEN
        N = N + 1
        FBIDX(J) = I
      ENDIF
    ENDDO
*
*-----Save file information
*
      IF (SAVOPT .EQ. 0 .OR. SAVOPT .EQ. 1) THEN
        CALL NORECS(IU, NREC)
        WRITE (OU, 800) FILNAM, UNITNM(FBUNIT), N, NREC
800    FORMAT (
      &    //, 'Summary for flow-bound data file: ',
      &    //, '=====',
      &    //, 12X, '      File name: ', A,
      &    //, 12X, '      Data unit: ', A,
      &    //, 12X, '      Number of arcs: ', I4,
      &    //, 12X, '      Number of records: ', I4)
*
      DO J = 1, N, 5
        K = J + 4
        IF ((J+4) .GT. N) THEN
          K = N
        ELSE
          K = J + 4
        ENDIF
        WRITE (OU, 805)
      &    (NDNAM(FBAR(1,FBIDX(I)))(1:SL(NDNAM(FBAR(1,FBIDX(I))))),
      &    I = J, K)
        WRITE (OU, 806)
      &    (NDNAM(FBAR(2,FBIDX(I)))(1:SL(NDNAM(FBAR(2,FBIDX(I))))),
      &    I = J, K)
        WRITE (OU, 807) (FBAR(3, FBIDX(I)), I = J, K)
        WRITE (OU, 808) (FBAR(0, FBIDX(I)), I = J, K)
805    FORMAT ('      List of upstream nodal names: ', 5A10)
806    FORMAT ('      List of downstream nodal names: ', 5A10)
807    FORMAT ('      List of flow-zone indexes: ', 5I10)
808    FORMAT ('      List of arc numbers: ', 5I10)
*
      DO L = 4, 5
        M = 1
        CTERM = ' '
        DO I = J, K
          IF (FBAR(L,FBIDX(I)) .NE. 0) THEN
            WRITE(CTERM(M:M+9), '(I10)') FBAR(L,FBIDX(I))
            ENDIF
            M = M + 10
          ENDDO
          IF (CTERM .NE. ' ') THEN
            WRITE (OU, 811) CTERM(1:50)
811    FORMAT ('      EXTENDED ARC NUMBERS: ', A)
          ENDIF
          ENDDO
        ENDDO
      ENDIF
    ENDIF
*
99  RETURN
END
*=====
* Name:      fb_arc
* Purpose:   Read current flow bounds and modify the flow bounds in the
*            arcs.
*=====
      SUBROUTINE FB_ARC(LO, HI, ARTYP, LDARC,
      &    NFBAR, FBAR, LDFBAR, FBTB, LDFBTB, FBFIL,
      &    MTH, PERD, XF, IU,
      &    CTERM, COLSTR, LDCOL)
      IMPLICIT NONE
      INTEGER LDARC, LO(LDARC), HI(LDARC), ARTYP(LDARC)
      INTEGER NFBAR, LDFBAR, FBAR(0:7,LDFBAR), LDFBTB
      REAL FBTB(0:LDFBTB, LDFBAR)
      LOGICAL FBFIL
*
      INTEGER IU, MTH
      REAL PERD, XF
*
      INTEGER LDCOL
      CHARACTER CTERM*(*), COLSTR(LDCOL)*(*)
*
      INTEGER I, J, N, ARC, ARC2, NV, ZNIDX
      REAL UC(0:2)
      LOGICAL STRM, TDFLAG
*
*-----Unit conversion factor: 0 -- ac-ft --> ac-ft
*            1 -- cfs --> ac-ft
*            2 -- cfd --> ac-ft
*
      UC(0) = 1.0
      UC(1) = PERD * 86400.0 / 43560.0
      UC(2) = PERD / 43560.0
*
*-----Read current flow bounds

```

```

*
      TDFLAG = .FALSE.
      IF (FBFIL) THEN
          READ (IU, '(A)', END = 50) CTERM
          CALL STR_DIVD(CTERM, I, COLSTR, LDCOL, 0, ' ', ')
          TDFLAG = .TRUE.
      ENDIF
50  CONTINUE
*
*-----Change flow bounds for arcs
*
      DO 100 J = 1, NFBAR
*-----Get a flow bound
*
          IF (FBAR(7, J) .EQ. 0) THEN
              N = MTH
              ELSE
                  N = 0
                  IF (TDFLAG) THEN
                      I = FBAR(7, J) + 1
                      READ (COLSTR(I), '(F10.0)') FBTB(0, J)
                      ELSE
                          FBTB(0, J) = 0.0
                          GOTO 100
                      ENDIF
                  ENDIF
*
          ARC = FBAR(0, J)
          ZNIDX = FBAR(3, J)
          WRITE (CTERM, '(I4)') ARTYP(ABS(ARC))
          IF (CTERM(3:3) .EQ. '1') THEN
              STRM = .TRUE.
              ARC2 = IABS(ARC) + 1
              ELSE
                  ARC2 = 0
                  ENDIF
          NV = NINT(FBTB(N, J) * UC(FBAR(6, J)) * XF)
          IF (ARC .GT. 0 .AND. ZNIDX .EQ. -1) THEN
              LO(ARC) = NV
              IF (ARC2 .GT. 0) LO(ARC2) = NV
              ELSE
                  HI(ABS(ARC)) = NV
                  IF (ARC2 .GT. 0) HI(ABS(ARC2)) = NV
                  ENDIF
*
100  CONTINUE
99  RETURN
END
*=====
* Name:      fb_hed
* Purpose:    Read header of the flow bound file.
*=====
      SUBROUTINE FB_HED(NDNAM, LDND, II, JJ, ARTYP, LDARC,
&                      NFBAR, FBAR, LDFBAR, FBUNIT,
&                      ENDFIL,
&                      PTDWAR, LDPTDW, NDDWAR, LDDWAR, FILNAM, IU,
&                      CTERM, COLSTR, LDCOL, ERR)
      IMPLICIT NONE
      INTEGER LDND, LDARC, II(LDARC), JJ(LDARC), ARTYP(LDARC)
      INTEGER NFBAR, LDFBAR, FBAR(0:7, LDFBAR), FBUNIT, IU
      INTEGER LDPTDW, PTDWAR(LDPTDW, 2), LDDWAR, NDDWAR(LDDWAR)
      CHARACTER NDNAM(LDND)*(*), FILNAM*(*)
      LOGICAL ENDFIL
      INTEGER LDCOL
      CHARACTER CTERM*(*), COLSTR(LDCOL)*(*)
*
      INTEGER ND1, ND2, ND, FBIDX, TYP, ZONE, ARC, ARC1, ISGN
      INTEGER I, J, K, L, N, I1, I2
      LOGICAL ERR, STRM, ARFIND
      INTEGER LDFB
      PARAMETER (LDFB = 100)
      INTEGER NFB, FB DAT(3, LDFB)
*
      ENDFIL = .TRUE.
*
*-----Flow units
*
      READ (IU, *, END = 99) FBUNIT
*
*-----Read upstream and downstream nodal names and zone index.
*
      DO I = 1, 3
          READ (IU, '(A)', END = 99) CTERM
          CALL STR_DIVD(CTERM, L, COLSTR, LDCOL, 0, ' ', ')
          IF (L .GT. LDFB) THEN
              PRINT *, '***ERROR*** ARRAY SIZE IS NOT BIG ENOUGH.'
              PRINT *, ' CHANGE LDFB IN FB_HED AT LEAST ', L
              STOP !CALL EXIT
              ENDIF
          IF (I .EQ. 3) THEN
              DO J = 2, L
                  READ (COLSTR(J), '(I2)') FB DAT(I, J-1)
              ENDDO
              ELSE

```

```

          NFB = L
          DO J = 1, L
            CALL NAMNUM(LDND, NDNAM, COLSTR(J), ND, 0, ERR)
            IF (ERR) THEN
              WRITE(*, 901) COLSTR(J), FILNAM
901      &      FORMAT( '***ERROR***NODAL NAME: ', A12,
          &      ' IN THE FILE: ', A)
              PRINT *, ' NOT FOUND IN THE NETWORK CONFIGURATION.'
              STOP !CALL EXIT
            ELSE
              FB DAT(I, J) = ND
            ENDIF
          ENDDO
        ENDIF
      ENDDO
    IF (NFBAR .EQ. 0) THEN
      *
      *-----seasonal data
      *
          NFBAR = NFB
          DO J = 1, NFB
            DO I = 1, 3
              FBAR(I, J) = FB DAT(I, J)
            ENDDO
            FBAR(6, J) = FBUNIT
            FBAR(7, J) = 0
          ENDDO
        ELSE
      *
      *-----Time-dependent data
      *
          N = 0
          DO 10 K = 1, NFB
            DO J = 1, NFBAR
              IF (FBAR(1, J) .EQ. FB DAT(1, K) .AND.
          &      FBAR(2, J) .EQ. FB DAT(2, K) .AND.
          &      FBAR(3, J) .EQ. FB DAT(3, K)) THEN
                FBAR(6, J) = FBUNIT
                FBAR(7, J) = K
                GOTO 10
              ENDIF
            ENDDO
            N = N + 1
            J = NFBAR + N
            DO I = 1, 3
              FBAR(I, J) = FB DAT(I, K)
            ENDDO
            FBAR(6, J) = FBUNIT
            FBAR(7, J) = K
          10    CONTINUE
      *
      *-----total number of seasonal & time-dependent flow bound arcs
      *
          NFBAR = NFBAR + N
        ENDIF
      *
      *-----Find corresponding arc number.
      *
          DO 50 J = 1, NFBAR
            ND1 = FBAR(1, J)
            ND2 = FBAR(2, J)
            FBIDX = FBAR(3, J)
            I1 = PTDWAR(ND1, 1)
            I2 = PTDWAR(ND1, 2)
            ARFIND = .FALSE.
            K = 3
            DO 20 I = I1, I2
              ARC = NDDWAR(I)
            20
          50
      *
      *-----Arc type
      *
          TYP = ARTYP(ABS(ARC))
          WRITE(CTERM, '(I4)') ARTYP(ABS(ARC))
          IF (CTERM(2:2) .NE. '1') THEN
            PRINT *, '*** ERROR ** INVALID ARC TYPE'
            GOTO 99
          ENDIF
          IF (CTERM(3:3) .EQ. '1') THEN
            STRM = .TRUE.
          ELSE
            STRM = .FALSE.
          ENDIF
          READ (CTERM(4:4), '(I1)') ZONE
          ZONE = ISGN(TYP) * ZONE
      *
      *-----Downstream node
      *
          ARC1 = ARC
          IF (STRM) THEN
            ARC = ARC + ISGN(ARC)
          ENDIF
          IF (ARC .GT. 0) THEN
            ND = JJ(ARC)
          ELSE

```

```

                                ND = II(-ARC)
                                ENDIF
*
*-----check the current downstream node
*
                                IF (ND .EQ. ND2) THEN
                                IF ( (ABS(ZONE)+1) .EQ. IABS(FBIDX) .AND.
&                                ZONE * FBIDX .GE. 0) THEN
                                IF (ARFIND) THEN
                                    K = K + 1
                                    FBAR(K, J) = ARC1
                                ELSE
                                    FBAR(0, J) = ARC1
                                    ARFIND = .TRUE.
                                ENDIF
                                IF (FBIDX .LT. 0) THEN
                                    FBIDX = FBIDX - 1
                                ELSE
                                    FBIDX = FBIDX + 1
                                ENDIF
                                ENDIF
                                ENDIF
20      CONTINUE
                                IF (.NOT. ARFIND) THEN
                                    PRINT *, CHAR(7)
                                    WRITE (*, 810) FILNAM
810      FORMAT('***ERROR*** THE FILE NAME IS ', A)
                                    WRITE (*, 811) NDNAM(ND1), NDNAM(ND2), FBIDX
811      FORMAT(' THERE DOES NOT AN ARC FROM ', A,
&              'TO ', A, ' WITH ZONE INDEX = ', I3)
                                    ERR = .TRUE.
                                ENDIF
50      CONTINUE
                                ENDFIL = .FALSE.
99      RETURN
                                END
*=====
* Name:      rc_dat
* Purpose:    Get seasonal rule curve.
*=====
SUBROUTINE RC_DAT(NNODS, NDNAM, LDND,
&                NRCND, RCND, LDRC, NPER, RCTB, LDRCTB, RCUNIT,
&                RCFLAG, FILNAM, NTLN, IN, OU, PN,
&                UNITNM, LDUNIT, CTERM, COLSTR, LDCOL)
IMPLICIT NONE
INTEGER NNODS, LDND, LDUNIT
CHARACTER*(*) NDNAM(LDND), FILNAM, UNITNM(0:LDUNIT)*(*)
INTEGER NRCND, LDRC, RCND(LDRC, 3), NPER, LDRCTB, RCUNIT
REAL RCTB(0:LDRCTB, LDRC)
INTEGER NTLN, IN, OU, PN
LOGICAL RCFLAG
INTEGER LDCOL
CHARACTER CTERM*(*), COLSTR(LDCOL)*(*)
*
INTEGER I, NREC, IU
LOGICAL FLAG, ENDFIL
*
*-----Open data file and skip title lines
*
IF (FILNAM .NE. ' ') THEN
                                FLAG = .TRUE.
                                IU = 9
                                CALL IO_OPFIL(IU, 1, FILNAM, 'ENTER RC FILE: ')
                                DO I = 1, NTLN
                                    !SKIP NTLN TITLE LINES
                                    READ (IU, *, END = 99)
                                ENDDO
ELSE
                                FLAG = .FALSE.
                                IU = IN
                                CALL PNCK(PN, IU, ENDFIL, CTERM, COLSTR, LDCOL)
                                IF (ENDFIL) GOTO 99
ENDIF
*
*-----Read units, nonal names, and data from a file and
* print these information into general output file.
*
CALL DATB(NNODS, NDNAM, LDND, NRCND, RCND, LDRC, RCUNIT,
&         NREC, RCTB, LDRCTB, RCFLAG,
&         UNITNM, LDUNIT, FILNAM, IU, OU, PN, ENDFIL,
&         'rule-curve elevations',
&         CTERM, COLSTR, LDCOL)
IF (NREC .LT. NPER) THEN
                                WRITE (OU, 805) NPER, NREC
805      FORMAT('***WARNING*** NO. OF SEASONAL RECORDS ARE LESS',
&              ' THAN NO. OF SEASONS IN RC DATA SET.',
&              ' NO. OF RECORDS: ', I3,
&              ' NO. OF SEASONS: ', I3)
                                ENDFIL
99      CONTINUE
                                IF (FLAG) CLOSE(IU)
                                RETURN
                                END
*=====
* Name:      rc_arcs

```



```

* Purpose:      Read current target water demand and create
*               corresponding TWS arcs.
*=====
      SUBROUTINE RC_ARC(NDNAM, NDSEQ, LDND, HI, LDARC,
&                     PTRE, RC, LDRES, REAR, REZN, LDREAR,
&                     NRCND, RCND, LDRC, RCTB, LDRCTB, RCFIL,
&                     MTH, XF, IU, CTERM, COLSTR, LDCOL)
      IMPLICIT NONE
      INTEGER LDND, NDSEQ(LDND), LDARC, HI(LDARC)
      CHARACTER NDNAM(LDND)*(*)
      INTEGER LDRES, PTRE(LDRES), LDREAR, REAR(LDREAR)
      REAL RC(LDRES), REZN(LDREAR)
      INTEGER NRCND, LDRC, RCND(LDRC, 3), LDRCTB
      REAL RCTB(0:LDRCTB, LDRC)
      LOGICAL RCFIL

      INTEGER IU, MTH
      REAL XF

      INTEGER LDCOL
      CHARACTER CTERM*(*), COLSTR(LDCOL)*(*)

      INTEGER I, J, J1, J2, N, ND, NNDS, RES, ARC
      REAL ZVA(3), UC(0:2), UPZN, LOZN, ZN
      LOGICAL GETZVA, FIRSTL, FIRSTU, TDFLAG

      UC(0) = 1.0
      UC(1) = 1.0 / 12.0
      UC(2) = 1.0 / 304.8

*-----Read current rule curve elevation for selected reservoirs
      TDFLAG = .FALSE.
      IF (RCFIL) THEN
        READ (IU, '(A)', END = 50) CTERM
        CALL STR_DIVD(CTERM, NNDS, COLSTR, LDCOL, 0, ' ', ' ')
        TDFLAG = .TRUE.
      ENDIF
50 CONTINUE

*-----Change the rule curve elevation.
      DO I = 1, NRCND
        ND = RCND(I, 1)
        IF (RCND(I, 3) .EQ. 0) THEN
          N = MTH
        ELSE
          N = 0
          IF (TDFLAG) THEN
            IF (RCND(I, 3) .LT. 0) THEN
              READ(COLSTR(NNDS), '(F15.0)') RCTB(0, I)
            ELSE
              J = RCND(I, 3) + 1
              READ(COLSTR(J), '(F15.0)') RCTB(0, I)
            ENDIF
          ELSE
            RCTB(0, I) = 0.0
          ENDIF
        ENDIF
        ZVA(1) = RCTB(N, I) * UC(RCND(I, 2))
        RES = NDSEQ(ND)
        IF (.NOT. GETZVA(ND, 1, ZVA, 26)) THEN
          PRINT *, CHAR(7)
          PRINT *, '***ERROR*** TRANSFORM Z-V-A:'
          PRINT *, '          POND = ', NDNAM(ND)
          PRINT *, '          RC = ', ZVA(1)
        ENDIF
        J1 = PTRE(RES)
        J2 = PTRE(RES+1) - 1
        FIRSTL = .TRUE.
        FIRSTU = .TRUE.

      UPZN = ZVA(2)
      LOZN = ZVA(2)
      DO J = J1, J2
        ARC = REAR(J)

        ZN = REZN(J)
        IF (ARC .LT. 0) THEN
          HI(-ARC) = NINT((LOZN - ZN)*XF)
          IF (HI(-ARC) .LT. 0) THEN
            HI(-ARC) = 0
          ELSE
            LOZN = ZN
          ENDIF
        ELSE IF (ARC .GT. 0) THEN
          HI(ARC) = NINT((ZN - UPZN)*XF)
          IF (HI(ARC) .LT. 0) THEN
            HI(ARC) = 0
          ELSE
            UPZN = ZN
          ENDIF
        ENDIF
      ENDIF
    ENDIF
  ENDIF

```

```

        ENDDO
        RC(RES) = ZVA(2)
    ENDDO
99    RETURN
    END
=====
* Name:      savbud
* Purpose:   Save system water budget into file.
* Author:    Xiaodong Jian
* Date:      04/02/97
=====
SUBROUTINE SAVBUD(NNDS, NDNAM, NDTYP, NODBUD, PTDWAR, LDND,
&               II, JJ, ARCBUD, NDDWAR, LDARC,
&               NOP, NSPS, XF, XP, OU,
&               CNDBT, CARBT, CSNDBT, LDSND, CSARBT, LDSAR)
    IMPLICIT NONE
    INTEGER LDND, LDARC, NNDS
    INTEGER NDTYP(LDND), NODBUD(LDND, 0:10), PTDWAR(LDND, 2)
    CHARACTER NDNAM(LDND)*(*)
    INTEGER II(LDARC), JJ(LDARC), NDDWAR(LDARC), ARCBUD(LDARC, 0:6)
    INTEGER NOP, NSPS, XP, OU
    REAL LDND, LDSND, LDSAR
    REAL CNDBT(0:10), CARBT(6)
    REAL CSNDBT(LDSND, 0:10), CSARBT(LDSAR, 0:6)

    REAL NDBT(0:10), ARBT(0:6), ALLBUD(8), ERRBD, BUD(6)
    INTEGER I, J, K, L, N, LIM1, LIM2, ARC, NARC, OJ
    CHARACTER FMT*60, FMT1*60, FMT2*50
    LOGICAL SWBFLG

    SWBFLG = .FALSE.
    FMT1 = '(10x, a, t35, 3f15.0)'
    FMT2 = '(10x, 69(''-'''), /, t35, a30, f15.0)'
    IF (XP .GT. 0) THEN
        WRITE (FMT1(20:20), '(i1)') XP - 1
        WRITE (FMT2(33:33), '(i1)') XP - 1
    ENDIF

    Initialize arrays for output budget

    DO I = 0, 10
        NDBT(I) = 0.0
        IF (NOP .EQ. 1) THEN
            CNDBT(I) = 0.0
        ENDIF
    ENDDO
    DO I = 1, 6
        ARBT(I) = 0.0
        IF (NOP .EQ. 1) THEN
            CARBT(I) = 0.0
        ENDIF
    ENDDO

    IF (NOP .EQ. 1) THEN
        DO N = 1, LDSND
            DO J = 0, 10
                CSNDBT(N, J) = 0.0
            ENDDO
        ENDDO
        DO N = 1, 500
            DO J = 0, 6
                CSARBT(N, J) = 0.0
            ENDDO
        ENDDO
    ENDIF

    -----Water budget of current time step
    -----Calculate the system water budget for current time step
    1. Nodal budget for current time step

    DO N = 1, NNDS
        DO I = 0, 8
            NDBT(I) = NDBT(I) + NODBUD(N,I)/XF
            IF (NOP .EQ. 1 .AND. I .EQ. 0) THEN
                CSNDBT(N, I) = NODBUD(N,I)/XF
            ELSE IF (I .GE. 1 .AND. I .LE. 7) THEN
                CSNDBT(N, I) = CSNDBT(N,I) + NODBUD(N,I)/XF
            ELSE IF (I .EQ. 8 .AND. NOP .EQ. NSPS) THEN
                CSNDBT(N, I) = NODBUD(N,I)/XF
            ENDIF
        ENDDO
    ENDDO

    2. Arc budget

    DO 100 N = 1, NNDS

```

```

LIM1 = PTDWAR(N, 1)
LIM2 = PTDWAR(N, 2)
DO 50 K = LIM1, LIM2
  ARC = NDDWAR(K)
  I = IABS(ARC)
  DO L = 1, 6
    IF (L .GE. 1 .AND. L .LE. 5) THEN
      ARBT(L) = ARBT(L) + ARCBUD(IABS(ARC), L)/ XF
    ENDIF
  *-----
    IF (NOP .EQ. 1 .AND. L .EQ. 2 ) THEN !INITIAL STORAGE
      CSARBT(I, L) = ARCBUD(I, L)/ XF
    ELSE IF (NOP .EQ. NSPS .AND. L .EQ. 5) THEN !FINAL STORAGE
      CSARBT(I, L) = ARCBUD(I, L)/XF
    ELSE
      CSARBT(I, L) = CSARBT(I, L) + ARCBUD(I, L)/XF
    ENDIF
  *-----
  ENDDO
*
*-----Outflow from the system
*
9    IF (ARC .GT. 0) THEN
      J = JJ(ARC)
    ELSE
      J = II(-ARC)
    ENDIF
    IF (NDNAM(J) .EQ. 'SKSC') THEN
      ARBT(6) = ARBT(6) + ARCBUD(IABS(ARC), 6)/ XF
    ENDIF
50   CONTINUE
100  CONTINUE
*
*      Save system water budgets of current time step into the file
*
IF (SWBFLG) THEN !SWBFLG -- SYSTEM WATER BUDEGT FLAG.
  WRITE(OU, 980) NOP
980  FORMAT (/, 'System water budgets for time step:', I3,
&      /, '=====')
  WRITE (OU, 955) 'Budget', 'Pond', 'Canal', 'Total'
  WRITE (OU, 955) '-----', '-----', '-----', '-----'
  ALLBUD(1) = NDBT(0)+ARBT(2)
  ALLBUD(2) = NDBT(2)
  ALLBUD(3) = NDBT(4)
  ALLBUD(4) = NDBT(3)+ARBT(4)
  ALLBUD(5) = NDBT(5)+ARBT(3)
  ALLBUD(6) = NDBT(6)
  ALLBUD(7) = ARBT(6)
  ALLBUD(8) = NDBT(8)+ARBT(5)
  WRITE(OU, FMT1) 'Initial storage:',
&      NDBT(0), ARBT(2), ALLBUD(1)
  WRITE(OU, FMT1) 'Total net inflow:',
&      NDBT(2), 0.0, ALLBUD(2)
  WRITE(OU, FMT1) 'Runoff:',
&      NDBT(4), 0.0, ALLBUD(3)
  WRITE(OU, FMT1) 'Evaporation:',
&      NDBT(3), ARBT(4), ALLBUD(4)
  WRITE(OU, FMT1) 'Groundwater:',
&      NDBT(5), ARBT(3), ALLBUD(5)
  IF (NDBT(6) .GT. 0) THEN
    WRITE(OU, FMT1) 'Water Withdraw:',
&      NDBT(6), 0.0, ALLBUD(6)
  ENDIF
  WRITE(OU, FMT1) 'Outflow:',
&      0.0, ARBT(6), ALLBUD(7)
  WRITE(OU, FMT1) 'Final storage:',
&      NDBT(8), ARBT(5), ALLBUD(8)
  ERRBD = ALLBUD(1) + ALLBUD(2) + ALLBUD(3) - ALLBUD(4)
&      - ALLBUD(5) - ALLBUD(6) - ALLBUD(7) - ALLBUD(8)
  WRITE (OU, FMT2) 'In - Out = ', ERRBD
ENDIF
*
*      Calculate the cumulative water budgets
*
IF (NOP .EQ. 1) THEN
  CNDBT(0) = NDBT(0)
  CARBT(2) = ARBT(2)
ENDIF
DO L = 2, 6
  CNDBT(L) = CNDBT(L) + NDBT(L)
ENDDO
*
*-----Final pond storage
*
CNDBT(8) = NDBT(8)
*
*-----Channel initial storage, loss, and final storage
*
IF (NOP .EQ. 1) THEN !INITIAL CANAL STORAGE
  CARBT(2) = ARBT(2)
ENDIF
DO L = 3, 4
  CARBT(L) = CARBT(L) + ARBT(L) !SEEPAGE AND EVAPORATE
ENDDO

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CARBT(6) = CARBT(6) + ARBT(6)      !OUTFLOW
CARBT(5) = ARBT(5)                 !FINAL CANAL STORAGE.
*
*      Save water budgets
*
IF (NOP .EQ. NSPS) THEN
*-----1. Save cumulative water budget of a single node
*
WRITE (OU, 990)
FMT = '(4X, a12, t16, i3, t21, 9f10.0) '
WRITE (FMT(30:30), '(i1)') XP-1
DO N = 1, NNDS
  WRITE (OU, FMT) NDNAM(N), NDTYP(N), (CSNDBT(N,I), I = 0,8)
ENDDO
*-----2. Save cumulative water budget of a single canal
*
WRITE (OU, 995)
FMT = '(i4, 1X, 2A12, T30, 6F10.0) '
WRITE(FMT(26:26), '(i1)') XP - 1
NARC = 0
DO 200 N = 1, NNDS
  LIM1 = PTDWAR(N, 1)
  LIM2 = PTDWAR(N, 2)
  OJ = 0
  DO 150 K = LIM1, LIM2
    ARC = NDDWAR(K)
    IF (ARC .GT. 0) THEN
      I = II(ARC)
    ELSE
      I = JJ(-ARC)
    ENDIF
    J = ARCBUD(IABS(ARC), 0)
*
*      New Canal reach:
*
    IF (J .NE. OJ) THEN
      IF (K .NE. LIM1) THEN
        NARC = NARC + 1
        WRITE(OU, FMT) NARC, NDNAM(I), NDNAM(OJ),
          &      (BUD(L), L = 1, 6)
      ENDIF
      OJ = J
      DO L = 1, 6
        BUD(L) = 0
      ENDDO
    ENDIF
*
*      add up the water budget for current canal reach
*
    DO L = 1, 6
      BUD(L) = BUD(L) + CSARBT(IABS(ARC),L)
    ENDDO
    IF (K .EQ. LIM2) THEN
      NARC = NARC + 1
      WRITE (OU, FMT) NARC, NDNAM(I), NDNAM(OJ),
        &      (BUD(L), L = 1, 6)
    ENDIF
150    CONTINUE
200    CONTINUE
*-----3. Save the final system water budget to the file
*
WRITE (OU, 950)
FORMAT(/, 'System water budgets',
  &      /, '=====')
*
ALLBUD(1) = CNDBT(0)+CARBT(2)
ALLBUD(2) = CNDBT(2)
ALLBUD(3) = CNDBT(4)
ALLBUD(4) = CNDBT(3)+CARBT(4)
ALLBUD(5) = CNDBT(5)+CARBT(3)
ALLBUD(6) = CNDBT(6)
ALLBUD(7) = CARBT(6)
ALLBUD(8) = CNDBT(8)+CARBT(5)
WRITE (OU, 955) 'Budget', 'Pond', 'Canal', 'Total'
WRITE (OU, 955) '-----', '-----', '-----', '-----'
WRITE(OU, FMT1) 'Initial storage:',
  &      CNDBT(0), CARBT(2), ALLBUD(1)
WRITE(OU, FMT1) 'Total net inflow:',
  &      CNDBT(2), 0.0, ALLBUD(2)
WRITE(OU, FMT1) 'Runoff:',
  &      CNDBT(4), 0.0, ALLBUD(3)
WRITE(OU, FMT1) 'Evaporation:',
  &      CNDBT(3), CARBT(4), ALLBUD(4)
WRITE(OU, FMT1) 'Ground-water seepage:',
  &      CNDBT(5), CARBT(3), ALLBUD(5)
IF (CNDBT(6) .GT. 0) THEN
  &      WRITE(OU, FMT1) 'Water withdrawal:',
    &      CNDBT(6), 0.0, ALLBUD(6)
ENDIF
WRITE(OU, FMT1) 'Outflow:',
  &      0.0, CARBT(6), ALLBUD(7)

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      WRITE(OU, FMT1) 'Final storage:',
&      CNDDBT(8), CARBT(5), ALLBUD(8)
&      ERRBD = ALLBUD(1) + ALLBUD(2) + ALLBUD(3) - ALLBUD(4)
&      - ALLBUD(5) - ALLBUD(6) - ALLBUD(7) - ALLBUD(8)
&      WRITE (OU, FMT2) 'In - Out = ', ERRBD
&      ENDIF
&      RETURN
955  FORMAT(10X, A, T35, 3A15)
990  FORMAT(/, 'Nodal water budgets for whole simulation',
&      /, '=====',
&      /, 'T21, ' Initial Upstream Local net',
&      /, 'Evap- Downstream Final',
&      /, 'T21, ' storage inflow inflow',
&      /, 'ration Rainfall Seepage Withdrawal release storage',
&      /, 'Node', T16, 'Node', T21, ' (acre- (acre- (acre-',
&      /, ' (acre- (acre- (acre- (acre- (acre- (acre-',
&      /, ' name', T16, 'type', T21, ' feet) feet) feet)',
&      /, ' feet) feet) feet) feet) feet) feet)',
&      /, '-----, T16, '-----, T21, '-----',
&      /, '-----')
995  FORMAT(/, 'Canal water budgets for whole simulation',
&      /, '=====',
&      /, 'T30, ' Surface',
&      /, 'T30, ' Initial Canal evapo- Final',
&      /, 'T30, ' Inflow storage seepage ration storage',
&      /, ' Outflow',
&      /, 'T30, ' (acre- (acre- (acre- (acre-',
&      /, ' (acre-',
&      /, ' No. From', T18, 'To',
&      /, 'T30, ' feet) feet) feet) feet)',
&      /, ' feet)',
&      /, '-----, T18, '-----',
&      /, 'T30, '-----',
&      /, '-----')
&      END
*=====
* Name:      namnum
* Purpose:   Search the node number with the node name, or vice versa.
* Author:    Xiaodong Jian
* Date:      9/13
*=====
SUBROUTINE NAMNUM(NNOD, NDNAMS, NDNAM, NDNUM, IW, ERR)
  IMPLICIT NONE
  INTEGER NNOD, NDNUM, IW
  CHARACTER NDNAMS(NNOD)*(*), NDNAM*(*)
  LOGICAL ERR, CN
*
  INTEGER I
*
  ERR = .FALSE.
  IF (IW .EQ. 0) THEN
    DO I = 0, NNOD
      IF (CN(NDNAMS(I), NDNAM, 1)) THEN
        NDNUM = I
        GOTO 99
      ENDIF
    ENDDO
  ELSE
    IF (NDNUM .LE. NNOD .AND. NDNUM .GE. 1) THEN
      NDNAM = NDNAMS(NDNUM)
      GOTO 99
    ENDIF
  ENDIF
  ERR = .TRUE.
  RETURN
99  END

```